



# Roadside Safety Pooled Fund Program Research Update

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**Roadside Safety Pooled Fund Program – Fall Meeting 2018**

Denver, Colorado

September 18<sup>th</sup> 2018



# On-going Projects

- **Engineering Support Services and Recommendations for Roadside Safety Issues/Problems for Member States**
- **MASH TL3 T-Intersection (Short Radius) System Design Variations**
- **MASH Testing of W-beam Guardrail in Concrete Mow-Strip**
- **Testing of Midwest Guardrail Systems with Reduced Post Spacing for MASH Compliance**
- **Testing and Evaluation of MGS System w/ Critical Flare at MASH TL3 Conditions**
- **Thrie/W-Beam/Tubular Barrier Gap Rail for MASH TL-3**
- **Placement of Guardrail on Slopes Phase IV: MASH TL-3 Testing of Guardrail on 1:1 Slope**
- **Accommodating Inlets with Transitions (TL-3)**



# **Engineering Support Services and Recommendations for Roadside Safety Issues/Problems for Member States**

**TTI Researcher: William Williams**

**Technical Representative: James Danila (MassDOT)**

# Engineering Support Services and Recommendations for Roadside Safety Issues/Problems for Member States

- Problem

- There is a need for an assessment of roadside safety barrier systems and hardware without necessarily performing full-scale crash testing.
- The objective of this research is to provide engineering support services and recommendations for those roadside safety barrier hardware and barrier systems that are prioritized/requested by pooled fund member states



# Engineering Support Services and Recommendations for Roadside Safety Issues/Problems for Member States

- Work Plan
  - Task 1 – Gathering of Information
    - Prioritize projects/needs from pooled states
    - Conduct literature review and review past crash testing
    - Collaborate with other testing houses on
  - Task 2 – Evaluation and Assessment
    - Perform engineering calculations as necessary
    - Use recent research, NCHRP 22-07, recent MASH crash testing
    - Provide assessment of the barrier/hardware system
      - Use FHWA Eligibility Form Approach
      - Provide justification why certain crash tests do not need to be performed
      - ... or provide justification that warrants crash testing

# Engineering Support Services and Recommendations for Roadside Safety Issues/Problems for Member States

- Current Priority Work List:

- 1.) MGS median barrier (TL-3) – 12" Blockouts or no blockouts? Further clarification needed.
- 2.) Does 32" F-shape CIP barrier Meet MASHTL-3? – Currently working on this task. We have this near complete and MASH compliant.
- 3.) 18'9" Thrie Beam Transition (TL-3) – Currently working on this task. We have collected information from MwRSF and others.
- 4.) Using a transition from guardrail to concrete shape different than crash tested – Vertical to sloped faced connection (?)
- 5.) Michigan temporary concrete barrier limited deflection – Further definition on barrier design needed
- 6.) Concrete shape transitions (transitioning from different shapes) – currently gathering information on this task.



# **MASH TL3 T-Intersection (Short Radius) System Design Variations**

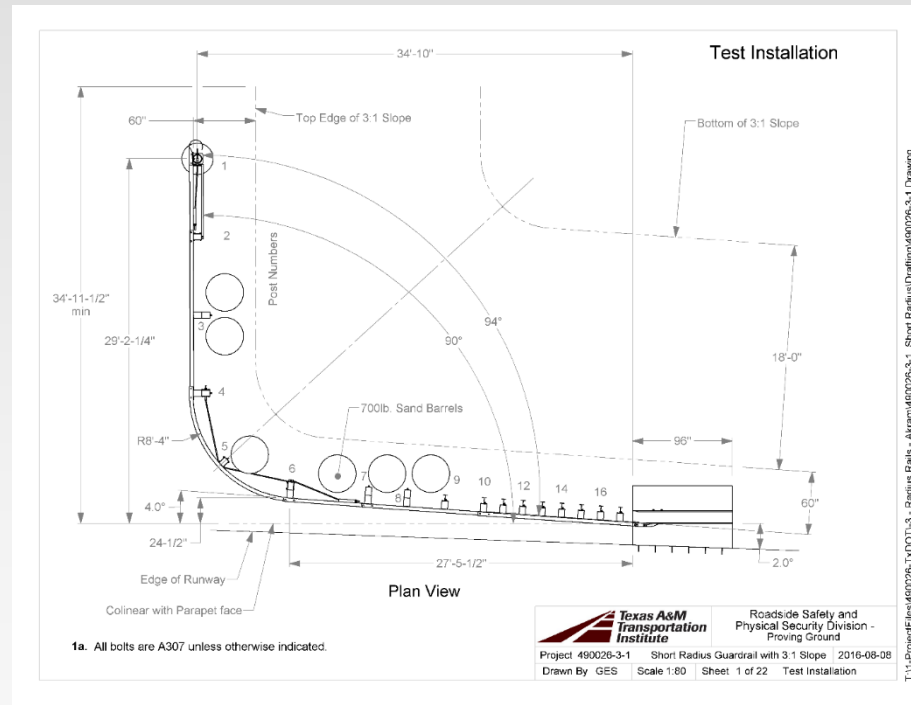
**TTI Researcher: Akram Abu-Odeh**

**Technical Representative: Christopher Lindsey (TxDOT)**

# T-intersection Design Variations

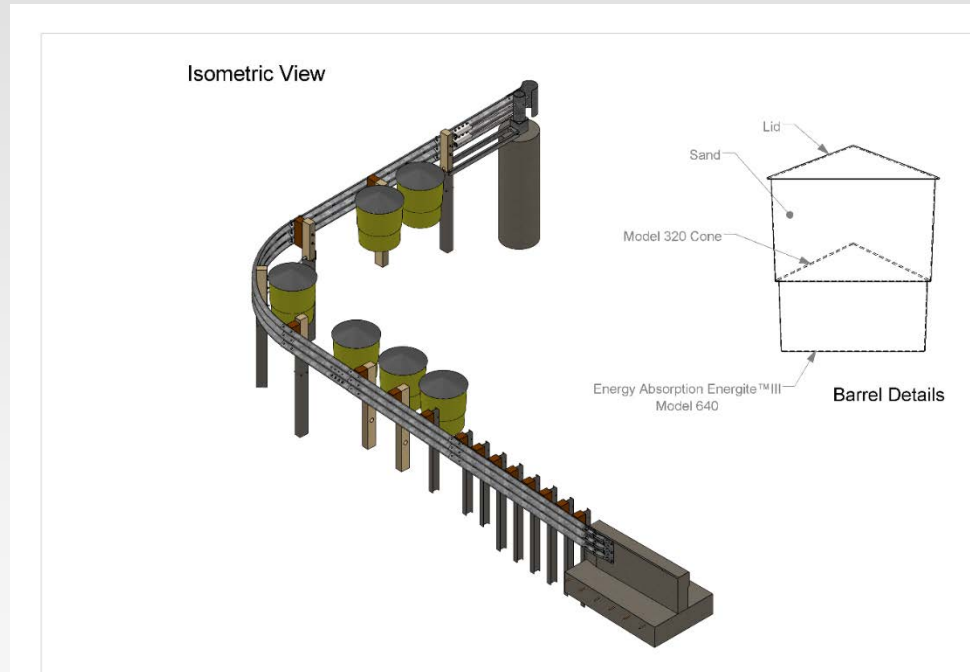
- Use design elements from successful studies (TxDOT, TL-3 and TL-2)
  - Keep small foot print short radius with 3TO1 ditch
  - Goal for MASH TL-3
  - Use drums instead of barrels (still 700 lb sand/unit).
  - Account for secondary roadway.

# TxDOT TL-3 System



- MASH TL-3
- All thrie-beam rail, Nose 8 ft 4 in radius
- Six 700 lb sand barrels

# TxDOT TL-3 System (cont'd)



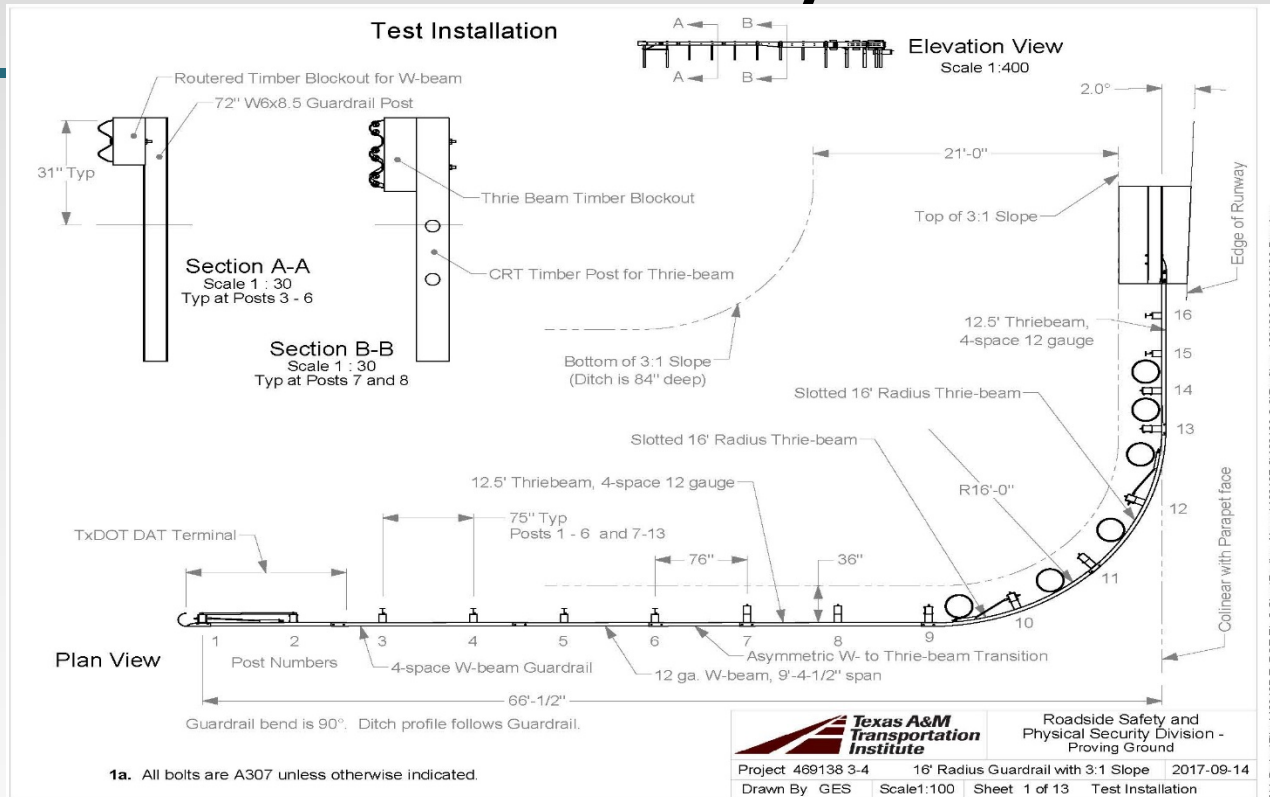
- Secondary driveway
- Rotating driveway anchor
- Five feet platform before the sloped ditch (3:1)



