

Test Report No. 608421-1 Test Report Date: September 2017

MASH TEST 3-11 OF 28-INCH W-BEAM GUARDRAIL SYSTEM WITH 8-INCH COMPOSITE BLOCKOUTS RAISED 4 INCHES ON STEEL POSTS

by

Chiara Silvestri Dobrovolny, Ph.D. Associate Research Scientist

Wanda L. Menges Research Specialist

and

Darrell L. Kuhn, P.E. Research Specialist

Contract No.: T4541 CP Test No.: 608421-1 Test Date: 2017-06-30



Sponsored by Roadside Safety Research Program Pooled Fund Study No. TPF-5(114)

TEXAS A&M TRANSPORTATION INSTITUTE PROVING GROUND

Mailing Address: Roadside Safety & Physical Security Texas A&M University System 3135 TAMU College Station, TX 77843-3135 Located at: Texas A&M University RELLIS Campus Building 7091 3100 State Highway 47 Bryan, TX 77807



DISCLAIMER

The contents of this report reflect the views of the authors who are solely responsible for the facts and accuracy of the data, and the opinions, findings, and conclusions presented herein. The contents do not necessarily reflect the official views or policies of the Roadside Safety Pooled Fund Study, The Texas A&M University System, or Texas A&M Transportation Institute. This report does not constitute a standard, specification, or regulation. In addition, the above listed agencies/ companies assume no liability for its contents or use thereof. The names of specific products or manufacturers listed herein do not imply endorsement of those products or manufacturers. The results reported herein apply only to the article being tested. The full-scale crash test was performed according to TTI Proving Ground quality procedures and according to the *MASH* guidelines and standards.

REPORT REVIEWED BY:

DocuSigned by: Glenn Schroeder -E692F9CB5047487...

Glenn Schroeder, Research Specialist Drafting & DCAT Reporting

-DocuSigned by: Gary Gerke

-FBA2101E9F6B4B7. Gary Gerke, Research Specialist

DCAS Construction

DocuSigned by: Scott Dobrovolny -1C613885787C44C..

Scott Dobrovolny, Research Specialist DCAS Mechanical Instrumentation

REPORT APPROVED BY:

DocuSigned by:

Ken Reeves

Ken Reeves, Research Specialist DCAS Electronics Instrumentation

---- DocuSigned by:

Pichard Badillo — 0F51DA60AB144F9...

Richard Badillo, Research Specialist DCAS Photographic Instrumentation

-DocuSigned by:

Wanda L. Menger -B92179622AF24FE.

Wanda L. Menges, Research Specialist DCAS Reporting & Deputy QM

DocuSigned by: ~~~ P CC23E85D5B4E7..

Darrell L. Kuhn, P.E., Research Specialist Quality Manager

—DocuSigned by: Matt Robinson

EAA22BFA5BFD417...

Matthew N. Robinson, Research Specialist Test Facility Manager & Technical Manager

DocuSigned by: 12 St Dy -36EDAD98EFE94EC

Chiara Silvestri Dobrovolny, Ph.D. Associate Research Scientist

			Technical Rep	ort Documentation Page
1. Report No.	2. Government Accession	n No.	3. Recipient's Catalog No).
4. Title and Subtitle MASH TEST 3-11 OF 28-INCH W-BEAM GUARDRAIL		5. Report Date September 2017		
SYSTEM WITH 8-INCH COMPO INCHES ON STEEL POSTS	DSITE BLOCKO	UTS RAISED 4	6. Performing Organizati	on Code
^{7.} Author(s) Chiara Silvestri Dobrovolny, Wanda	L. Menges, and D	arrell L. Kuhn	8. Performing Organizati Test Report No. 6	on Report No. 508421-1
9. Performing Organization Name and Address Texas A&M Transportation Institute	Proving Ground		10. Work Unit No. (TRA)	IS)
3135 TAMU College Station, Texas 77843-3135	-		11. Contract or Grant No. T4541 CP	
12. Sponsoring Agency Name and Address Washington State Department of Tra Transportation Building MS 47272	ansportation		13. Type of Report and Pe Technical Report March Sontamk	eriod Covered
Olympia, Washington 98504-7372			14. Sponsoring Agency C	lode
Raised 4 Inches on Steel Posts Name of Contacting Representative: 16. Abstract	Ali Hangul, Tenn	lessee Department	of Transportation	DIOCKOUIS
Inch composite blockouts raised on steel complement any existing guideline regar achieve recommended rail height for a V The purpose of the test reported with 8-inch composite blockouts raised included in the American Association o <i>Assessing Safety Hardware (MASH)</i> . T involves a 2270P vehicle impacting the impact speed and impact angle of 62 mi This report provides details of t documentation of the crash test perform system according to <i>MASH</i> Test 3-11 ev The 28-inch W-beam guardrail vehicle. The vehicle did not penetrate, the test was 52.6 inches. Detached bloc compartment, or present undue hazard t occurred. The 2270P vehicle remained were 32 degrees and 12 degrees, respec <i>MASH</i> .	posts as a means of rding the procedure of V-beam guardrail. I herein was to assess on steel posts accord f State Highway and he crash test was per 28-inch W-beam gu i/h and 25 degrees, re he 28-inch W-beam ued, results of the cra valuation criteria. system with raised c underride, or overrid kouts did not penetry o others in the area. upright during and a tively. Occupant risk	adjusting rail height of raising blockout n is the performance of ding to the safety-per- Transportation Offi- formed in accordance ardrail system with espectively. guardrail system with sh test, and assessme composite blockouts le the installation. No ate or show potentia No reduction or intra- fifer the collision per k factors were within	The outcome of the hounting height on standard the 28-inch W-beam formance evaluation cials (AASHTO) <i>Mace with MASH</i> Test 3 raised composite block the raised composite block the raised composite the ent of the performant contained and redire formation data and redire for the performant data and redire for the performant contained and redire for the performant data and the preferred limits and the preferred limits and the performant data and th	us study will teel posts to n guardrail system n guidelines <i>anual for</i> 3-11, which ckouts at a target blockouts, ce of the tested ected the 2270P eflection during occupant nt compartment and pitch angles a specified in
The 28-inch W-beam guardrail MASH Test 3-11. ^{17. Key Words} W-Beam, Guardrail, MASH, Longit Blockouts, Raised Blockouts, Comp	udinal Barriers, osite Blockouts,	ite blockouts raised 18. Distribution Statemen Copyrighted. No consent from the	4 inches performed a t t to be copied or re Roadside Safety P	acceptably for eprinted without ooled Fund.
The 28-inch W-beam guardrail MASH Test 3-11. 17. Key Words W-Beam, Guardrail, MASH, Longit Blockouts, Raised Blockouts, Comp Crash Testing, Roadside Safety 19. Security Classif.(of this report)	udinal Barriers, osite Blockouts, 20. Security Classif.(of thi	18. Distribution Statemen Copyrighted. No consent from the	4 inches performed a t t to be copied or re <u>Roadside Safety P</u> 21. No. of Pages	acceptably for eprinted without <u>ooled Fund</u> . 22. Price

