

Test Report No. 612831-01 Test Report Date: May 2020

# MASH EVALUATION OF F-SHAPE AND SINGLE-SLOPE CONCRETE BARRIERS WITH DRAINAGE SCUPPERS

by

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Contract No.: 1906480

Test No.: 612831-01-1 / -2 / -3

Test Date: 2020-01-07 / 2020-01-09 / 2019-12-17

Sponsored by

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**Technical Report Documentation Page** 

1. Report No.	2. Government Accession No.	3. Recipient's Catalog No.
4. Title and Subtitle MASH EVALUATION OF F-SHAPE AND SINGLE-SLOPE CONCRETE BARRIERS WITH DRAINAGE SCUPPERS		5. Report Date May 2020 6. Performing Organization Code
7. Author(s) Nauman M. Sheikh, Sana M. Moran, Wanda L. Menges, Bill L. Griffith, and Darrell L. Kuhn		8. Performing Organization Report No. Test Report No. 612831-01
9. Performing Organization Name and Address Texas A&M Transportation Institute Proving Ground 3135 TAMU College Station, Texas 77843-3135		10. Work Unit No. (TRAIS)  11. Contract or Grant No. 1906480 (T4541)
12. Sponsoring Agency Name and Address Washington State Department of Transportation Transportation Building, MS: 47372 Olympia, WA 98504-7372		13. Type of Report and Period Covered Technical Report: June 2019 – May 2020  14. Sponsoring Agency Code

15. Supplementary Notes

Project Title: MASH Testing of Single-Slope and F-Shape Barriers with Drainage Scuppers Name of Contacting Representative: Tim Moeckel, WSDOT

16. Abstract

Washington State Department of Transportation (WSDOT) desired to evaluate F-shape and single slope concrete barriers with 6-inch tall drainage scuppers for compliance with American Association of State Highway and Transportation Official's (AASHTO) *Manual for Assessing Safety Hardware (MASH)* evaluation criteria. The F-shape was a temporary concrete barrier with pin-and-loop connection. It was evaluated as a free-standing and a pinned-down barrier system on asphalt pavement. The single slope concrete barrier was comprised of 12.5-ft precast barrier segments that were connected using the grouted rebar grid connections. The single slope barrier system was embedded 4 inches in asphalt.

The free-standing and pinned F-shape barrier systems were evaluated for *MASH* Test Level 3 (TL-3) criteria by performing *MASH* Test 3-11. The single slope barrier was evaluated for *MASH* Test Level 4 (TL-4) criteria by performing *MASH* Test 4-12.

The pinned-down and the free-standing F-shape barriers with drainage scuppers passed the criteria for *MASH* Test 3-11 for longitudinal barriers. The embedded single-slope barrier with drainage scuppers also performed acceptably for *MASH* Test 4-12 for longitudinal barriers.

17. Key Words 18. Distribution Statement Copyrighted. Not to be copied or reprinted without Portable concrete barrier, PCB, temporary concrete consent from the Roadside Safety Research Pooled barrier, TCB, pinned down, anchored, free standing, embedded, drainage scuppers, F-shape, MASH, Fund. crash testing, roadside safety 19. Security Classif.(of this report) 20. Security Classif.(of this page) 21. No. of Pages 22. Price Unclassified Unclassified 146