# TxDOT TL-3 System (cont'd)



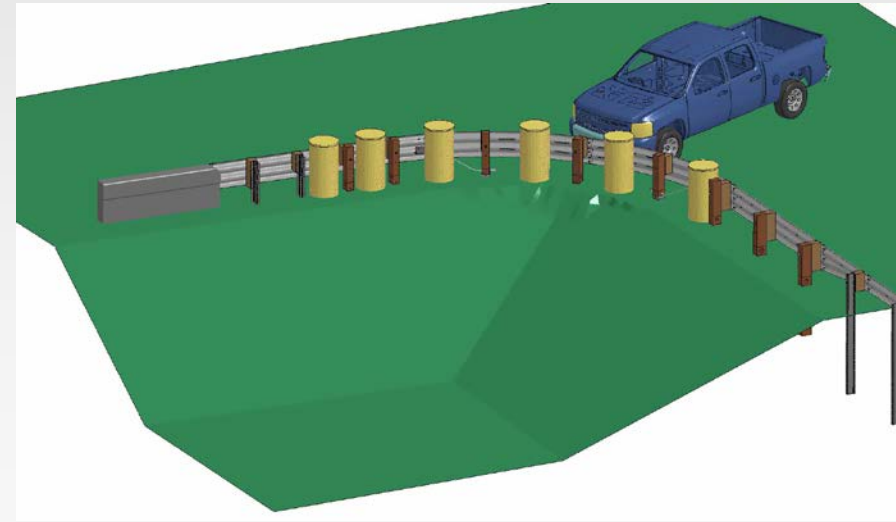
# TxDOT TL-2 System



- MASH TL-2
- All thrie-beam rail on primary and then transition to W-beam on secondary roadway
- Six 700 lb sand drums



# TxDOT TL-2 System (cont'd)



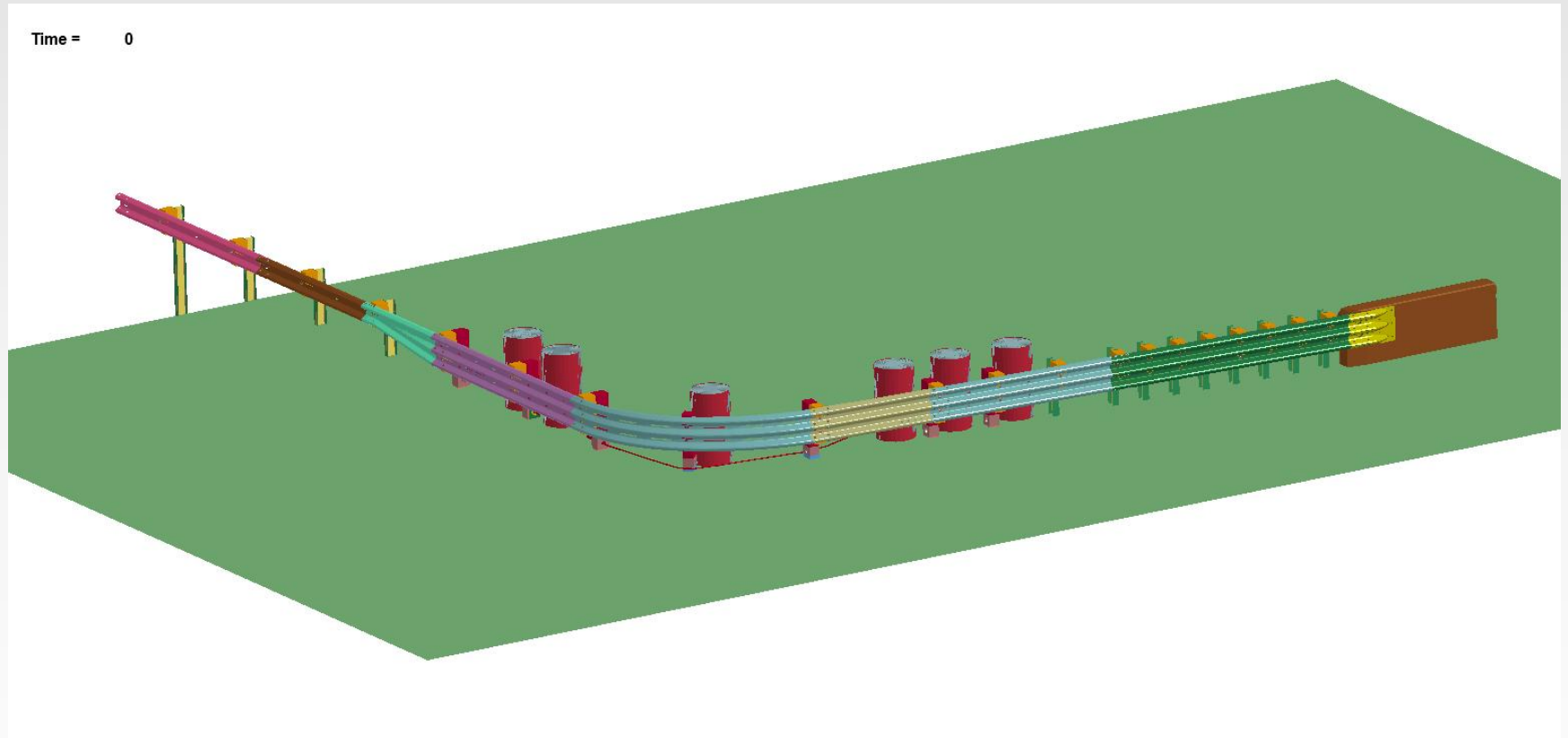
- Nose 16 ft radius
- Three feet of platform then sloped ditch (3:1)
- Terminal on secondary roadway

13

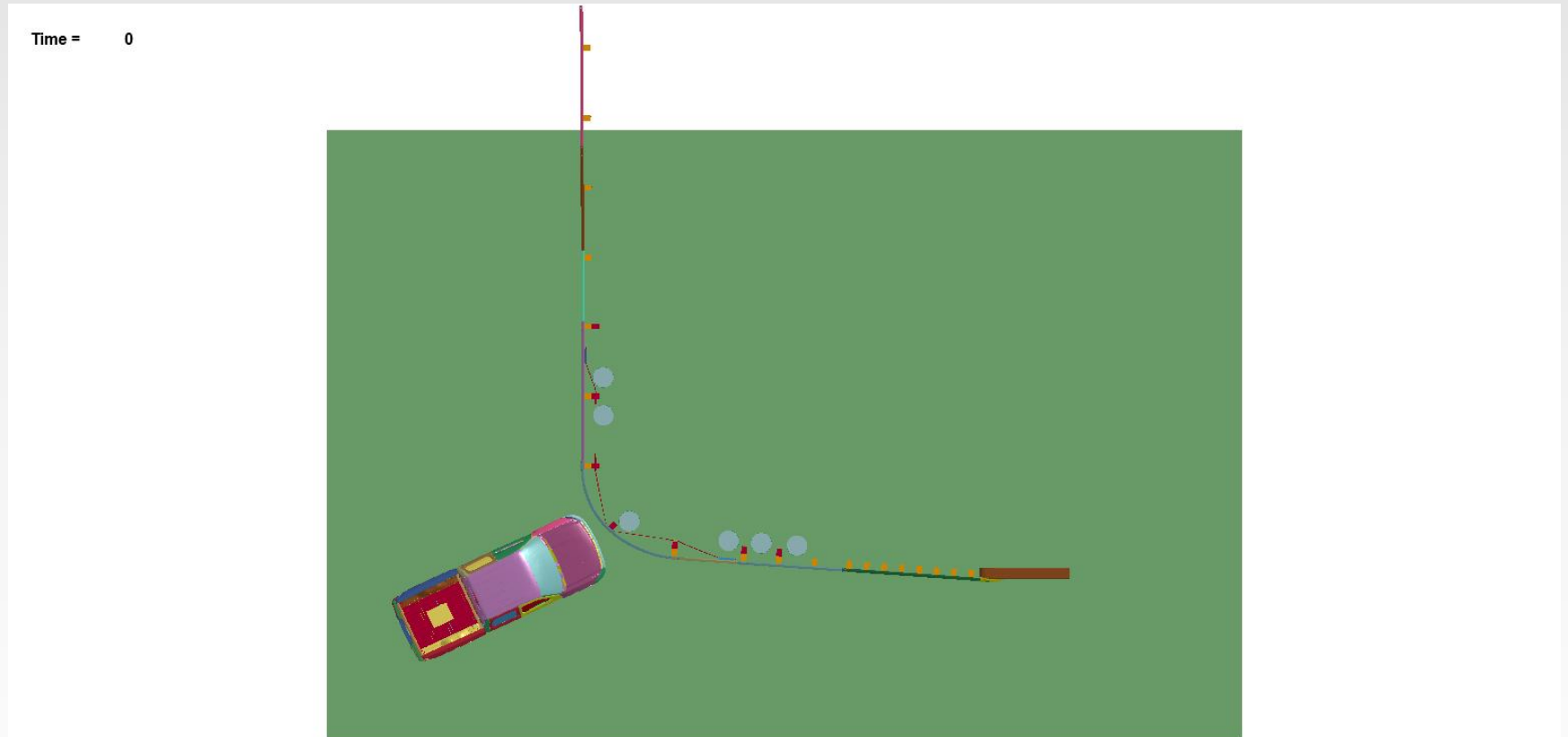
# Pool Fund System

- MASH TL-3
- 8-ft 4-in nose radius
- Sand drums
- Three-foot platform
- Secondary roadway
- Thrie-beam on primary roadway becoming w-beam on secondary roadway using an asymmetric transition rail element