	SI" (MODER	N METRIC) CONV	ERSION FACIORS	
	APPRO	(IMATE CONVERSTIC	ONS TO SI UNITS	
Symbol	When You Know	Multiply By	To Find	Symbol
		LENGTH		
in	inches	25.4	millimeters	mm
ft	feet	0.305	meters	m
yd	yards	0.914	meters	m
mi	miles	1.61	kilometers	km
		AREA		0
in ²	square inches	645.2	square millimeters	mm²
ft ²	square feet	0.093	square meters	m²
yd²	square yards	0.836	square meters	m²
ac	acres	0.405	nectares	na lum ²
mi-	square miles	2.59 VOLUME	square kilometers	Km-
floz	fluid ourooo		millilitoro	ml
11 02		29.57	litere	I I I
9ai #3	gallons cubic foot	3.765	illers	L m ³
ud ³	cubic varde	0.028	cubic meters	m ³
yu	NOTE: volu	imes greater than 1000	shall be shown in m ³	
	NOTE: VOI	MASS		
07	OUDCES	28 35	arams	a
lb	pounds	0 454	kilograms	y ka
Т	short tons (2000 lb)	0.907	megagrams (or metric ton")	Ma (or "t")
	T	EMPERATURE (exac	t degrees)	ing (or t)
°F	- Fahrenheit	5(F-32)/9	Celsius	°C
•	r unionion	or (F-32)/1.8	0010100	Ũ
		ILLUMINATIO	N	
fc	foot-candles	10.76	lux	lx
fl	foot-Lamberts	3.426	candela/m ²	cd/m ²
	FO	RCE and PRESSURE	or STRESS	
lbf	poundforce	4.45	newtons	Ν
lbf/in ²	poundforce per square inc	ch 6.89	kilopascals	kPa
	APPROXI	MATE CONVERSTIO	NS FROM SI UNITS	
Symbol	When You Know	Multiply By	To Find	Symbol
				Oynibol
		LENGTH		Cymbol
mm	millimeters	LENGTH 0.039	inches	in
mm m	millimeters meters	LENGTH 0.039 3.28	inches feet	in ft
mm m m	millimeters meters meters	LENGTH 0.039 3.28 1.09	inches feet yards	in ft yd
mm m m km	millimeters meters meters kilometers	LENGTH 0.039 3.28 1.09 0.621	inches feet yards miles	in ft yd mi
mm m m km	millimeters meters meters kilometers	LENGTH 0.039 3.28 1.09 0.621 AREA	inches feet yards miles	in ft yd mi
mm m km mm ²	millimeters meters meters kilometers square millimeters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016	inches feet yards miles square inches	in ft yd mi in ²
mm m km mm ² m ²	millimeters meters meters kilometers square millimeters square meters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764	inches feet yards miles square inches square feet	in ft yd mi in ² ft ²
mm m km mm ² m ²	millimeters meters meters kilometers square millimeters square meters square meters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195	inches feet yards miles square inches square feet square yards	in ft yd mi in ² ft ² yd ²
mm m km mm ² m ² ha 2	millimeters meters meters kilometers square millimeters square meters square meters hectares	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.000	inches feet yards miles square inches square feet square yards acres	in ft yd mi in ² ft ² yd ² ac
mm m km mm ² m ² ha km ²	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386	inches feet yards miles square inches square feet square feet square yards acres square miles	in ft yd mi in ² ft ² yd ² ac mi ²
mm m km mm ² m ² ha km ²	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME	inches feet yards miles square inches square feet square feet square yards acres square miles	in ft yd mi in ² ft ² yd ² ac mi ²
mm m km mm ² m ² ha km ²	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.001	inches feet yards miles square inches square inches square feet square yards acres square miles fluid ounces	in ft yd mi in ² ft ² yd ² ac mi ² oz
mm m km mm ² m ² ha km ² ha km ² mL L	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 25 214	inches feet yards miles square inches square inches square feet square yards acres square miles fluid ounces gallons	in ft yd mi in ² ft ² yd ² ac mi ² oz gal
mm m km mm ² m ² ha km ² ha km ² L L m ³ m ³	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.207	inches feet yards miles square inches square feet square feet square yards acres square miles fluid ounces gallons cubic feet	in ft yd mi in ² ft ² yd ² ac mi ² Oz gal ft ³ yd ³
mm m km mm ² m ² ha km ² ha km ² L L m ³ m ³	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³
mm m km mm ² m ² ha km ² mL L m ³ m ³	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³
mm m km mm ² m ² ha km ² mL L m ³ m ³ g	millimeters meters meters kilometers square millimeters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 p") 1.103	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds schott tops (2000lb)	in ft yd mi in ² ft ² yd ² ac mi ² Oz gal ft ³ yd ³ Oz lb T
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t")	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (avec	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb)	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t")	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to T	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (exac	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb) t degrees)	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t")	millimeters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to T Celsius	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (exac 1.8C+32	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb) t degrees) Fahrenheit	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t") °C	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to T Celsius	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (exac 1.8C+32 ILLUMINATIO 0.0290	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb) t degrees) Fahrenheit	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T °F
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t") °C	millimeters meters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to T Celsius	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (exact 1.8C+32 ILLUMINATIO 0.0929 2.2010	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb) t degrees) Fahrenheit N foot-candles	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T °F
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t") °C Ix cd/m ²	millimeters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to T Celsius	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (exact 1.8C+32 ILLUMINATIO 0.0929 0.2919	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb) t degrees) Fahrenheit N foot-candles foot-Lamberts	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T °F fc fl
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t") °C lx cd/m ²	millimeters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to T Celsius lux candela/m ²	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (exac 1.8C+32 ILLUMINATIO 0.0929 0.2919 RCE and PRESSURE	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb) t degrees) Fahrenheit N foot-candles foot-Lamberts	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T °F fc fl
mm m km mm ² m ² ha km ² mL L m ³ m ³ g kg Mg (or "t") °C Ix cd/m ²	millimeters meters kilometers square millimeters square meters square meters hectares Square kilometers milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric to T Celsius lux candela/m ² FO newtons	LENGTH 0.039 3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 n") 1.103 EMPERATURE (exac 1.8C+32 ILLUMINATIO 0.0929 0.2919 RCE and PRESSURE 0.225	inches feet yards miles square inches square feet square yards acres square miles fluid ounces gallons cubic feet cubic yards ounces pounds short tons (2000lb) t degrees) Fahrenheit N foot-candles foot-Lamberts or STRESS poundforce	in ft yd mi in ² ft ² yd ² ac mi ² oz gal ft ³ yd ³ oz lb T °F fc fl lbf

*SI is the symbol for the International System of Units. Appropriate rounding should be made to comply with Section 4 of ASTM E380. (Revised March 2003)

ACKNOWLEDGMENTS

This research project was performed under a pooled fund program between the following States and Agencies. The authors acknowledge and appreciate their guidance and assistance.

Roadside Safety Research Pooled Fund Committee

Revised August 2017

<u>ALASKA</u>

Jeff C. Jeffers, P.E.

Statewide Standard Specifications Alaska Department of Transportation & Public Facilities 3132 Channel Drive P.O. Box 112500 Juneau, AK 99811-2500 (907) 465-6951 Jeff.Jeffers@alaska.gov

CALIFORNIA

Bob Meline, P.E. Caltrans Office of Materials and Infrastructure Division of Research and Innovation 5900 Folsom Blvd Sacramento, CA 95819 (916) 227-7031 Bob.Meline@dot.ca.gov

John Jewell, P.E.

(916) 227-5824 John_Jewell@dot.ca.gov

<u>COLORADO</u>

Larry Brinck, P.E. Standards & Specifications Engineer Project Development Branch 4201 E Arkansas Ave, 4th Floor Denver, CO 80222 (303) 757-9474 Larry.Brinck@state.co.us

CONNECTICUT

David Kilpatrick State of Connecticut Department of Transportation 2800 Berlin Turnpike Newington, CT 06131-7546 (806) 594-3288 David.Kilpatrick@ct.gov

DELAWARE

Adam Weiser, P.E., PTOE Safety Programs Manager Delaware Department of Transportation 169 Brick Store Landing Road Smyrna, DE 19977 (302) 659-4073 Adam.Weiser@state.de.us

FLORIDA

Derwood C. Sheppard, Jr., P.E. Design Standards Administrator FDOT Roadway Design Office Florida Department of Transportation 605 Suwannee Street Tallahassee, FL 32399-0450 (850) 414-4334 Derwood.Sheppard@dot.state.fl.us

IDAHO

Gary Sanderson Design/Traffic Section Idaho Transportation Department P. O. Box 7129 Boise, ID 83707-112 (208) 334-8211 Gary.Sanderson@ITD.idaho.gov

ILLINOIS

Paul L. Lorton Acting Bureau Chief Bureau of Safety Programs and EngineeringIllinois Department of Transportation 2300 Dirksen Parkway, Room 323 Springfield, IL 62764 (217) 785-0720 Paul.Lorton@illinois.gov

ILLINOIS (continued)

Filberto (Fil) Sotelo

Safety Evaluation Engineer Illinois Department of Transportation Bureau of Safety Programs and Engineering 2300 South Dirksen Parkway, Room 005 Springfield, IL 62764 (717) 557-2563 Filiberto.Sotelo@illinois.gov

Tracy Borchardt

Illinois Tollway General Engineering Consultant (312) 823-5005 Tracy.Borchardt@aecom.com

LOUISIANA

Kurt Brauner, P.E. Bridge Engineer Manager Louisiana Transportation Center P. O. Box 94245 Baton Rouge, LA 79084-9245 (225) 379-1328 Kurt.Brauner@la.gov

MASSACHUSETTS

Alex Bardow Director of Bridges and Structure Massachusetts Department of Transportation 10 Park Plaza Boston, MA 02116 (517) 335-9430 Alexander.Bardow@state.ma.us

James Danila

Assistant State Traffic Engineer Massachusetts Department of Transportation 10 Park Plaza, Room 7210 Boston, MA 02116 (857) 368-9640 James.Danila@state.ma.us

MICHIGAN

Carlos Torres, P.E. Crash Barrier Engineer Geometric Design Unit, Design Division Michigan Department of Transportation P. O. Box 30050 Lansing, MI 48909 (517) 335-2852 TorresC@michigan.gov

MINNESOTA

Michael Elle, P.E.

Design Standards Engineer Minnesota Department of Transportation 395 John Ireland Blvd, MS 696 St. Paul, MN 55155-1899 (651) 366-4622 Michael.Elle@state.mn.us

Michelle Moser

Assistant Design Standards Engineer MnDOT-Office of Project Management and Technical Support (651) 366-4708 Michelle.Moser@state.mn.us

<u>OREGON</u>

Christopher Henson

Senior Roadside Design Engineer Oregon Department of Transportation Technical Service Branch 4040 Fairview Industrial Drive, SE Salem, OR 97302-1142 (503) 986-3561 Christopher.S.Henson@odot.state.or.us

PENNSYLVANIA

Divyang P. Pathak, EIT Standards & Criteria Engineer Pennsylvania Department of Transportation Bureau of Project Delivery 400 North Street Harrisburg, PA 17105 (717) 705-4190 DPathak@pa.gov

TENNESSEE

Ali Hangul, P.E., CPESC Assistant Director Tennessee Department of Transportation Roadway Design & Office of Aerial Surveys James K. Polk State Office Bldg, Ste 1300 505 Deaderick Street Nashville, TN 37243-0348 (615) 741-0840 Ali.Hangul@tn.gov

<u>TEXAS</u>

Chris Lindsey Transportation Engineer Design Division Texas Department of Transportation 125 East 11th Street Austin, TX 78701-2483 (512) 416-2750 Christopher.Lindsey@txdot.gov

Taya Retterer P.E.

TXDOT Bridge Standards Engineer 125 E 11th ST. Austin, TX 78701 (512) 416-2719 Taya.Retterer@txdot.gov

WASHINGTON

Jeffery K. Petterson, P.E. Design Policy & Strategic Analysis Estimating Manager Roadside Safety Engineer Washington State Department of Transportation P. O. Box 47329 Olympia, WA 98504-7246 (360) 705-7278 PetterJ@wsdot.wa.gov

Rhonda Brooks

Director of Research Office Washington State Department of Transportation P.O. Box 47372 Olympia, WA 98504-7372 (360) 705-7945 BrookRh@wsdot.wa.gov

WEST VIRGINIA

Donna J. Hardy, P.E. Safety Programs Engineer West Virginia Department of Transportation – Traffic Engineering Building 5, Room A-550 1900 Kanawha Blvd E. Charleston, WV 25305-0430 (304) 558-9576 Donna.J.Hardy@wv.gov

WEST VIRGINIA (continued)

Joe Hall, P.E., P.S. Division of Highways & Engineering Technical Policy QA/QC Engineer Value Engineering Coordinator 1334 Smith Street Charleston, WV 25305-0430 (304) 558-9733 Joe.H.Hall@wv.gov

Ted Whitmore Traffic Services Engineer (304) 558-9576 Ted.J.Whitmore@wv.gov

<u>WISCONSIN</u>

Erik Emerson, P.E. Standards Development Engineer – Roadside Design Wisconsin Department of Transportation Bureau of Project Development 4802 Sheboygan Avenue, Room 651 P. O. Box 7916 Madison, WI 53707-7916 (608) 266-2842 Erik.Emerson@wi.gov

<u>ONTARIO</u>

Mark Ayton, P. Eng. Senior Engineer, Highway Design Design & Contract Standards Office Ontario Ministry of Transportation Garden City Tower, 2nd Floor North 301 St. Paul Street St. Catharines, Ontario L2R 7R4 (904) 704-2051 Mark.Ayton@ontario.ca

FEDERAL HIGHWAY ADMINISTRATION

Richard B. (Dick) Albin, P.E. Safety Engineer FHWA Resource Center Safety & Design Technical Services Team 711 S. Capital Olympia, WA 98501 (303) 550-8804 Dick.Albin@dot.gov

FEDERAL HIGHWAY ADMINISTRATION (continued)

William Longstreet

Highway Engineer FHWA Office of Safety Design Room E71-107 1200 New Jersey Avenue, S.E. Washington, DC 20590 (202) 366-0087 Will.Longstreet@dot.gov

Eduardo Arispe

Research Highway Safety Specialist U.S. Department of Transportation Federal Highway Administration Turner-Fairbank highway Research Center Mail Code: HRDS-10 6300 Georgetown Pike McLean, VA 22101 (202) 493-3291 Eduardo.arispe@dot.gov

Greg Schertz, P.E.

FHWA – Federal Lands Highway Division Safety Discipline Champion 123 West Dakota Ave. Ste. 210 Lakewood, CO 80228 (720)-963-3764 (303)-588-4042 cell Greg.Schertz@dot.gov

WebSite: safety.fhwa.dot.gov

TEXAS A&M TRANSPORTATION INSTITUTE

D. Lance Bullard, Jr., P.E. Senior Research Engineer Roadside Safety & Physical Security Div. Texas A&M Transportation Institute 3135 TAMU College Station, TX 77843-3135 (979) 845-6153 L-Bullard@tti.tamu.edu

Roger P. Bligh, Ph.D., P.E. Senior Research Engineer (979) 845-4377 R-Bligh@tti.tamu.edu

Chiara Silvestri Dobrovolny, Ph.D.

Associate Research Scientist (979) 845-8971 C-Silvestri@tti.tamu.edu

WebSite: tti.tamu.edu

www.roadsidepooledfund.org

TABLE OF CONTENTS

]	Page
Disclaimer	•••••••••••••••••••••••••••••••••••••••	ii
List of Figure	es	viii
List of Tables	5	ix
Chapter 1.	Introduction	1
1.1	Problem Statement	1
1.2	Background	1
1.3	Objective	2
Chapter 2.	System Details	3
2.1.	Test Article and Installation Details	3
2.2.	Material Specifications	3
2.3.	Soil Conditions	3
Chapter 3.	Test Requirements and Evaluation Criteria	7
3.1.	Crash Test Performed / Matrix	7
3.2.	Evaluation Criteria	8
Chapter 4.	Test Conditions	9
4 .1.	Test Facility	9
4.2	Vehicle Tow and Guidance System	9
4.3	Data Acquisition Systems	9
4.3.1	Vehicle Instrumentation and Data Processing	9
4.3.2	Anthropomorphic Dummy Instrumentation	10
4.3.3	Photographic Instrumentation Data Processing	10
Chapter 5.	MASH Test 3-11 (Crash Test No. 608421-1)	13
5.1	Test Designation and Actual Impact Conditions	13
5.2	Weather Conditions	13
5.3	Test Vehicle	13
5.4	Test Description	14
5.5	Damage to Test Installation	15
5.6	Vehicle Damage	17
5.7	Occupant Risk Factors	18
Chapter 6.	Summary and Conclusions	21
6.1.	Assessment of Test Results	21
6.2	Conclusions	21
6.3	Implementation	21
References	1	23
Appendix A.	Details of the 28-inch W-Beam Guardrail System with Raised	
II.	Composite Blockouts	25
Appendix B.	Supporting Certification Documents	27
Appendix C.	Soil Properties	29
Appenidx D.	MASH Test 3-11 (Crash Test No. 608421-1)	31
D1	Vehicle Properties and Information	31
D2	Sequential Photographs	35
D3	Vehicle Angular Displacements	38
D4	Vehicle Accelerations	39

LIST OF FIGURES

		Page
Figure 2.1.	Details of the 28-inch W-Beam Guardrail System with Raised Composite Blockouts.	4
Figure 2.2.	28-inch W-Beam Guardrail System with Raised Composite Blockouts prior to Testing	5
Figure 3.1.	Target CIP for <i>MASH</i> Test 3-11 on the 28-inch W-beam Guardrail System with Raised Composite Blockouts.	7
Figure 5.1.	28-inch W-Beam Guardrail System with Raised Composite Blockouts/Test Vehicle Geometrics for Test No. 608421-1	13
Figure 5.2.	Test Vehicle before Test No. 608421-1.	14
Figure 5.3.	Test Vehicle and Guardrail System after Test No. 608421-1	15
Figure 5.4.	Upstream Terminal after Test No. 608421-1.	16
Figure 5.5.	Posts 11 through 14 after Test No. 608421-1.	16
Figure 5.6.	Posts 15 through 18 after Test No. 608421-1.	17
Figure 5.7.	Field Side of Guardrail and Released Blockouts after Test No. 608421-1	17
Figure 5.8.	Test Vehicle after Test No. 608421-1	18
Figure 5.9.	Interior of Test Vehicle for Test No. 608421-1.	18
Figure 5.10.	Summary of Results for MASH Test 3-11 on 28-inch W-Beam Guardrail	
	System with Raised Composite Blockouts.	20
Figure D.1.	Sequential Photographs for Test No. 608421-1 (Overhead and Frontal Views).	35
Figure D.2.	Sequential Photographs for Test No. 608421-1 (Rear View).	37
Figure D.3.	Vehicle Angular Displacements for Test No. 608421-1	38
Figure D.4.	Vehicle Longitudinal Accelerometer Trace for Test No. 608421-1	
-	(Accelerometer Located at Center of Gravity).	39
Figure D.5.	Vehicle Lateral Accelerometer Trace for Test No. 608421-1	
	(Accelerometer Located at Center of Gravity).	40
Figure D.6.	Vehicle Vertical Accelerometer Trace for Test No. 608421-1	
	(Accelerometer Located at Center of Gravity).	41
Figure D.7.	Vehicle Longitudinal Accelerometer Trace for Test No. 608421-1	
	(Accelerometer Located Rear of Center of Gravity)	42
Figure D.8.	Vehicle Lateral Accelerometer Trace for Test No. 608421-1	
	(Accelerometer Located Rear of Center of Gravity)	43
Figure D.9.	Vehicle Vertical Accelerometer Trace for Test No. 608421-1	
	(Accelerometer Located Rear of Center of Gravity)	44