# Pool Fund System (cont'd)

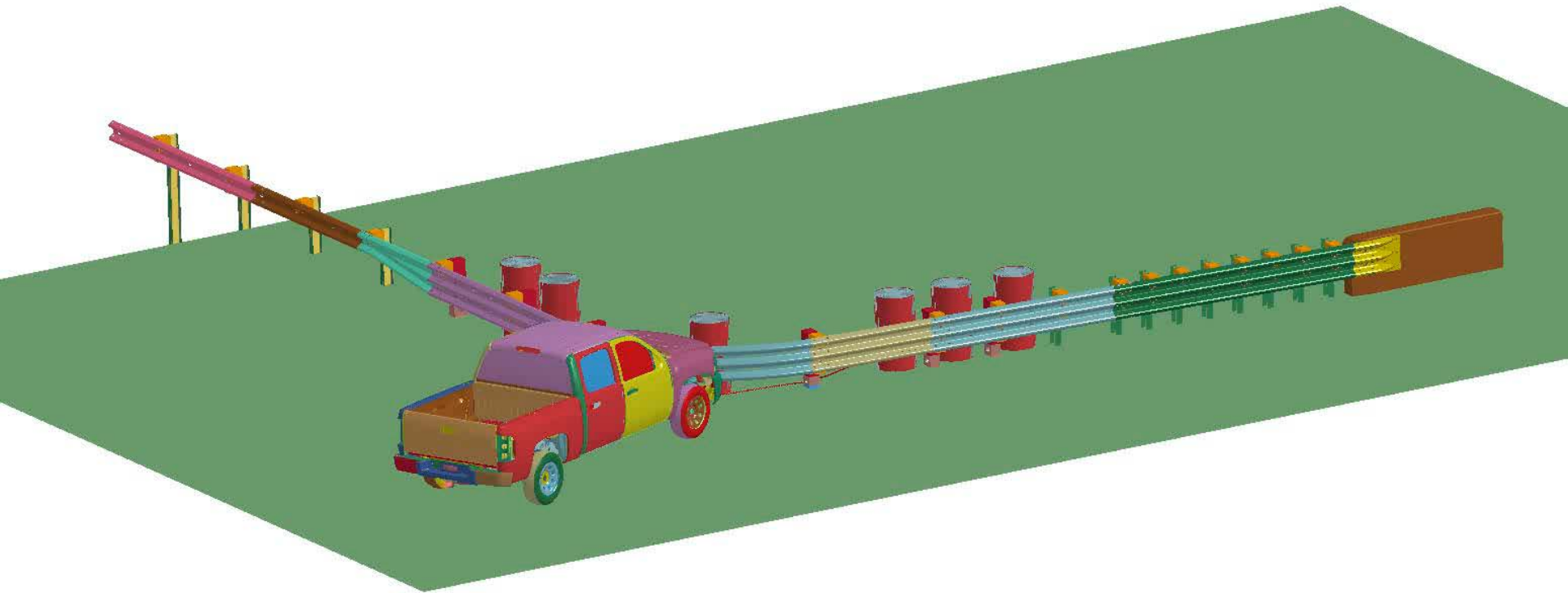


# Pool Fund System (cont'd)



# Pool Fund System (cont'd)

Time = 0



# Upcoming Activities

- Simulating different ditch conditions
  - 3TO1 ditch (underway)
  - 2TO1 ditch if feasible



# **MASH Testing of W-beam Guardrail in Concrete Mow-Strip**

**TTI Researcher: Nauman Sheikh**

**Technical Representative: Michael Elle (MNDOT)**

# Guardrail in Concrete Mow-Strip

## Objective:

Determine MASH TL-3 compliance of the W-beam guardrail installed in concrete mow-strip

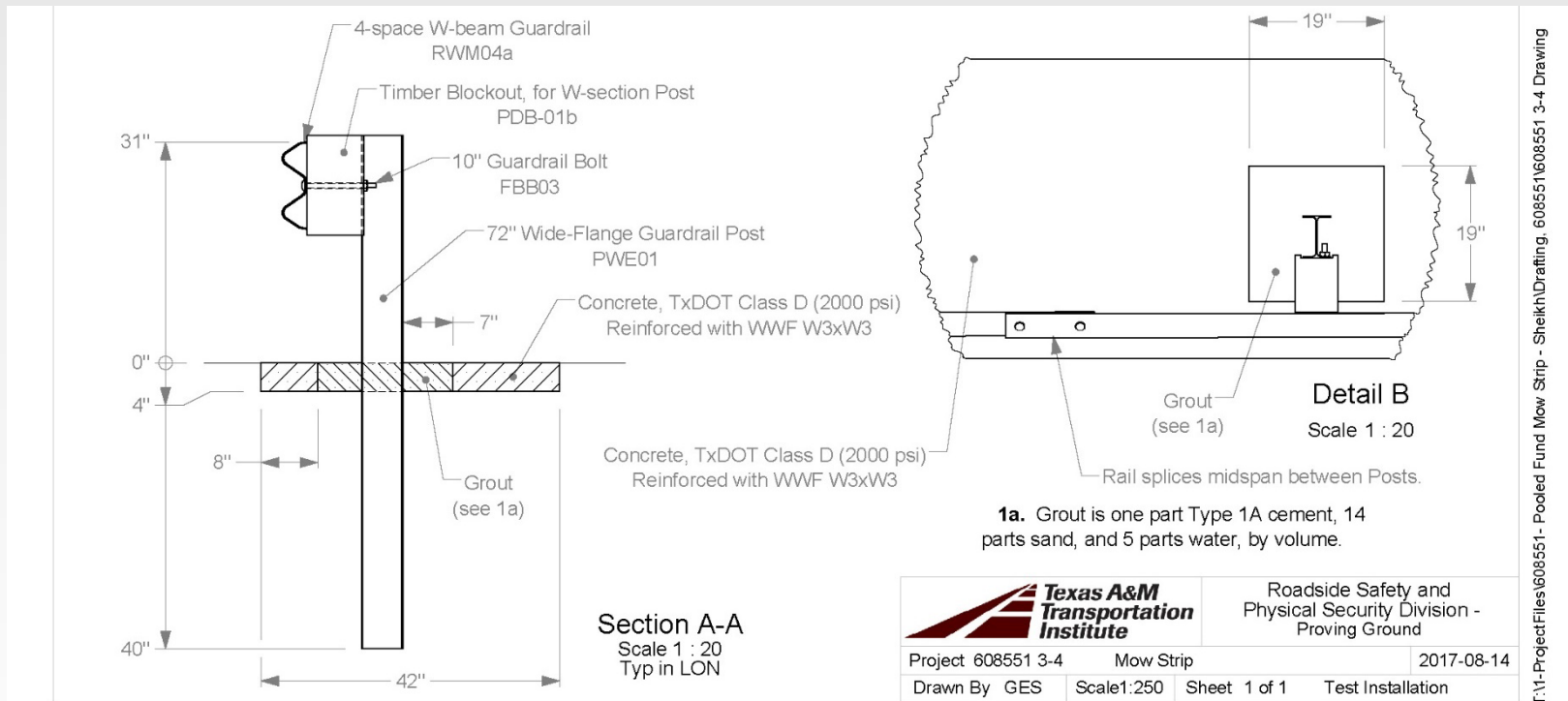
## Scope:

- Steel post guardrail
  - Test 3-10 and Test 3-11
- Wood post guardrail
  - Test 3-10 and Test 3-11



# Guardrail in Concrete Mow-Strip

## Test Installation (Steel Post):



# Guardrail in Concrete Mow-Strip

## Test Installation (Steel Post):

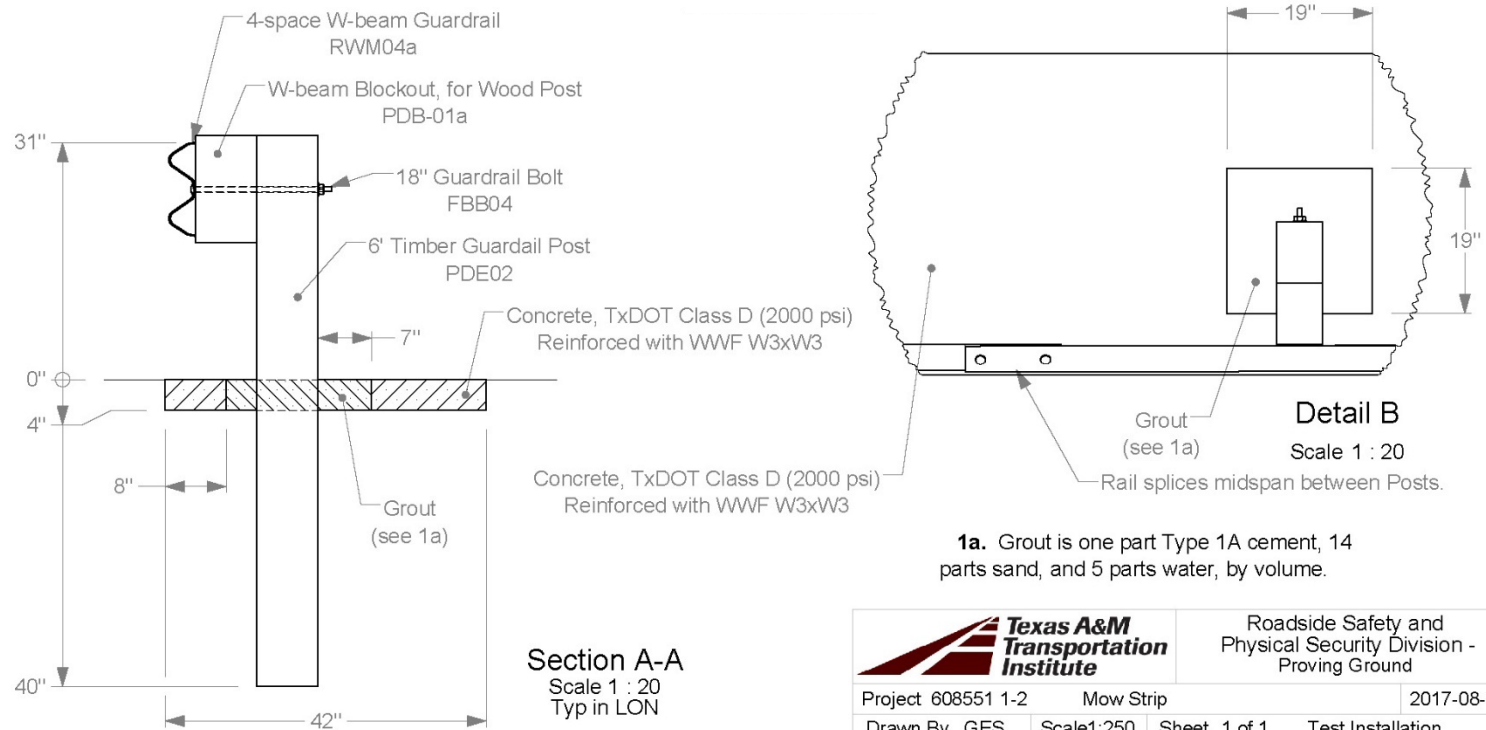
181'-3" installation length (including terminals)

100' concrete mow-strip in the center



# Guardrail in Concrete Mow-Strip

## Test Installation (Wood Post):



T:\1-ProjectFiles\608551- Pooled Fund Mow Strip - Sheikh\Drafting, 608551\608551 1-2 Drawing



Roadside Safety and  
Physical Security Division -  
Proving Ground

Project 608551 1-2	Mow Strip	2017-08-14
Drawn By GES	Scale: 1:250	Sheet 1 of 1
Test Installation		



# Guardrail in Concrete Mow-Strip

## Test Installation (Wood Post):

Same installation and mow strip length as steel

### Low-strength grout properties

- Specified as: 1 part Type 1A cement, 14 parts sand, and 5 parts water, by volume
- Achieves maximum strength approximately ranging from 120 psi to 200 psi



# Guardrail in Concrete Mow-Strip

Steel Post (Test 3-10):



# Guardrail in Concrete Mow-Strip

## Steel Post (Test 3-11):





# Guardrail in Concrete Mow-Strip

## Results (Steel Post)

**Max. deflection:**

**3-10:** 27.4 dynamic / 17.0 perm.

**3-11:** 50.8 dynamic / 21.0 perm.

**Both tests passed MASH**



# Guardrail in Concrete Mow-Strip

Wood Post (Test 3-10):





# Guardrail in Concrete Mow-Strip

Wood Post (Test 3-11):



# Guardrail in Concrete Mow-Strip

## Results (Wood Post)

Max. deflection:

3-10: 27.4 dynamic / 17.0 perm.