LIST OF TABLES

		Page
Table 3.1.	Test Conditions and Evaluation Criteria Specified for MASH TL-3.	7
Table 3.2.	Evaluation Criteria Required for MASH Test 3-11	
Table 5.1.	Events during Test No. 608421-1.	
Table 5.2.	Occupant Risk Factors for Test No. 608421-1.	
Table 6.1.	Performance Evaluation Summary for MASH Test 3-11 on 28-inch W-	
	Beam Guardrail System with Raised Composite Blockouts	
Table C.1.	Summary of Strong Soil Test Results for Establishing Installation	
	Procedure.	
Table C.2.	Test Day Static Soil Strength Documentation for Test No. 608421-1	
Table D.1.	Vehicle Properties for Test No. 608421-1.	
Table D.2.	Measurements of Vehicle Vertical CG for Test No. 608421-1.	
Table D.3.	Exterior Crush Measurements for Test No. 608421-1	
Table D.4.	Occupant Compartment Measurements for Test No. 608421-1.	

Chapter 1. INTRODUCTION

1.1 PROBLEM STATEMENT

With recent changes/clarifications about appropriate height for W-beam guardrails, existing locations have been identified where rail height is lower than recommended. Pavement overlays can create locations where this occurs. Raising the blockout on the post is a simple, low-cost mean to adjust the rail height. However, it is unknown how the rail will perform with raised composite blockouts.

The purpose of this research is to test and evaluate the performance of a 28-inch W-beam rail system with 8-inch composite blockouts raised on steel posts as a means of adjusting rail height. The outcome of this study will complement any existing guidelines regarding the procedure of raising blockout mounting height on steel posts to achieve the recommended rail height for a W-beam guardrail.

The information compiled from this research will enable the Departments of Transportation to decide whether raising blockouts on the posts can be used as a low-cost mean to adjust rail height.

1.2 BACKGROUND

On May 17, 2010, FHWA issued a technical memorandum to provide guidance to State DOTs and FHWA Division Offices on height of guardrail for new installations on the National Highway System (NHS) (1). The technical memorandum details the minimum mounting heights of systems successfully crash tested per the National Cooperative Highway Research Program (NCHRP) Report 350 "Recommended Procedures for the Safety Performance Evaluation of Highway Features" and the American Association of State Highway and Transportation Officials (AASHTO) *Manual for Assessing Safety Hardware (MASH) (2, 3)*. In regard to *MASH*, the memorandum recognized performance issues with modified G4(1S) guardrail, and recommended adoption of 31-inch high guardrail designs for new installations.

The FHWA Office of Safety Design and the FHWA Resource Center give suggestions on how to adjust rail height when pavement work is needed. When the barrier does not need to be moved it is a common practice to raise the blockout on the post up to three inches as a costeffective means to adjust rail height. This requires field drilling or punching of a new hole in the guardrail post.

Raising the blockouts above the post can induce flexural stresses into the blockout in addition to the compressive stresses that are normally present during an impact. Thus, since the stresses in the blockouts are altered compared to intended design conditions, the impact performance of the guardrail with raised blockouts needs to be investigated. The information compiled from this research will enable the Departments of Transportation to decide whether raising blockouts on the posts can be chosen as a cost-effective means to adjust rail height when below recommended value, without compromising the rail system performance.

A recently completed research study funded by the Roadside Safety Pooled Fund group investigated behavior of wood and composite blockouts raised 4 inches on steel posts (4). Pendulum tests were conducted to determine the dynamic impact performance of wood and composite blockouts raised on a steel guardrail post. From the comparison of the energy plots, it appears that the system with wood blockouts was able to absorb more energy during the impact event in comparison to the systems which used proprietary composite blockouts. Also, the wood blockouts remained attached to the post and were not fractured as a consequence of the first impact from the pendulum nose. In each test, however, the composite blockout had sufficient strength to develop the capacity of the steel guardrail post. If the raised composite blockout fractured or detached, this behavior occurred after the post had twisted more than 90 degrees out of plane with the guardrail and in some cases was related to a secondary impact from the pendulum as it swung back after the initial impact event. If a guardrail post is laterally loaded to the point it twists 90 degrees or more as it bends and deflects, it is likely that the guardrail has detached from the blockout and the effective offset distance has been reduced. Researchers concluded that fracture of the blockout at this time is not likely to affect the outcome of the impact event.

1.3 OBJECTIVE

The purpose of the test reported herein was to assess the performance of a 28-inch W-beam guardrail system with 8-inch wide composite blockouts raised on steel posts according to the safety-performance evaluation guidelines included in the AASHTO *MASH*. The crash test was performed in accordance with *MASH* Test 3-11, which involves a 2270P vehicle impacting the 28-inch W-beam guardrail system with raised composite blockouts at a target impact speed and impact angle of 62 mi/h and 25 degrees, respectively.

This report provides details of the 28-inch W-beam guardrail system with raised composite blockouts, documentation of the crash test performed, results of the crash test, and assessment of the performance of the tested system according to *MASH* Test 3-11 evaluation criteria.

Chapter 2. SYSTEM DETAILS

2.1. TEST ARTICLE AND INSTALLATION DETAILS

The test installation consisted of a 28-inch tall W-beam with structural steel posts (posts 3-27) guardrail system with a TxDOT GF (31) DAT-14 terminal on the each end, for a total installation length of 175 ft-0 inches. Mondo Polymer Blockouts (Model #GB14SH1) were installed on posts 3 through 27.

Standard 12-gauge W-beam guardrail (type RWM04a) was used in the system. The top of the W-beam was 28 inches above grade, with the top of the post 25 inches above grade. The top of the Mondo Polymer Blockouts were 1-inch above the guardrail and 4-inches above the posts. Guardrail splices were located at every other post, and the posts were equally spaced at 6 ft-3 inches.

Guardrail posts 3 through 27 were modified PWE01 line posts fabricated from W6×8.5 ASTM A36 structural steel shape, with an additional $^{13}/_{16}$ -inch diameter hole centered 3 inches below the post's top and 1½ inches from the centerline of the web. The two existing $^{13}/_{16}$ -inch diameter holes (centered 7 inches below the top and 2¼ inches straddling the web) were not used. These 25 posts were installed 47-inch deep in drilled holes. Guardrail offset for posts 3 to 27 was accomplished by use of 7½-inch deep Mondo Polymer Blockouts (Model #GB14SH1) attached with standard 10-inch long guardrail bolts and recessed nuts (FBB03) in the aforementioned hole centered 3 inches from the top.

Each TxDOT GF (31) DAT-14 terminal was 9 ft-4¹/₂ inches long as measured from the anchor posts to the W-beam splice between posts 2 and 3 and posts 27 and 28, respectively.

Figure 2.1 presents overall information on the 28-inch W-beam guardrail system with raised composite blockouts, and Figure 2.2 provides photographs of the installation. Appendix A provides further details of the 28-inch W-beam guardrail system with raised composite blockouts, Mondo Blockout, and TxDOT GF (31) DAT-14 terminal system.

2.2. MATERIAL SPECIFICATIONS

Appendix B provides material certification documents for the materials used to install/construct the 28-inch W-beam guardrail system with raised composite blockouts.

2.3. SOIL CONDITIONS

The test installation was installed in standard soil meeting AASHTO standard specifications for "Materials for Aggregate and Soil Aggregate Subbase, Base and Surface Courses", designated M147-65(2004), grading B.



Figure 2.1. Details of the 28-inch W-Beam Guardrail System with Raised Composite Blockouts.



Figure 2.2. 28-inch W-Beam Guardrail System with Raised Composite Blockouts prior to Testing.

In accordance with Appendix B of *MASH*, soil strength was measured the day of the crash test. During installation of the 28-inch W-beam guardrail system with raised composite blockouts for full-scale crash testing, two W6×16 posts were installed in the immediate vicinity of the 28-inch W-beam guardrail system with raised composite blockouts, utilizing the same fill materials and installation procedures used in the test installation and standard dynamic test (see Table C.1 in Appendix C for establishment minimum soil strength properties in the dynamic test performed in accordance with *MASH* Appendix B).

As determined in the tests shown in Appendix C, Table C.1, the minimum post load required at deflections of 5 inches, 10 inches, and 15 inches, measured at a height of 25 inches, is 3940 lb, 5500 lb, and 6540 lb, respectively (90% of static load for the initial standard installation). On the day of the test, June 30, 2017, load on the post at deflections of 5 inches, 10 inches, and 15 inches was 5757 lbf, 6313 lbf, and 6767 lbf, respectively. In Appendix C, Table C.2 shows that the strength of the backfill material, in which the 28-inch W-beam guardrail system with raised composite blockouts was installed, met minimum soil strength requirements.

Chapter 3. TEST REQUIREMENTS AND EVALUATION CRITERIA

3.1. CRASH TEST PERFORMED / MATRIX

Table 3.1 shows the test conditions and evaluation criteria for longitudinal barriers for *MASH* Test Level 3 (TL-3). *MASH* Test 3-11 involves a 2270P vehicle weighing 5000 lb \pm 110 lb and impacting the critical impact point (CIP) of the 28-inch W-beam guardrail system with raised composite blockouts at an impact speed of 62 mi/h \pm 2.5 mi/h and an angle of 25 degrees \pm 1.5 degrees. The target CIP selected for the test was determined according to the information provided in *MASH* Section 2.3.2.1 and *MASH* Appendix A2.3, and was 0.7 ft \pm 1 ft downstream of the centerline of post 12, as shown in Figure 3.1.

Table 3.1. Test Conditions and Evaluation Criteria Specified for MASH TL-3.

Test Article	Test Test Designation Vehicle		Impact Conditions		Evaluation	
Designation		venicie	Speed	Angle	Criteria	
Longitudinal	3-10	1100C	62 mi/h	25	A, D, F, H, I	
Barrier	3-11	2270P	62 mi/h	25	A, D, F, H, I	



Figure 3.1. Target CIP for *MASH* Test 3-11 on the 28-inch W-beam Guardrail System with Raised Composite Blockouts.

The crash test(s) and data analysis procedures were in accordance with guidelines presented in *MASH*. Chapter 4 presents brief descriptions of these procedures.

3.2. EVALUATION CRITERIA

The appropriate safety evaluation criteria from Tables 2-2A and 5-1A through 5-1C of *MASH* were used to evaluate the crash test reported herein. The test conditions and evaluation criteria required for *MASH* Test 3-11 are listed in Table 3.1, and the substance of the evaluation criteria in Table 3.2. An evaluation of the crash test results are presented in detail under the section Assessment of Test Results.

Evaluation Factors	Evaluation Criteria		
Structural Adequacy	A. Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.		
	D. Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present undue hazard to other traffic, pedestrians, or personnel in a work zone.		
Occupant	Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.2.2 and Appendix E of MASH.		
Risk	<i>F.</i> The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.		
	H. Occupant impact velocities (OIV) should satisfy the following limits: Preferred value of 30 ft/s, or maximum allowable value of 40 ft/s.		
	I. The occupant ridedown accelerations should satisfy the following: Preferred value of 15.0 g, or maximum allowable value of 20.49 g.		

 Table 3.2. Evaluation Criteria Required for MASH Test 3-11.

Chapter 4. TEST CONDITIONS

4.1. TEST FACILITY

The full-scale crash test reported herein was performed at Texas A&M Transportation Institute (TTI) Proving Ground, an International Standards Organization (ISO) 17025-accredited laboratory with American Association for Laboratory Accreditation (A2LA) Mechanical Testing Certificate 2821.01. The full-scale crash test was performed according to TTI Proving Ground quality procedures, and according to the *MASH* guidelines and standards.

The test facilities of the TTI Proving Ground are located on the Texas A&M University RELLIS Campus which consists of a 2000-acre complex of research and training facilities situated 10 miles northwest of the flagship campus of Texas A&M University. The site, formerly a United States Army Air Corps base, has large expanses of concrete runways and parking aprons well suited for experimental research and testing in the areas of vehicle performance and handling, vehicle-roadway interaction, durability and efficacy of highway pavements, and evaluation of roadside safety hardware and perimeter protective devices. The site selected for construction and testing of the 28-inch W-beam guardrail system with raised composite blockouts was along the edge of an out-of-service apron. The apron consists of an unreinforced jointed-concrete pavement in 12.5-ft \times 15-ft blocks nominally 6 inches deep. The aprons were built in 1942, and the joints have some displacement, but are otherwise flat and level.

4.2 VEHICLE TOW AND GUIDANCE SYSTEM

The test vehicle was towed into the test installation using a steel cable guidance and reverse tow system. A steel cable for guiding the test vehicle was tensioned along the path, anchored at each end, and threaded through an attachment to the front wheel of the test vehicle. An additional steel cable was connected to the test vehicle, passed around a pulley near the impact point, through a pulley on the tow vehicle, and then anchored to the ground such that the tow vehicle moved away from the test site. A 2:1 speed ratio between the test and tow vehicle existed with this system. Just prior to impact with the installation, the test vehicle was released and ran unrestrained. The vehicle remained freewheeling (i.e., no steering or braking inputs) until it cleared the immediate area of the test site (no sooner than 2 s after impact), after which the brakes were activated, if needed, to bring the test vehicle to a safe and controlled stop.

4.3 DATA ACQUISITION SYSTEMS

4.3.1 Vehicle Instrumentation and Data Processing

The test vehicle was instrumented with a self-contained, on-board data acquisition system. The signal conditioning and acquisition system is a 16-channel, Tiny Data Acquisition System (TDAS) Pro produced by Diversified Technical Systems, Inc. The accelerometers, which measure the x, y, and z axis of vehicle acceleration, are strain gauge type with linear millivolt output proportional to acceleration. Angular rate sensors, measuring vehicle roll, pitch, and yaw rates, are ultra-small, solid state units designed for crash test service. The TDAS Pro hardware and software conform to the latest SAE J211, Instrumentation for Impact Test. Each of the 16 channels is capable of providing precision amplification, scaling, and filtering based on transducer specifications and calibrations. During the test, data are recorded from each channel at a rate of 10,000 values per second with a resolution of one part in 65,536. Once data are recorded, internal batteries back these up inside the unit should the primary battery cable be severed. Initial contact of the pressure switch on the vehicle bumper provides a time zero mark as well as initiates the recording process. After each test, the data are downloaded from the TDAS Pro unit into a laptop computer at the test site. The Test Risk Assessment Program (TRAP) software then processes the raw data to produce detailed reports of the test results.

Each of the TDAS Pro units is returned to the factory annually for complete recalibration and all instrumentation used in the vehicle conforms to all specifications outlined by SAE J211. All accelerometers are calibrated annually by means of an ENDEVCO[®] 2901, precision primary vibration standard. This standard and its support instruments are checked annually and receive a National Institute of Standards Technology (NIST) traceable calibration. The rate transducers used in the data acquisition system receive a calibration via a Genisco Rate-of-Turn table. The subsystems of each data channel are also evaluated annually, using instruments with current NIST traceability, and the results are factored into the accuracy of the total data channel, per SAE J211. Calibrations and evaluations are also made any time data are suspect. Acceleration data is measured with an expanded uncertainty of ± 1.7 percent at a confidence factor of 95 percent (k=2).

TRAP uses the data from the TDAS Pro to compute occupant/compartment impact velocities, time of occupant/compartment impact after vehicle impact, and the highest 10-millisecond (ms) average ridedown acceleration. TRAP calculates change in vehicle velocity at the end of a given impulse period. In addition, maximum average accelerations over 50-ms intervals in each of the three directions are computed. For reporting purposes, the data from the vehicle-mounted accelerometers are filtered with a 60-Hz low-pass digital filter, and acceleration versus time curves for the longitudinal, lateral, and vertical directions are plotted using TRAP.

TRAP uses the data from the yaw, pitch, and roll rate transducers to compute angular displacement in degrees at 0.0001-s intervals, then plots yaw, pitch, and roll versus time. These displacements are in reference to the vehicle-fixed coordinate system with the initial position and orientation of the vehicle-fixed coordinate systems being initial impact. Rate of rotation data is measured with an expanded uncertainty of ± 0.7 percent at a confidence factor of 95 percent (k=2).