Test 3-10 passed but 3-11 failed  
MASH



# Guardrail in Concrete Mow-Strip

- Conclusion
  - The steel post system successfully passed MASH
  - The wood post system failed to pass MASH due to failure with pickup truck test
  - Current Status:
    - Additional Test 3-11 with wood post system is scheduled with reduced post embedment depth of 36 inches
      - Reducing post embedment may prevent sudden failure of wood posts, which may lead to a successful design





# Testing of Midwest Guardrail Systems with Reduced Post Spacing for MASH Compliance

**TTI Researcher: James Kovar**

**Technical Representative: Joe Hall (WV DOT)**

# Research Need

- Joint implementation agreement between AASHTO and FHWA requiring the use of MASH compliant hardware
- Reduced post spacing guardrail systems used when lower deflections are required
- Pooled Fund prioritized this project in the last meeting

# Research Plan

- Three systems
  - Quarter Post Spacing (18 3/4-inches)
  - Half Post Spacing (37 1/2-inches)
  - Transition between Full (75-inches) and Quarter Post Spacing
- Otherwise, typical MGS system with 31-inch tall w-beam guardrail and W6x9 posts

# Research Plan

- Four Tests
  - Quarter Post Spacing with 3-10
  - Quarter Post Spacing with 3-11
  - Half Post Spacing with 3-11
    - 3-11 viewed as critical test because snagging and occupant risk danger is much higher with quarter post spacing 3-10 test
  - Transition between Full and Quarter Post Spacing with 3-21
    - 3-21 viewed as critical test because snagging and occupant risk danger is much higher with quarter post spacing 3-20 test



# Research Status

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- First Installation is constructed (will be repaired after each test)
- Expecting all four tests to be completed by end of October
- Expecting report to be issued January 2019





# **Testing and Evaluation of the MGS System with Critical Flare at MASH Test Level 3 Conditions**

**TTI Researcher: Chiara S. Dobrovolny**  
**Technical Representative: Jeff Jeffers (AKDOT)**

# MASH TL-3 Testing of the MGS w/ Critical Flare

- **Previous Work Performed**

- MwRSF evaluated MGS flare rate of 13:1, 7:1 and 5:1 according to NCHRP Report 350 TL-3.

- **Issue**

- Test and Evaluate the MGS with critical flare rate at MASH TL-3 conditions.

- **Solution and Work Proposed**

- Use engineering analysis and computer simulation to select critical flare rate for full-scale crash tests.
- Conduct full-scale crash tests according to MASH TL-3 criteria.

# Testing and Evaluation of the MGS System with Critical Flare at MASH Test Level 3 Conditions

## Impact Severity

Test Objective	Vehicle Type	Test Criteria	Angle	Velocity (mph)	Impact Severity (kips-ft)	Difference
Straight MGS	Small Car	NCHRP 350	20	62	27	105%
		MASH	25	62	56	
	Pickup Truck	NCHRP 350	25	62	101	14%
		MASH	25	62	115	
13:1 Flare	Small Car	NCHRP 350	24.4	62	40	89%
		MASH	29.4	62	75	
	Pickup Truck	NCHRP 350	29.4	62	137	14%
		MASH	29.4	62	155	

# Testing and Evaluation of the MGS System with Critical Flare at MASH Test Level 3 Conditions

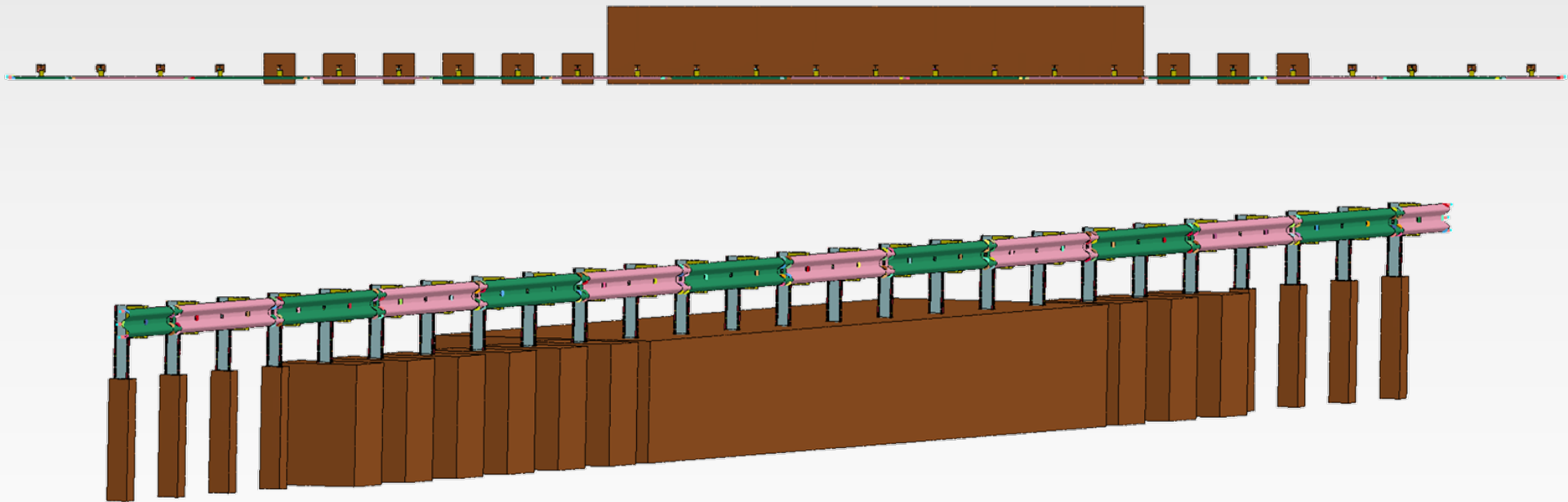
## Impact Severity

Test Objective	Vehicle Type	Test Criteria	Angle	Velocity (mph)	Impact Severity (kips-ft)	Difference
7:1 Flare	Small Car	NCHRP 350	28.13	62	52	80%
		MASH	33.13	62	93	
	Pickup Truck	NCHRP 350	33.13	62	169	14%
		MASH	33.13	62	192	
5:1 Flare	Small Car	NCHRP 350	31.31	62	63	74%
		MASH	36.31	62	109	
	Pickup Truck	NCHRP 350	36.31	62	199	14%
		MASH	36.31	62	225	

# MASH TL-3 Testing of the MGS w/ Critical Flare

## FE model validation (on-going)

- Straight MGS system model with soil



# MASH TL-3 Testing of the MGS w/ Critical Flare

## FE model validation (3-11)

- Compare with MwRSF Update to NCHRP 350 crash tests

T=0.00s



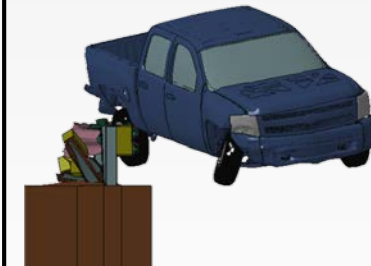
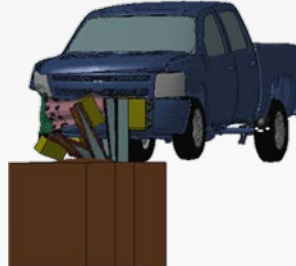
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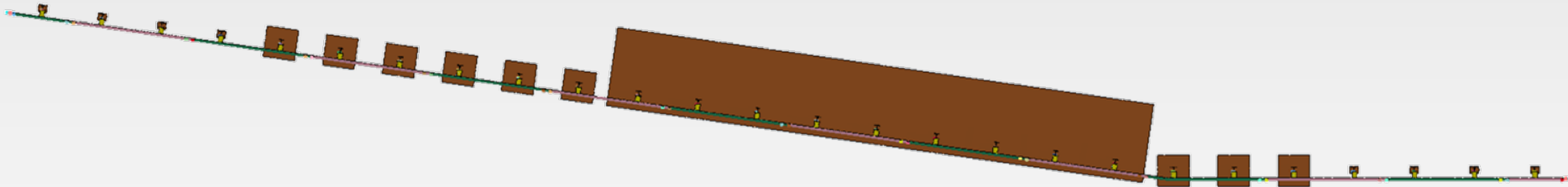
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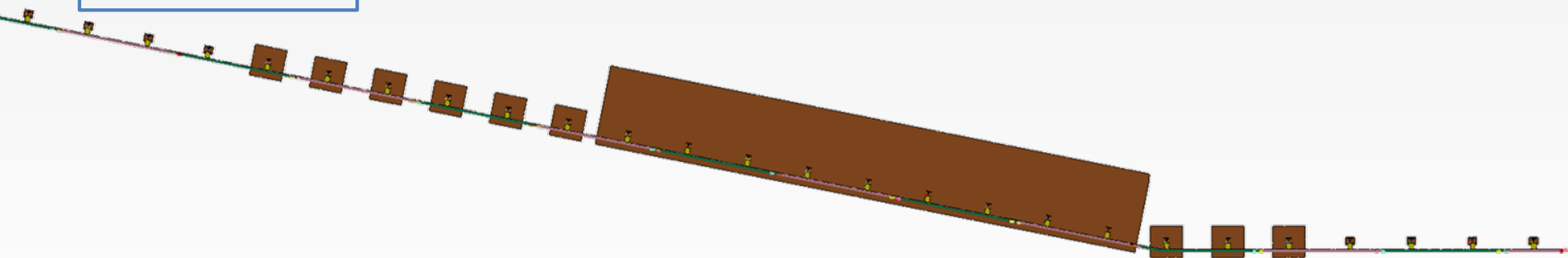
# MASH TL-3 Testing of the MGS w/ Critical Flare