4.3.2 Anthropomorphic Dummy Instrumentation

According to *MASH*, use of a dummy in the 2270P vehicle is optional, and no dummy was used in the test.

4.3.3 Photographic Instrumentation Data Processing

Photographic coverage of the/each test included three digital high-speed cameras:

• One overhead with a field of view perpendicular to the ground and directly over the impact point;

- One placed behind the installation at an angle; and
- A third placed to have a field of view parallel to and aligned with the installation at the downstream end.

A flashbulb on the impacting vehicle was activated by a pressure-sensitive tape switch to indicate the instant of contact with the 28-inch W-beam guardrail system with raised composite blockouts. The flashbulb was visible from each camera. The video files from these digital high-speed cameras were analyzed to observe phenomena occurring during the collision and to obtain time-event, displacement, and angular data. A digital camera recorded and documented conditions of each test vehicle and the installation before and after the test.

Chapter 5. MASH TEST 3-11 (CRASH TEST NO. 608421-1)

5.1 TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS

MASH Test 3-11 involves a 2270P vehicle weighing 5000lb \pm 110 lb impacting the CIP of the 28-inch W-beam guardrail system with raised composite blockouts at an impact speed of 62 mi/h \pm 2.5 mi/h and an angle of 25 degrees \pm 1.5 degrees. The CIP for *MASH* Test 3-11 on the 28-inch W-beam guardrail system with raised composite blockouts was 0.7 ft \pm 1 ft downstream of the centerline of post 12.

The 2011 Dodge RAM 1500 pickup truck used in the test weighed 5017 lb, and the actual impact speed and angle were 64.1 mi/h and 24.4 degrees, respectively. The actual impact point was 1 ft downstream of post 12. Minimum target impact severity (IS) was 106 kip-ft, and actual IS was 118 kip-ft.

5.2 WEATHER CONDITIONS

The test was performed on the morning of June 30, 2017. Weather conditions at the time of testing were as follows: wind speed: 15 mi/h; wind direction: 183 degrees (vehicle was traveling in a southwesterly direction); temperature: 90°F; relative humidity: 71 percent.

5.3 TEST VEHICLE

The 2011 Dodge RAM 1500 pickup truck, shown in Figure 5.1 and Figure 5.2, was used for the crash test. The vehicle's test inertia weight was 5017 lb, and its gross static weight was 5017 lb. The height to the lower edge of the vehicle bumper was 11.5 inches, and height to the upper edge of the bumper was 27.0 inches. The height to the vehicle's center of gravity was 28.375 inches. Tables D.1 and D.2 in Appendix D1 give additional dimensions and information on the vehicle. The vehicle was directed into the installation using a cable reverse tow and guidance system, and was released to be freewheeling and unrestrained just prior to impact.



Figure 5.1. 28-inch W-Beam Guardrail System with Raised Composite Blockouts/Test Vehicle Geometrics for Test No. 608421-1.



Figure 5.2. Test Vehicle before Test No. 608421-1.

5.4 TEST DESCRIPTION

The test vehicle, traveling at an impact speed of 64.1 mi/h, contacted the 28-inch W-beam guardrail system with raised composite blockouts 12 inches downstream of post 12 at an impact angle of 24.4 degrees. Table 5.1 lists events that occurred during Test No. 608421-1. Figures D.1 and D.2 in Appendix D2 present sequential photographs during the test.

TIME	EVENT
0.017	Post #12 begins to deflect to field side
0.020	Post #13 begins to deflect to field side
0.022	Guardrail begins to deform between Posts #12 and #13
0.037	Post #14 begins to deflect to field side
0.060	Vehicle begins to redirect
0.070	Blockout begins to separate from post #13
0.072	Right front tire contacts Post #13 and rides up and over post
0.077	Post #15 begins to deflect to field side
0.105	Right front tire begins to deflate
0.105	Blockout begins to separate from post #14
0.116	Blockout releases from Post #14
0.127	Post #16 begins to deflect to field side
0.130	Right front tire impacts Post #14 and rides up and over post
0.140	Passenger door begins to open at top window frame
0.178	Blockout begins to separate from post #15
0.205	Vehicle rear bumper impacts rail near Post #12
0.268	Vehicle begins traveling parallel with the installation
0.700	Vehicle lost contact with installation traveling at 43.4 mi/h and 10.0 degrees

Table 5.1.	Events	during	Test N	o. 608	421-1.
		~~~ <u>~</u>	100011	0.000	

For longitudinal barriers, it is desirable that the vehicle redirects and exits the barrier within the exit box criteria (not less than 32.8 ft downstream from impact for 1100C and 2270P vehicles). The 2270P vehicle exited within the exit box criteria defined in *MASH*. After loss of contact with the barrier, the vehicle came to rest 175 ft downstream of the impact and 40 ft toward the field side.

## 5.5 DAMAGE TO TEST INSTALLATION

Figure 5.4 shows the damage to the 28-inch W-beam guardrail system with raised composite blockouts. Post 1 was pulled downstream 1.25 inches at ground level, and post 2 was pulled downstream 0.625 inch at ground level. The rail element separated from the shelf bracket at post 1 and released from posts 2 and 3. The rail element was pulled downstream 3.0 inches from post 1 toward impact. Post 11 had gaps between the post and the soil of 0.75 inch on the traffic side and 0.25 inch on the field side, and the post was leaning toward the field side at 88 degrees. Post 12 had gaps between the post and the soil of 2.5 inch on the traffic side and 0.75 inch on the field side, and the post was leaning toward the field side at 88 degrees. The blockouts separated from the posts and rail element from posts 14 through 16, and the rail element released from posts 13 through 17. Post 17 was leaning downstream at 79 degrees and toward the field side 76 degrees. Post 18 had gaps between the post and the soil of 0.125 inch on the traffic side and 0.5 inch on the field side. No movement in the rail or posts was noted beyond post 18. Working width was 69.6 inches at a height of 53.0 inches. Maximum dynamic deflection during the test was 52.6 inches, and maximum permanent deformation was 36.0 inches.



Figure 5.3. Test Vehicle and Guardrail System after Test No. 608421-1.



Figure 5.4. Upstream Terminal after Test No. 608421-1.



Figure 5.5. Posts 11 through 14 after Test No. 608421-1.



Figure 5.6. Posts 15 through 18 after Test No. 608421-1.



Figure 5.7. Field Side of Guardrail and Released Blockouts after Test No. 608421-1.

# 5.6 VEHICLE DAMAGE

Figure 5.8 shows the damage sustained by the vehicle. The front bumper, right front fender, right front upper and lower A-arms, right front upper ball joint, right front tire and rim, sway bar, right front tie rod, right front and rear doors, right rear exterior bed, right rear rim, and rear bumper were damaged. Maximum exterior crush to the vehicle was 10.0 inches in the front

plane at the right front corner at bumper height. No reduction or intrusion of the occupant compartment occurred. Figure 5.9 shows the interior of the vehicle. Tables D.3 and D.4 in Appendix D1 provide exterior crush and occupant compartment measurements.



Figure 5.8. Test Vehicle after Test No. 608421-1.



Figure 5.9. Interior of Test Vehicle for Test No. 608421-1.

# 5.7 OCCUPANT RISK FACTORS

Data from the accelerometer, located at the vehicle center of gravity, were digitized for evaluation of occupant risk and are shown in Table 5.2. Figure 5.10 summarizes these data and other pertinent information from the test. Figure D.3 in Appendix D3 shows the vehicle angular displacements, and Figures D.4 through D.9 in Appendix D4 show accelerations versus time traces.

Occupant Risk Factor	Value	Time
Impact Velocity		
Longitudinal	15.7 ft/s	at 0.1587 s on right side of
Lateral	14.4 ft/s	interior
<b>Ridedown Accelerations</b>		
Longitudinal	5.8 g	0.2811 - 0.2911 s
Lateral	6.5 g	0.2861 - 0.2961 s
тніу	22.0 km/h	at 0.1513 s on right side of
11110	6.1 m/s	interior
PHD	8.1 g	0.2817 - 0.2917 s
ASI	0.57	0.3864 - 0.4364 s
Maximum 50-ms Moving Average		
Longitudinal	-4.5 g	0.0661 - 0.1161 s
Lateral	-4.8 g	0.3589 - 0.4089 s
Vertical	-2.2 g	1.3655 - 1.4155 s
Maximum Roll, Pitch, and Yaw Angles		
Roll	31.6°	0.7862 s
Pitch	-12.3°	0.9569 s
Yaw	-43.7°	0.7853 s

Table 5.2. Occupant Risk Factors for Test No. 608421-1.



**General Information** 

Test Standard Test No	MASH Test 3-11	An
TTI Test No	608421-1	Loc
Test Date	2017-06-30	
Test Article		Impa
Туре	Longitudinal Barrier - Guardrail	Exit
Name	28-inch W-Beam with Raised Blockouts	Sp
Installation Length	175 ft	An
Material or Key Elements	28-inch tall W-beam with structural steel	Occi
-	posts guardrail system with Mondo	Lor
	Polymer Blockouts (Model #GB14SH1)	Lat
	with rail and blockouts raised 4 inches	Lor
Soil Type and Condition	AASHTO M147-65(2004), grading B Soil	Lat
	(crushed limestone), dry	TH
Test Vehicle		PH
Type/Designation	2270P	AS
Make and Model	2011 Dodge RAM 1500 Pickup	Max.
Curb	4882 lb	L
Test Inertial	5017 lb	L
Dummy	No dummy	N
Gross Static	5017 lb	

Test Agency..... Texas A&M Transportation Institute (TTI)

Impact Conditions

Speed	64.1 mi/h
Angle	24.4 degrees
Location/Orientation	12 inches d/s of
	Post 12
Impact Severity	118 kip-ft
Exit Conditions	
Speed	43.4 mi/h
Angle	10.0 degrees
Occupant Risk Values	-
Longitudinal OIV	15.7 ft/s
Lateral OIV	14.4 ft/s
Longitudinal Ridedown	5.8 g
Lateral Ridedown	6.5 g
THIV	22.0 km/h
PHD	8.1 g
ASI	0.57
Max. 0.050-s Average	
Longitudinal	-4.5 g
Lateral	-4.8 g
Vertical	-2.2 g

#### Post-Impact Trajectory

Stopping Distance	. 175 ft downstream
	40 ft twd field side
Vehicle Stability	
Maximum Yaw Angle	. 44 degrees
Maximum Pitch Angle	. 12 degrees
Maximum Roll Angle	. 32 degrees
Vehicle Snagging	. No
Vehicle Pocketing	. No
Test Article Deflections	
Dynamic	. 52.6 inches
Permanent	. 36.0 inches
Working Width	. 69.6 inches
Height of Working Width	. 53.0 inches
Vehicle Damage	
VDS	. 01RFQ4
CDC	. 01FREW4
Max. Exterior Deformation	. 10.0 inches
OCDI	. RF0000000
Max. Occupant Compartment	
Deformation	None

Figure 5.10. Summary of Results for MASH Test 3-11 on 28-inch W-Beam Guardrail System with Raised Composite **Blockouts.** 

# Chapter 6. SUMMARY AND CONCLUSIONS

#### 6.1. ASSESSMENT OF TEST RESULTS

An assessment of the test based on the applicable safety evaluation criteria for *MASH* Test 3-11 is provided in Table 6.1.

#### 6.2 CONCLUSIONS

The 28-inch W-beam guardrail system with raised composite blockouts contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 52.6 inches. Detached blockouts did not penetrate or show potential for penetrating the occupant compartment, or present undue hazard to others in the area. No reduction or intrusion of the occupant compartment occurred. The 2270P vehicle remained upright during and after the collision period. Maximum roll and pitch angles were 32 degrees and 12 degrees, respectively. Occupant risk factors were within the preferred limits specified in *MASH*.

The 28-inch W-beam guardrail system with composite blockouts raised 4 inches performed acceptably for *MASH* Test 3-11.

#### 6.3 IMPLEMENTATION*

For the test conducted in this study, a guardrail height of 28 inches was chosen, and rail splices were positioned on posts. These selections represent the worst case condition for testing. Taller rail heights, offset rail splices, and raising of the blockout less than 4 inches are considered acceptable based on the results of this more critical test. The practice can be used to raise the height of a deficient guardrail to an acceptable height (i.e., 28 inches or greater), or could be used to raise the height of existing guardrail to improve performance (e.g., 31-inch rail height).

^{*} The opinions/interpretations expressed in this section are outside the scope of TTI Proving Ground's A2LA Accreditation.

# Table 6.1. Performance Evaluation Summary for MASH Test 3-11 on 28-inch W-Beam Guardrail System with Raised<br/>Composite Blockouts.

Tes	t Agency: Texas A&M Transportation Institute	Test No.: 608421-1	Test Date: 2017-06-30
	MASH Test 3-11 Evaluation Criteria	Test Results	Assessment
Str A.	<b>uctural Adequacy</b> Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.	The 28-inch W-beam guardrail system with raised composite blockouts contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 52.6 inches.	Pass
<u>Oc</u> D.	<u>cupant Risk</u> Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone.	Detached blockouts did not penetrate or show potential for penetrating the occupant compartment, or present undue hazard to others in the area.	Pass
	Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.2.2 and Appendix E of MASH.	No reduction or intrusion of the occupant compartment occurred.	
<i>F</i> .	The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.	The 2270P vehicle remained upright during and after the collision period. Maximum roll and pitch angles were 32 degrees and 12 degrees, respectively.	Pass
<i>H</i> .	Occupant impact velocities (OIV) should satisfy the following limits: Preferred value of 30 ft/s, or maximum allowable value of 40 ft/s.	Longitudinal OIV was 15.7 ft/s, and lateral OIV was 14.4 ft/s.	Pass
Ι.	The occupant ridedown accelerations should satisfy the following limits: Preferred value of 15.0 g, or maximum allowable value of 20.49 g.	Maximum longitudinal ridedown acceleration was 5.8 g, and maximum lateral ridedown acceleration was 6.5 g.	Pass