## FE model of selected flared MGS (3-11)

1:7 flare rate



1:5 flare rate







# MASH TL-3 Testing of the MGS w/ Critical Flare

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- **Current Status**

- Calibrate the FE models based on real crash tests
- Determining the impact point for small car and pickup truck tests
- Conducting simulations according to MASH test 3-11 and 3-10



# **Thrie/W-Beam/Tubular Barrier Gap Rail for MASH TL-3**

**TTI Researcher: William Williams**

**Technical Representative: Mike Elle (MNDOT)**

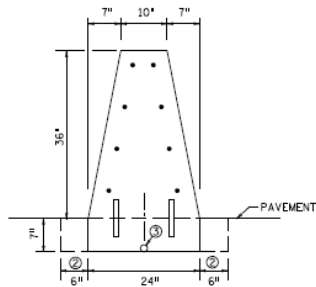
# Thrie/W-Beam/Tubular Barrier Gap Rail for MASH TL-3

- Problem
  - Sometimes manholes and other features in the alignment of barriers
  - Need to provide 8-foot maximum wide gap to access manhole/features
  - Need to provide structural barrier that is removable for access
  - Removable barrier needs to meet crash requirements of MASH TL-3

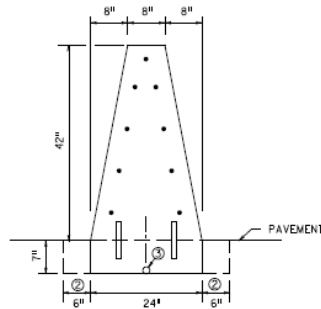
# Thrie/W-Beam/Tubular Barrier Gap Rail for MASH TL-3

- Work Plan
  - Task 1 – Engineering Analyses & Detailing
    - Option 1 – W-beams with brace frames
    - Option 2 – Steel Tubes with Slotted Plates
  - Task 2 – Construction & Drafting
  - Task 3 – Perform Full Scale Crash Test
    - Perform Mash Test 3-10 (1100C, 25 degs., 100km/hr.)
    - Perform Mash Test 3-11 (2270P, 25 degs., 100 km/hr.)

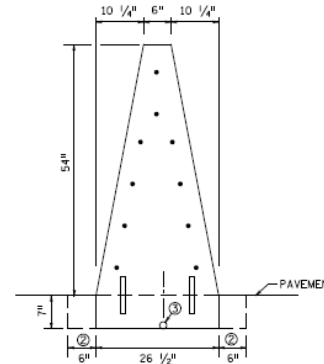
# Details of the Minnesota Barrier to use in the Design



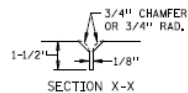
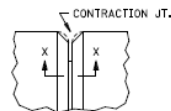
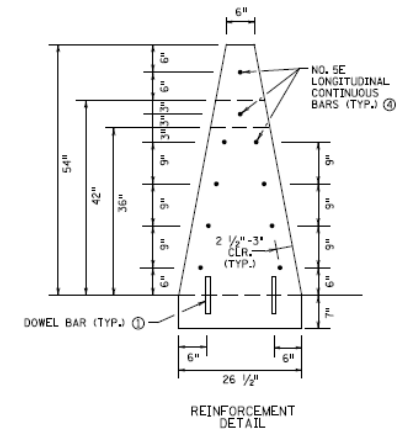
TYPE 36 A



TYPE 42 A



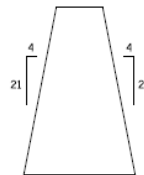
TYPE 54 A



CONTRACTION JOINT  
(TYPE A, TYPE A STEP, AND TYPE A-A MEDIAN BARRIERS;  
END ANCHOR BARRIER AND LIGHT POLE  
FOUNDATION/SIGN BASE TRANSITION BARRIER)

## CONTRACTION JOINT NOTES:

- IF JOINT SPACING IS NOT INDICATED IN THE PLANS, THE BASIS OF JOINT SPACING IS AS FOLLOWS:  
1) BITUMINOUS SECTION ADJACENT TO THE BARRIER;  
15 FT. SPACING.
- CONCRETE SECTION ADJACENT TO THE BARRIER;  
BARRIER CONTRACTION JOINTS SHALL ALIGN WITH JOINTS IN CONCRETE SECTION, NOT TO EXCEED 15 FT.
- REINFORCING TO BE CONTINUOUS THROUGH JOINT.
- IF FOOTING IS CONSTRUCTED SEPARATELY, PLACE BARRIER JOINTS DIRECTLY ABOVE FOOTING JOINTS.



BARRIER FACE SLOPE VALUE  
(ALL TYPE A BARRIERS)

SINGLE SLOPE BARRIER BILL OF REINFORCEMENT		
BARRIER TYPE	LONGITUDINAL BAR SIZE	NUMBER OF BARS EACH
36 A	5E	8
42 A	5E	9
54 A	5E	10

## NOTES:

- ALL BARS EPOXY COATED PER SPEC. 3301, UNLESS OTHERWISE NOTED.
- USE 3/4" CHAMFER OR 1" RADIUS ON ALL EXPOSED SHARP EDGES UNLESS OTHERWISE NOTED.
- DURING SLIP-FORM CONSTRUCTION, PROVIDE ALL NECESSARY SUPPORTS NEEDED TO MAINTAIN LONGITUDINAL REINFORCEMENT BARS AT SPECIFIED DIMENSIONS (INCIDENTAL).
- DURING FIXED-FORM CONSTRUCTION, MAINTAIN LONGITUDINAL REINFORCEMENT BARS AT DIMENSIONS SHOWN ON THE PLAN BY PROVIDING VERTICAL SUPPORT BARS AT 2'-0" MAX. SPACING (INCIDENTAL).
- IF FOOTING IS CONSTRUCTED SEPARATELY, PROVIDE TWO 1" DIA. DOWEL BARS (OR REBARS) 8" LONG AND SPACED EVERY 2'-0" ON CENTER, PROVIDE A ROUGH TEXTURE ON SURFACE OF FOOTING.
- 6" ADDITIONAL FOOTING WIDTH REQUIRED WHEN CONCRETE MEDIAN BARRIER IS ADJACENT TO BITUMINOUS PAVEMENT OR BITUMINOUS SHOULDER.
- WHEN REQUIRED, PROVIDE A 1-1/2" NOMINAL DIAMETER PVC-TYPE I CONDUIT (SPEC. 3803), LOCATE AS DIRECTED BY PLAN OR ENGINEER.
- CONTINUOUS NO. 5E LONGITUDINAL REINFORCEMENT BARS WITH 2'-1/2" - 3" MINIMUM CLEARANCE EVENLY SPACED AS SHOWN IN THE DETAILS. MINIMUM LAP SPICE IS 3'-1" FOR ALL BARS. SEE TABLE FOR BAR QUANTITIES.



REVISOR  
Tom Sh  
STATE DESIGN ENGINEER

REVISED

APPROVED  
8-10-2016

CONCRETE MEDIAN BARRIER SINGLE SLOPE  
TYPE 36A, 42A, AND 54A

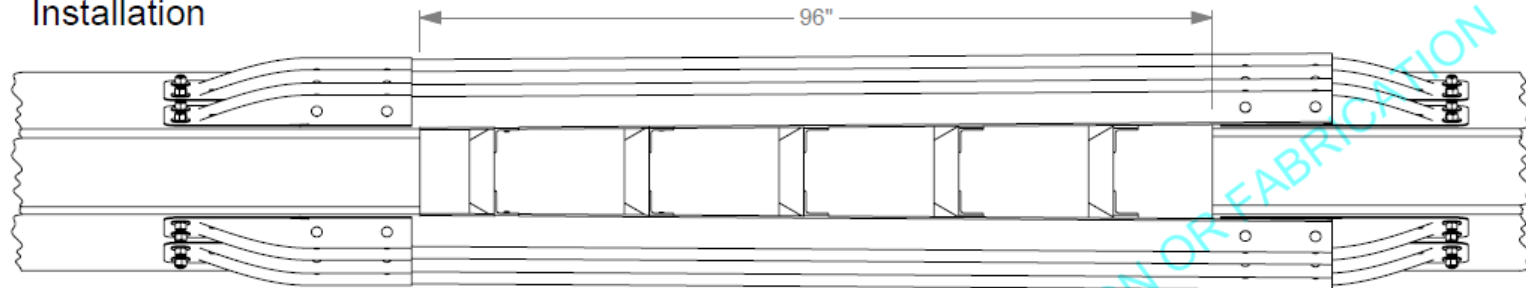
STANDARD PLAN 5-297.681

1 OF 6

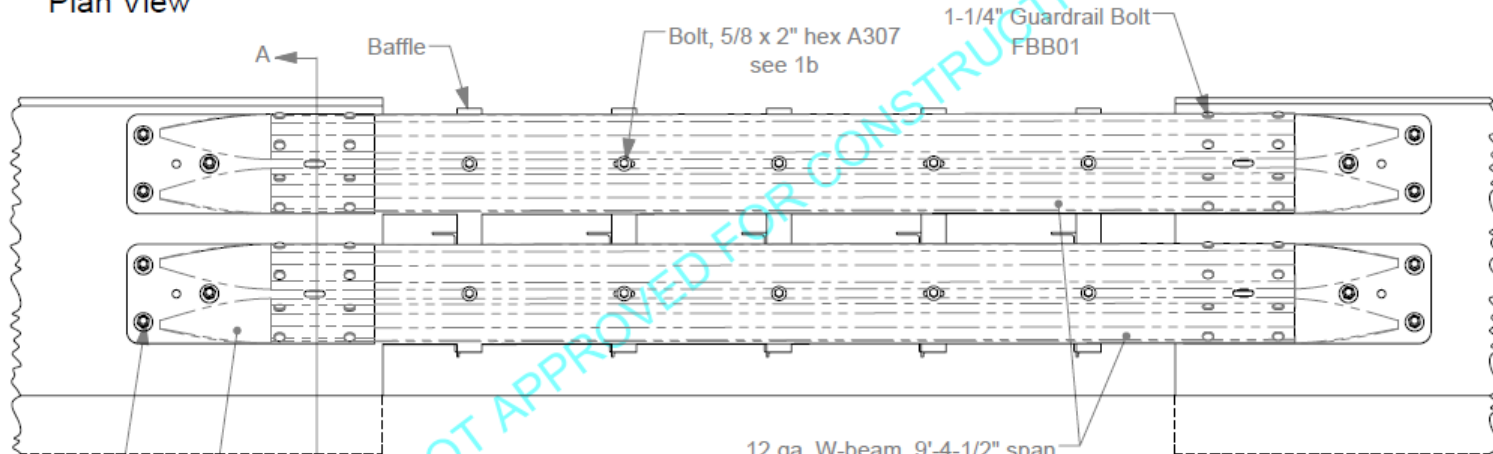


# Option 1 – W-Beams with Brace Frames

Installation



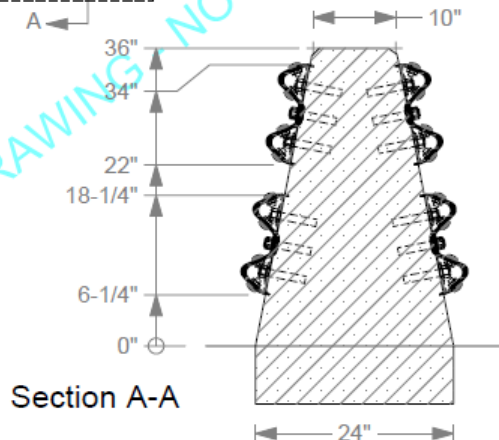
Plan View



Elevation View

B7 Threaded Rod,  
Ø7/8" x 5 3/4"  
with SAE Hardened Washer  
and Heavy Hex Nut - See 1a

W-beam Terminal Connector  
RWE02b  
Typ x 2 each end



Section A-A

1a. Secure with Hilti HIT-RE 500 V3 epoxy according to manufacturer's instructions, with 4-1/2" embedment.

1b. Recessed Guardrail Nut on all 5/8 Bolts. USS Flat Washer under the head on all 5/8 hex bolts securing Rails to Baffles (Bolt head and Washer on traffic side).



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Project #610461 Barrier Gap

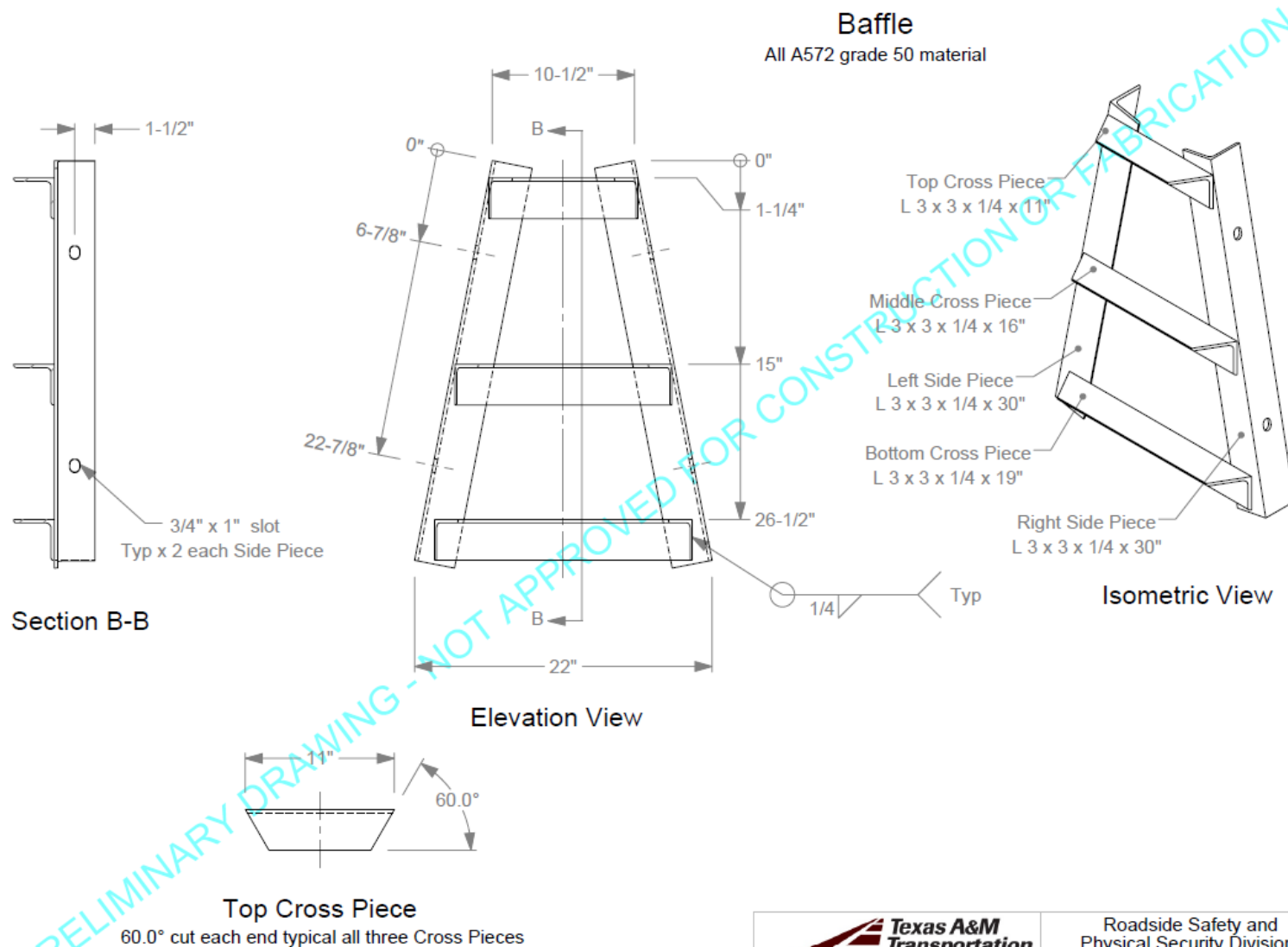
2018-09-13

Drawn by GES

Scale 1:20

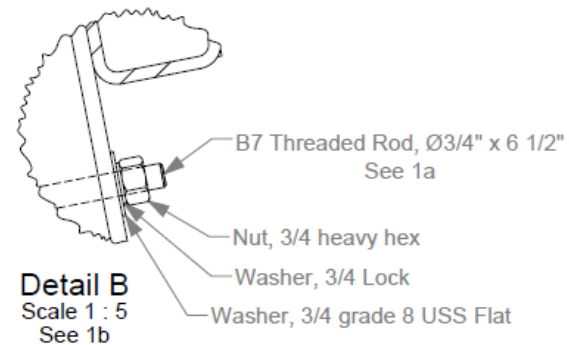
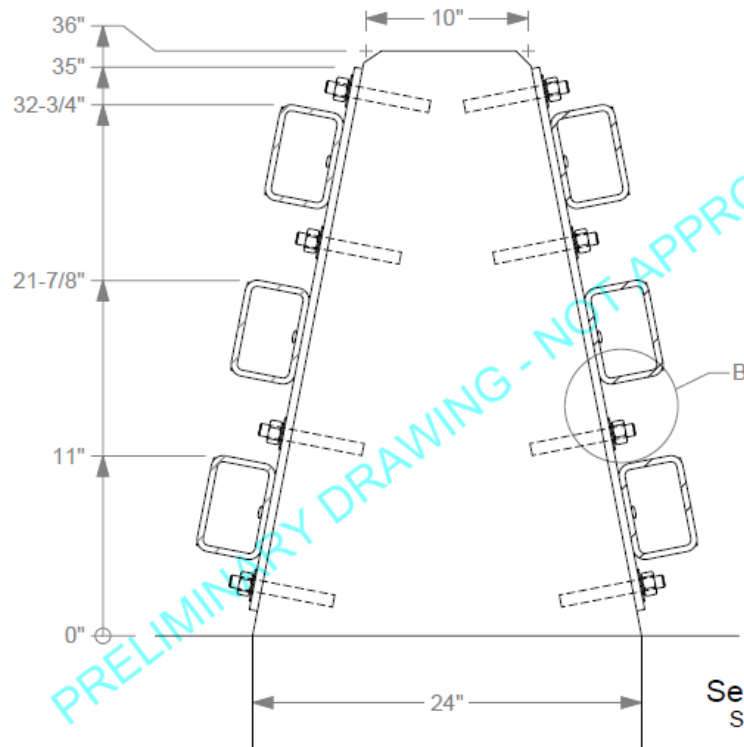
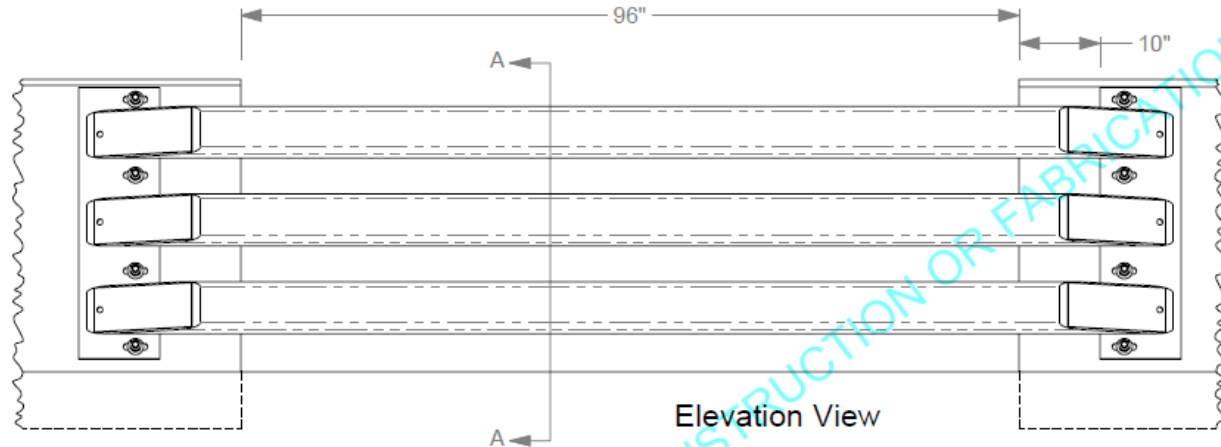
Sheet 1 of 3 Installation

# Option 1 – W-Beams with Brace Frames



# Option 2 – Steel Tubes with Slotted Plates

Installation



**1a.** Install centered in slots, with 4-1/2" embedment. Secure with Hilti HIT-RE 500 V3 epoxy according to manufacturer's instructions. Tighten to snug fit only after epoxy has cured.