# REFERENCES

- 1. Roadside Design: Steel Strong-Post W-beam Guardrail, May 17, 2010, Memorandum, Office of Safety Design, Federal Highway Administration, U.S. Department of Transportation.
- H.E. Ross, Jr., D.L. Sicking, R.A. Zimmer and J.D. Michie, *Recommended Procedures* for the Safety Performance Evaluation of Highway Features, National Cooperative Highway Research Program Report 350, Transportation Research Board, National Research Council, Washington, D.C., 1993.
- 3. AASHTO. *Manual for Assessing Roadside Safety Hardware, Second Edition.* 2016, American Association of State Highway and Transportation Officials: Washington, D.C.
- C. Silvestri Dobrovolny, W. L. Menges, and D. L. Kuhn. *Pendulum Testing on Composite Blockouts Raised on Steel Posts*. Technical Memo 605311, Texas A&M Transportation Institute, College Station, TX, February 15, 2017.



APPENDIX A. SYSTEM WITH RAISED COMPOSITE BLOCKOUTS **DETAILS OF THE 28-INCH W-BEAM GUARDRAIL** 

TR No. 608421-1

25

2017-09-05



TR No. 608421-1

26

2017-09-05

# **APPENDIX B. SUPPORTING CERTIFICATION DOCUMENTS**

Mondo Polymer Technologies, Inc. P.O. Box 250 / 27620 State Rt. 7 North Reno, OH 45773 Phone: (888) 607-4790 Plastics from Today for Tomorrow.....

# **Material Specification**

<b>Product ID:</b>	GB14SH2
Description:	Composite Recycled Guardrail Block 14" x 8" x 5 1/8" for Steel Post w/hanger (see attached drawing for dimensions and tolerances)
Lot #:	16-04-22-1
Composition: ¹	≥ 85% Thermoplastic Polyolefins ≤ 13% Fillers and/or Trace Plastics Minimum of 2 % UV Stabilizers
Density:	$0.90 - 0.98 \text{ g/cm}^3$
Specific Gravity:	<1
Hardness:	Shore D 45-70
Melt Temperature	e: $\geq 244^{\circ} \text{ F} (118^{\circ} \text{ C})$
Water Absorption	a: <0.01

¹ Manufactured from no less than 75% recycled content material





I abit C.2. I tot Day Static Son Stichgin Documentation for I tot 100 000721-		Table C.2.	<b>Test Day</b>	v Static Se	oil Strength	Documentation	for Test No	<b>608421-1</b>
-------------------------------------------------------------------------------	--	------------	-----------------	-------------	--------------	---------------	-------------	-----------------

Date	2017-06-30
Test Facility and Site Location	TTI Proving Ground – 3100 SH 47, Bryan, Tx
In Situ Soil Description (ASTM D2487)	Sandy gravel with silty fines
Fill Material Description (ASTM D2487) and sieve analysis	AASHTO Grade B Soil-Aggregate (see sieve analysis)
Description of Fill Placement Procedure	6-inch lifts tamped with a pneumatic compactor

# APPENIDX D. MASH TEST 3-11 (CRASH TEST NO. 608421-1)

#### D1 VEHICLE PROPERTIES AND INFORMATION

	16	inte .	D.1. Venic	le riope			0421-1.		
Date: 201	7-06-30		Test No.:	608421-	1	VIN No.:	1D7RB16P7	BS5473	371
Year: 201	1		Make:	Dodge		Model:	RAM 1500		
Tire Size:	265/70	R17			Tire I	nflation Pre	ssure: <u>35 psi</u>		
Tread Type:	Highwa	ay				Odo	meter: <u>14689</u>	8	
Note any dar	mage to th	ne vel	hicle prior to	est: <u>N</u>	one				
<ul> <li>Denotes a</li> </ul>	ccelerom	eter lo	ocation.		-	◀───X─ ◀──₩─━┥	•		
NOTES: N	lone			1		717			
				_     A M					
Engine Type Engine CID:	: <u>V-8</u> 4.7	liter			ACK				WHEEL TRACK
Transmissior <u>x</u> Auto FWD	n Type: or x R	WD	_ Manual 4WD	·	R			TIAL C. M.	
Optional Equ None	ipment:							2	BB
Dummy Data Type: Mass: Seat Positio	a: <u>No o</u> NA on: NA	dumm	Ŋ	j J-j J				))- - D-	
Geometry:	inches				Ĭ	FRONT	Ĭ	REAR	
A 78.5	0	F	40.00	К	20.75	Р	3.00	U	27.00
B 74.0	0	G	28.375	 	30.00	Q	30.50	V	30.00
C 227.5	0	Н	62.50	M	68.50	R	18.00	W	62.50
D 47.0	0	Ι	11.50	N	68.00	S	13.50	Х	77.75
E 140.5	0	J	27.00	0	45.50	T	77.00	_	
Wheel Ce Height F	nter ront		14.75 Cle	Wheel Wearance (Fror	ell nt)	6.00	Bottom Frame Height - Front		17.00
Wheel Ce Height F	nter Rear		14.75 Cle	Wheel W earance (Rea	ell ar)	9.25	Bottom Frame Height - Rear		25.50
GVWR Ratin	us.		Mass: Ib	C	urb	Test I	nertial	Gros	s Static
Front	3700		Mfront	<u> </u>	2855	<u> </u>	2785	0.00	
Back	3900	-	M _{rear}		2027		2232		
Total	6700	-	MTotal		4882		5017		
Mass Dietrik	(Allowable Range for TIM and GSM = 5000 lb ±110 lb)								
lb		LF:	1391	RF:	1394	LR:	<u>1114</u> R	R:	1118

# Table D.1. Vehicle Properties for Test No. 608421-1

Date: 2017-06-30 Test No.: 608421-1 VIN: 1D7RB16P7BS547371								
Year: 2011		Make:	Dodge		Model:	RAM 150	0	
Body Style:	Quad Cab				Mileage:	146898		
Engine: 4.7	liter V-8			Tran	smission:	Automatio	2	
	Empty	Bal	llast:	207	lh		(4	10 lb max)
	Empty	Dai	iidət	201			(4	+0 ID 111aX)
Tire Pressure:	Front:	<u>35</u> ps	si Re	ar: <u>35</u>	_psi _	Size: <u>265/</u>	70R17	
Measured Ve	hicle Wei	ghts: (I	b)					
LF:	1391		RF:	1394		Front Ax	de: 2785	
L D.	1111			1110		Deer Av	day 0000	
	1114		KK.	1110		RearAx	ae. 2232	
Left:	2505		Right:	2512		Tot	al: 5017	
						5000	±110 lb allow ed	
Wh	eel Base:	140.5	inches	Track: F:	68.5	inches	R: 68	inches
	148 ±12 inch	es allow ed			Track = (F+R	.)/2 = 67 ±1.5 in	ches allow ed	
Contor of Cra			noncion N	Acthor d				
Center of Gra	ivily, SAE	Jo14 Sus	spension i	hethod				
X:	62.51	inches	Rear of F	ront Axle	(63 ±4 inche	s allow ed)		
Y:	0.05	inches	Left -	Right +	of Vehicle	Centerline		
Ζ:	28.375	inches	Above Gr	ound	(minumum 28	3.0 inches allow	red)	
Hood Heig	ght:	45.50	inches	Front	Bumper H	leight:	27.00	inches
	43 ±4	inches allowed	d					
Front Overha	na:	40.00	inches	Rear	Bumper H	leiaht:	30.00	inches
39 ±3 inches allowed								
		007 50						
Overall Lenç	gin:	227.50						
	237 ±1	I 3 Inches allow	ved					

# Table D.2. Measurements of Vehicle Vertical CG for Test No. 608421-1.

Date:	2017-06-30	Test No.:	608421-1	VIN No.:	1D7RB16P7BS547371
Year:	2011	Make:	Dodae	Model:	RAM 1500

# Table D.3. Exterior Crush Measurements for Test No. 608421-1.

## VEHICLE CRUSH MEASUREMENT SHEET¹

Complete When Applicable							
End Damage	Side Damage						
Undeformed end width	Bowing: B1 X1						
Corner shift: A1	B2 X2						
A2							
End shift at frame (CDC)	Bowing constant						
(check one)	X1+X2 _						
< 4 inches	2						
$\geq$ 4 inches							

#### Note: Measure C₁ to C₆ from Driver to Passenger Side in Front or Rear impacts – Rear to Front in Side Impacts.

		Direct Damage									
Specific Impact Number	Plane* of C-Measurements	Width** (CDC)	Max*** Crush	Field L**	$C_1$	C ₂	C ₃	C ₄	C ₅	C ₆	±D
1	Front plane at bumper ht	20	10	28	0.5	1	1.5	3	6	10	-21
2	Side plane at bumper ht	20	9	60	1	1			9	9	+67
	Measurements recorded										
	in inches										

¹Table taken from National Accident Sampling System (NASS).

*Identify the plane at which the C-measurements are taken (e.g., at bumper, above bumper, at sill, above sill, at beltline, etc.) or label adjustments (e.g., free space).

Free space value is defined as the distance between the baseline and the original body contour taken at the individual C locations. This may include the following: bumper lead, bumper taper, side protrusion, side taper, etc. Record the value for each C-measurement and maximum crush.

**Measure and document on the vehicle diagram the beginning or end of the direct damage width and field L (e.g., side damage with respect to undamaged axle).

***Measure and document on the vehicle diagram the location of the maximum crush.

Note: Use as many lines/columns as necessary to describe each damage profile.











*Lateral area across the cab from driver's side kick panel to passenger's side kick panel.

## OCCUPANT COMPARTMENT DEFORMATION MEASUREMENT

	Before	After (inches)	Differ.
A1	65.00	65.00	0
A2	62.75	62.75	0
A3	65.50	65.50	0
B1	44.75	44.75	0
B2	37.75	37.75	0
B3	44.75	44.75	0
B4	39.25	39.25	0
B5	43.25	43.25	0
B6	39.25	39.25	0
C1	29.00	29.00	0
C2			0
C3	26.25	26.25	0
D1	11.25	11.25	0
D2			0
D3	11.25	11.25	0
E1	58.50	58.50	0
E2	63.50	63.75	+0.25
E3	63.50	63.50	0
E4	63.25	63.25	0
F	59.00	59.00	0
G	59.00	59.00	0
Н	38.00	38.00	0
I	38.00	38.00	0
J*	23.25	23.25	0





Figure D.1. Sequential Photographs for Test No. 608421-1 (Overhead and Frontal Views).





Figure D.1. Sequential Photographs for Test No. 608421-1 (Overhead and Frontal Views) (Continued).





0.400 s



0.500 s



0.600 s



0.300 s

Figure D.2. Sequential Photographs for Test No. 608421-1 (Rear View).



0.200 s



Figure D.3. Vehicle Angular Displacements for Test No. 608421-1.

Yaw.

Pitch.

1. 2.

3. Roll.

TR No. 608421-1





**D**4

VEHICLE ACCELERATIONS

#### Figure D.4. Vehicle Longitudinal Accelerometer Trace for Test No. 608421-1 (Accelerometer Located at Center of Gravity).



#### Figure D.5. Vehicle Lateral Accelerometer Trace for Test No. 608421-1 (Accelerometer Located at Center of Gravity).



#### Figure D.6. Vehicle Vertical Accelerometer Trace for Test No. 608421-1 (Accelerometer Located at Center of Gravity).



#### Figure D.7. Vehicle Longitudinal Accelerometer Trace for Test No. 608421-1 (Accelerometer Located Rear of Center of Gravity).



Figure D.8. Vehicle Lateral Accelerometer Trace for Test No. 608421-1 (Accelerometer Located Rear of Center of Gravity).



#### Figure D.9. Vehicle Vertical Accelerometer Trace for Test No. 608421-1 (Accelerometer Located Rear of Center of Gravity).