**1b.** Threaded Rods and all connecting hardware shall be galvanized.



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Project #610461 Barrier Gap

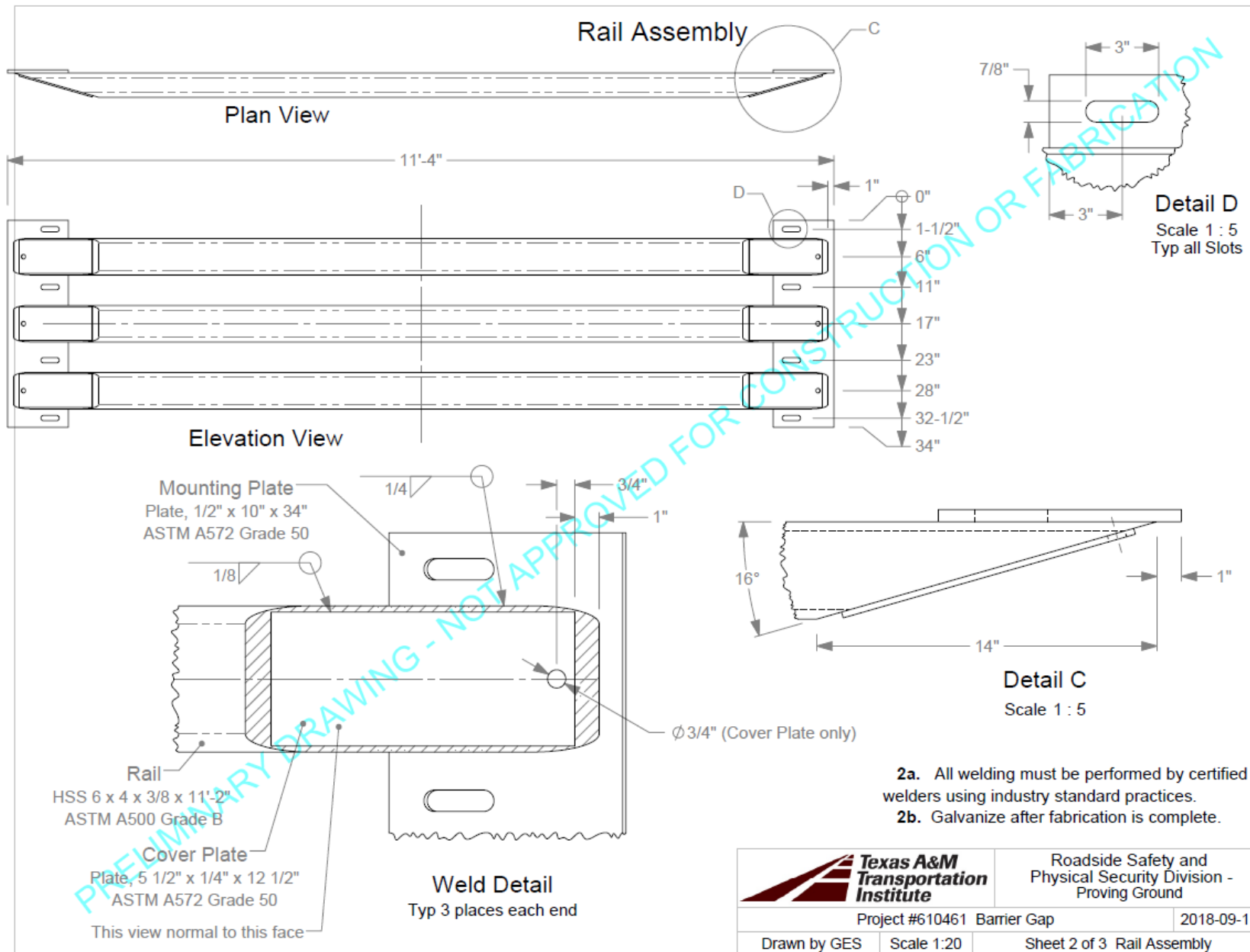
2018-09-11


Drawn by GES

Scale 1:20

Sheet 1 of 3 Installation

# Option 2 – Steel Tubes with Slotted Plates





# **Placement of Guardrail on Slopes Phase IV: MASH TL-3 Testing of Guardrail on 1:1 Slope**

**TTI Researcher: Akram Abu-Odeh**

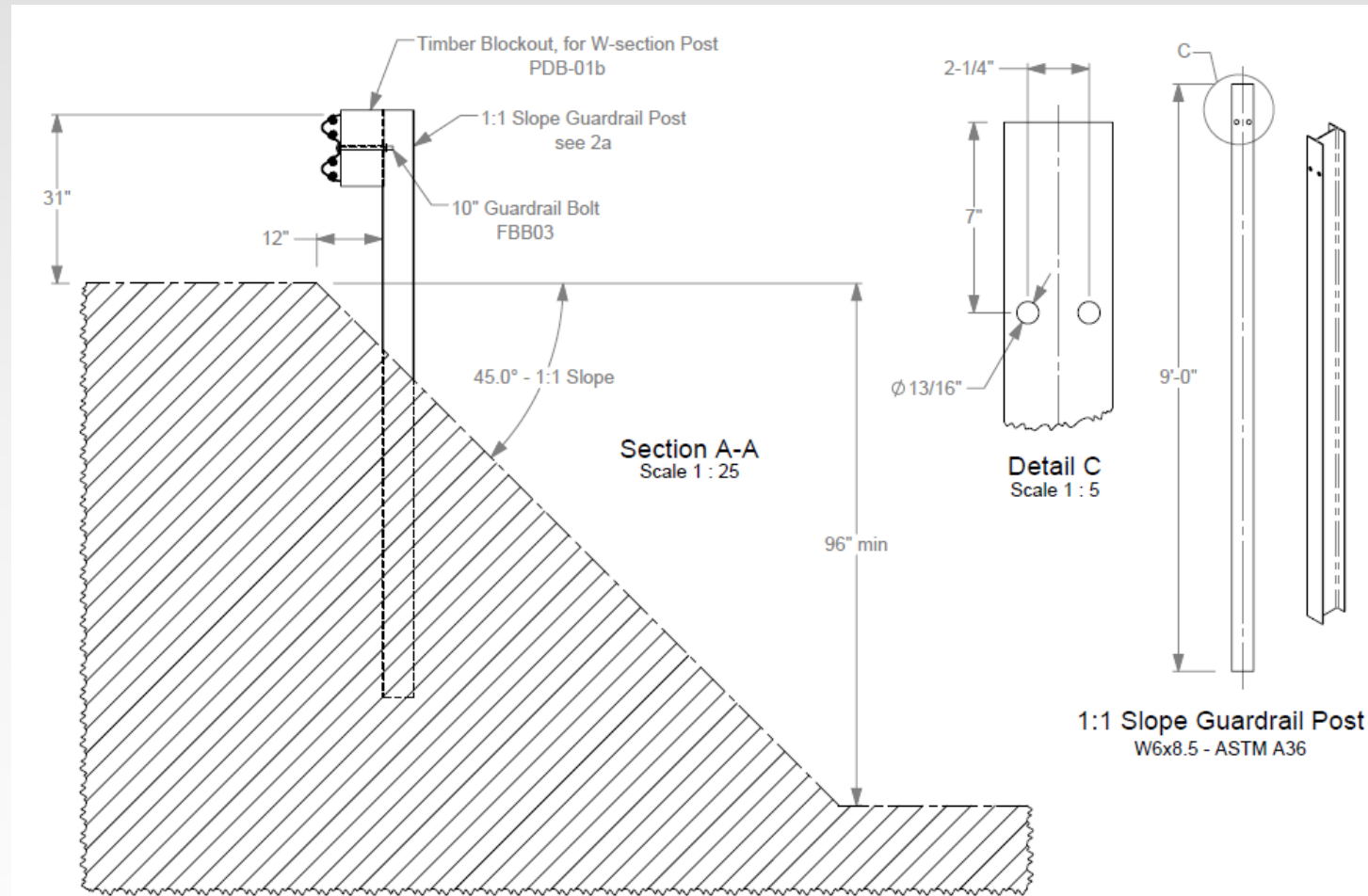
**Technical Representative: Joe H. Hall (WV DOT)**



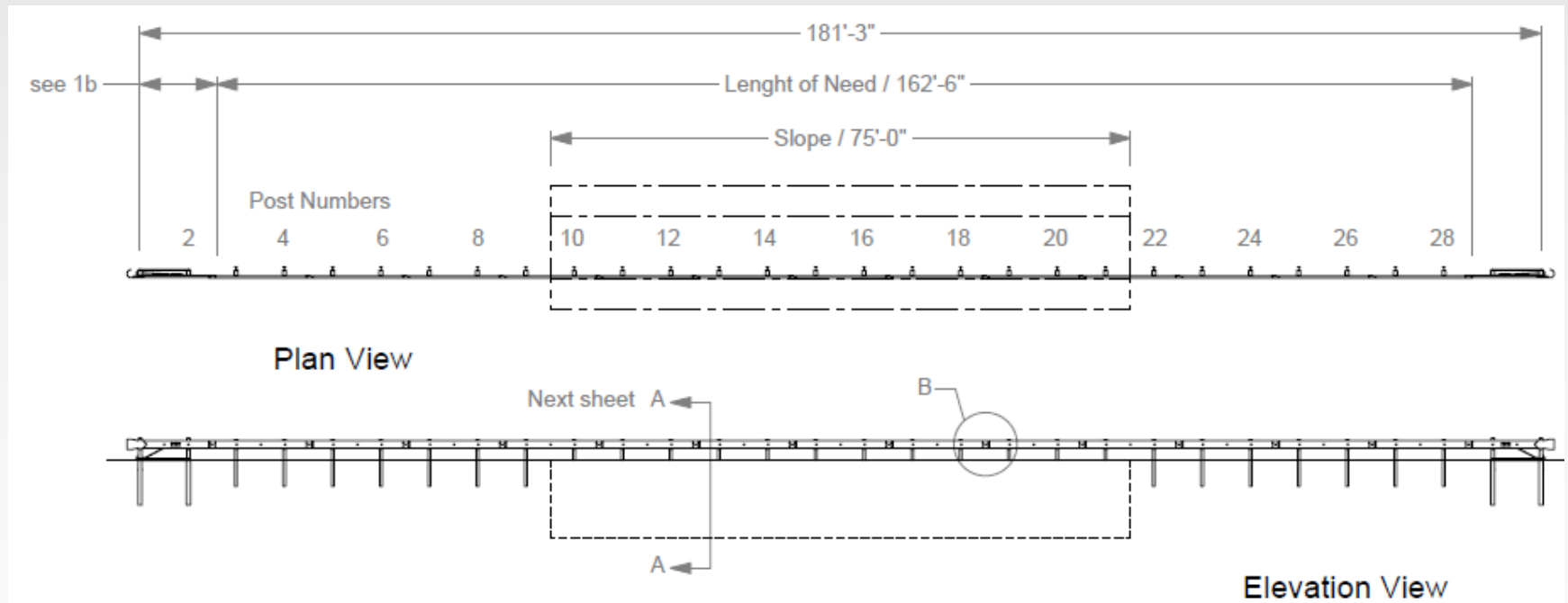
# MASH TL-3 Guardrail on 1:1 Slope

- A new guardrail design to be evaluated under MASH TL-3 test conditions
- 31-inch w-beam system.
- Splices are in between posts with standard post spacing.
- 9-ft posts are installed on the slope so the face of the guardrail aligned with the slope break.

# MASH TL-3 Guardrail on 1:1 Slope



# MASH TL-3 Guardrail on 1:1 Slope



# MASH TL-3 Guardrail on 1:1 Slope

MASH Test 3-10





# MASH TL-3 Guardrail on 1:1 Slope





# MASH TL-3 Guardrail on 1:1 Slope



# MASH TL-3 Guardrail on 1:1 Slope





# MASH TL-3 Guardrail on 1:1 Slope





# MASH TL-3 Guardrail on 1:1 Slope



# MASH TL-3 Guardrail on 1:1 Slope

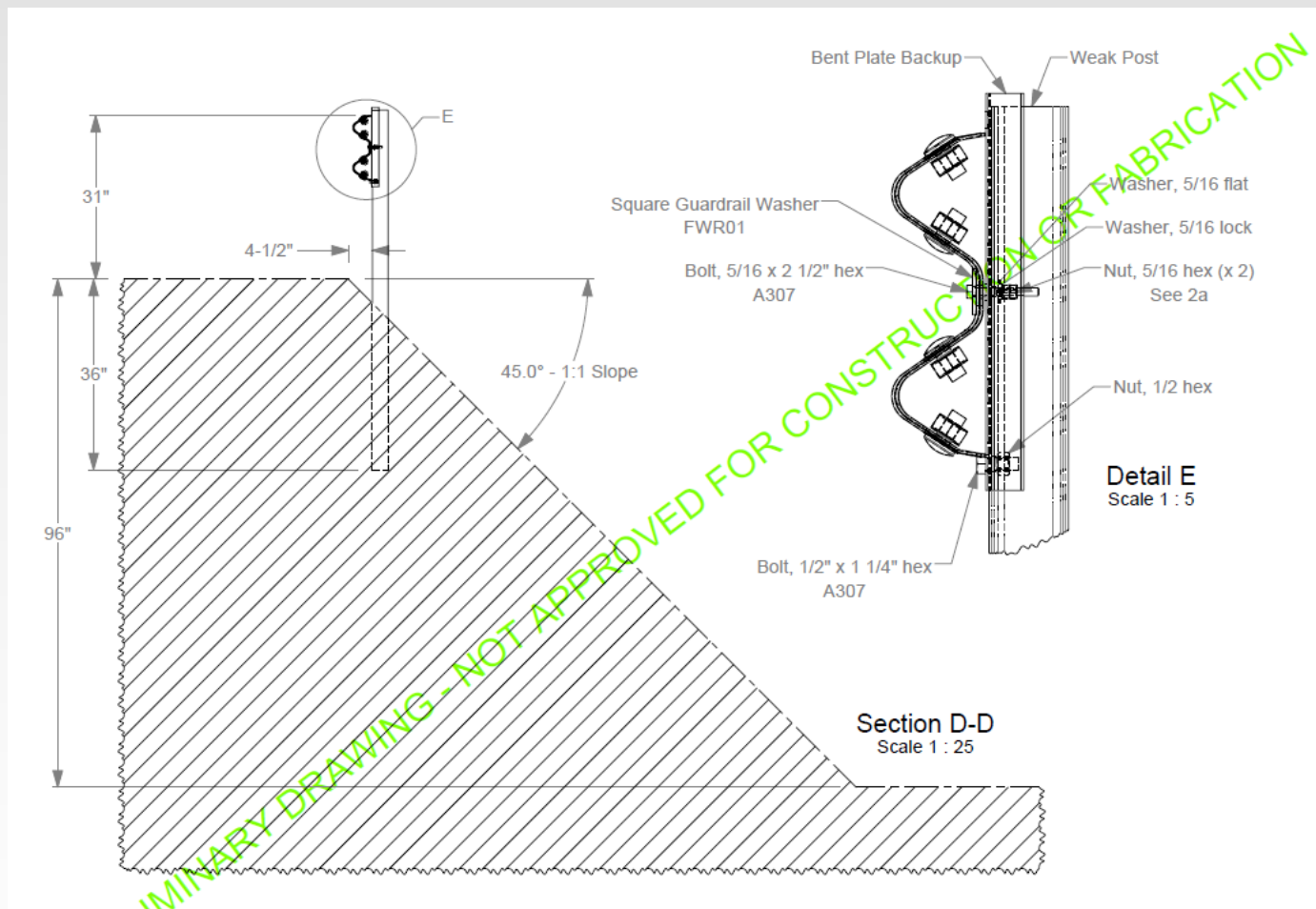
- It is assessed that the rail edge engaged with a sharp interior edge behind the fender
- The rail system seems to be stiffer than desired
- Two recommended ways for reducing the rail stiffness
  - Shortening rail embedment
  - Using weaker posts

# MASH TL-3 Guardrail on 1:1 Slope

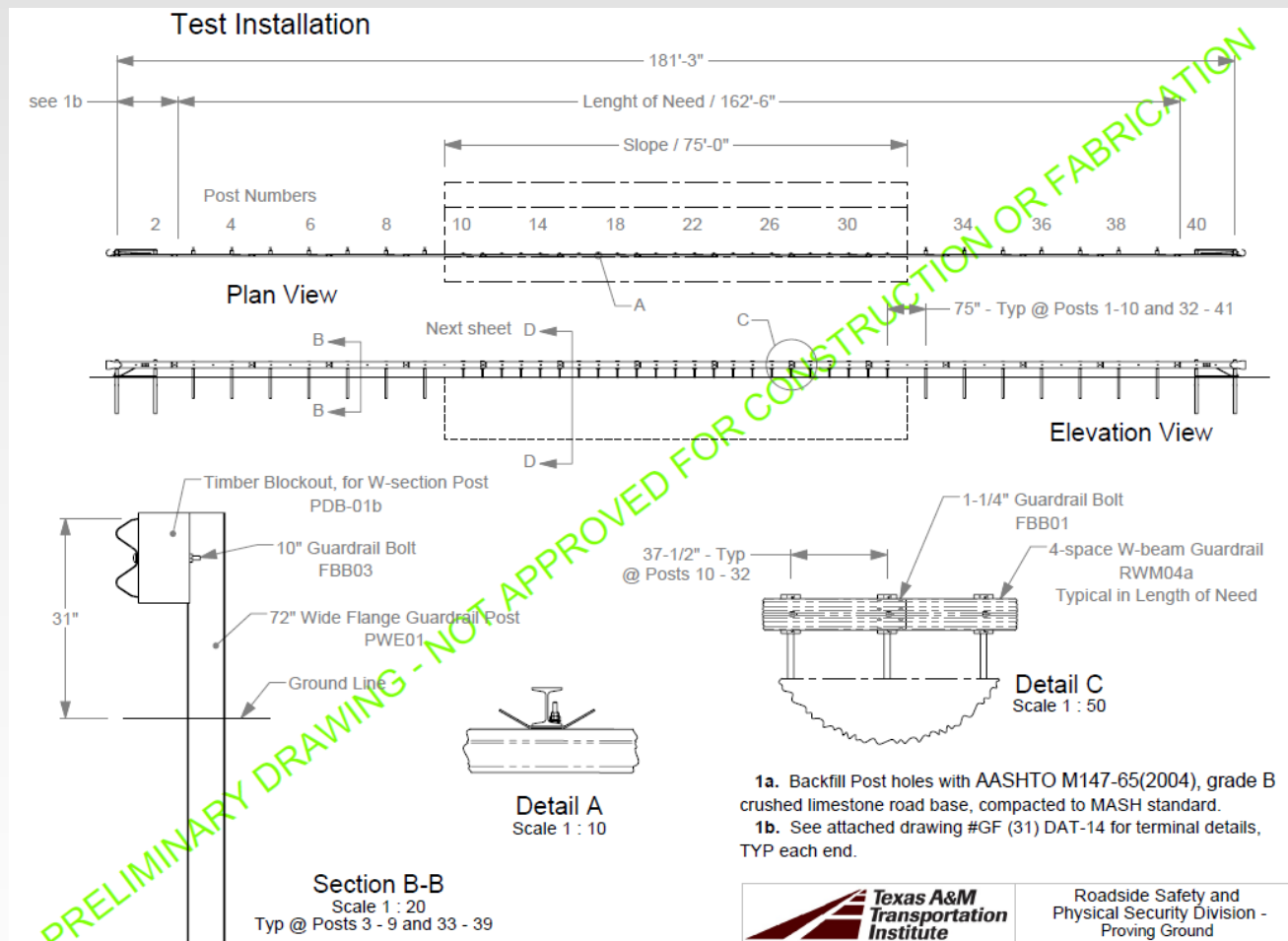
- Using weak post system seems to be more practical
  - Easier and consistent in terms of installation (S3 x 5.7), especially given the mountainous rock formations
  - Less embedment depth
  - Reduced soil dependency
  - Closer the slope break



# MASH TL-3 Guardrail on 1:1 Slope




# MASH TL-3 Guardrail on 1:1 Slope



# MASH TL-3 Guardrail on 1:1 Slope

- Items left to complete the project
  - Request time extension
  - LS-DYNA analysis with pickup truck
  - Request additional funds to test the truck and potential the small car
  - The testing could be in an extension to the current project or a new testing project for the weak post system



# **Accommodating Inlets with Transitions (TL-3)**

**TTI Researcher: Akram Abu-Odeh**

**Technical Representative: Derwood C. Sheppard, (FDOT)**

# Accommodating Inlets with Transitions

- Develop a transition design to be evaluated under MASH TL-3 test conditions
- Hydraulic inlets configurations to be accommodated in the design
- Proposal being prepared for the following research activities.
  - Polling State DOT's with these inlets configurations
  - Perform nonlinear finite element analyses
  - Perform MASH TL 3 transition test for the 2270P test vehicle