SI* (MODERN METRIC) CONVERSION FACTORS				
APPROXIMATE CONVERSIONS TO SI UNITS				
Symbol	When You Know	Multiply By	To Find	Symbol
	1	LENGTH	1	1 2 3 11111 21
in	inches	25.4	millimeters	mm
ft	feet	0.305	meters	m
yd	yards	0.914	meters	m
mi	miles	1.61	kilometers	km
		AREA		
in <sup>2</sup>	square inches	645.2	square millimeters	$\text{mm}^2$
ft <sup>2</sup>	square feet	0.093	square meters	m <sup>2</sup>
yd <sup>2</sup>	square yards	0.836	square meters	m <sup>2</sup>
ac	acres	0.405	hectares	ha
mi <sup>2</sup>	square miles	2.59	square kilometers	km <sup>2</sup>
	5 q a a	VOLUME		
fl oz	fluid ounces	29.57	milliliters	mL
gal	gallons	3.785	liters	I
ft <sup>3</sup>	cubic feet	0.028	cubic meters	m <sup>3</sup>
yd <sup>3</sup>	cubic yards	0.765	cubic meters	m <sup>3</sup>
yu		mes greater than 1000l		""
	NOTE. Volu	MASS	L SHAII DE SHOWH III III	
07	OLIDOOS	28.35	gromo	<b>a</b>
OZ	ounces	26.35 0.454	grams	g
lb T	pounds	0.454	kilograms	kg
1	short tons (2000 lb)		megagrams (or metric ton")	Mg (or "t")
۰.		EMPERATURE (exac		
°F	Fahrenheit	5(F-32)/9	Celsius	°C
		or (F-32)/1.8		
		RCE and PRESSURE		
lbf	poundforce	4.45	newtons	N
lbf/in <sup>2</sup>	poundforce per square inc		kilopascals	kPa
		MATE CONVERSION		
Symbol	When You Know	Multiply By	To Find	Symbol
		LENGTH		
mm	millimeters	0.000		
		0.039	inches	in
m	meters	3.28	inches feet	in ft
m m		3.28	feet	ft
	meters			
m	meters meters	3.28 1.09 0.621	feet yards	ft yd
m km	meters meters kilometers	3.28 1.09 0.621 <b>AREA</b>	feet yards miles	ft yd mi
m km mm²	meters meters kilometers square millimeters	3.28 1.09 0.621 <b>AREA</b> 0.0016	feet yards miles square inches	ft yd mi in²
m km mm <sup>2</sup> m <sup>2</sup>	meters meters kilometers square millimeters square meters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764	feet yards miles square inches square feet	ft yd mi in <sup>2</sup> ft <sup>2</sup>
m km mm² m² m²	meters meters kilometers  square millimeters square meters square meters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195	feet yards miles  square inches square feet square yards	ft yd mi in <sup>2</sup> ft <sup>2</sup> yd <sup>2</sup>
m km mm² m² m² ha	meters meters kilometers  square millimeters square meters square meters hectares	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47	feet yards miles  square inches square feet square yards acres	ft yd mi in <sup>2</sup> ft <sup>2</sup> yd <sup>2</sup> ac
m km mm² m² m²	meters meters kilometers  square millimeters square meters square meters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386	feet yards miles  square inches square feet square yards	ft yd mi in <sup>2</sup> ft <sup>2</sup> yd <sup>2</sup>
m km mm² m² m² ha km²	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b>	feet yards miles  square inches square feet square yards acres square miles	ft yd mi in <sup>2</sup> ft <sup>2</sup> yd <sup>2</sup> ac mi <sup>2</sup>
m km mm² m² m² ha km²	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers milliliters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034	feet yards miles  square inches square feet square yards acres square miles  fluid ounces	ft yd mi in <sup>2</sup> ft <sup>2</sup> yd <sup>2</sup> ac mi <sup>2</sup>
m km mm² m² m² ha km² mL	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons	ft yd mi in <sup>2</sup> ft <sup>2</sup> yd <sup>2</sup> ac mi <sup>2</sup> oz gal
m km mm² m² m² ha km² mL L m³	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet	ft yd mi  in² ft² yd² ac mi²  oz gal ft³
m km mm² m² m² ha km² mL	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314 1.307	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons	ft yd mi  in² ft² yd² ac mi²  oz gal
m km mm² m² m² ha km² mL L m³ m³	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters cubic meters	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314 1.307 <b>MASS</b>	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet cubic yards	ft yd mi  in² ft² yd² ac mi²  oz gal ft³ yd³
m km mm² m² ha km² mL L m³ m³	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters cubic meters grams	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314 1.307 <b>MASS</b> 0.035	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet cubic yards  ounces	ft yd mi  in² ft² yd² ac mi²  oz gal ft³ yd³  oz
m km mm² m² ha km² mL L m³ m³	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters cubic meters grams kilograms	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314 1.307 <b>MASS</b> 0.035 2.202	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet cubic yards  ounces pounds	ft yd mi  in² ft² yd² ac mi²  oz gal ft³ yd³  oz lb
m km mm² m² ha km² mL L m³ m³	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric ton	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314 1.307 <b>MASS</b> 0.035 2.202 1.103	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet cubic yards  ounces pounds short tons (2000lb)	ft yd mi  in² ft² yd² ac mi²  oz gal ft³ yd³  oz
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m km mm² m² ha km² mL L m³ m³	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric ton	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314 1.307 <b>MASS</b> 0.035 2.202 1.103 <b>EMPERATURE (exac</b> 1.8C+32	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet cubic yards  ounces pounds short tons (2000lb)  ct degrees) Fahrenheit	ft yd mi  in² ft² yd² ac mi²  oz gal ft³ yd³  oz lb
m km  mm² m² m² m² ha km²  mL L m³ m³ dg kg Mg (or "t")	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric ton TE Celsius	3.28 1.09 0.621 AREA 0.0016 10.764 1.195 2.47 0.386 VOLUME 0.034 0.264 35.314 1.307 MASS 0.035 2.202 1.103 EMPERATURE (exaction (exactio	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet cubic yards  ounces pounds short tons (2000lb)  ct degrees) Fahrenheit E or STRESS	ft yd mi  in² ft² yd² ac mi²  oz gal ft³ yd³  oz lb T
m km  mm² m² m² ha km²  mL L m³ m³ m³	meters meters kilometers  square millimeters square meters square meters hectares Square kilometers  milliliters liters cubic meters cubic meters grams kilograms megagrams (or "metric ton	3.28 1.09 0.621 <b>AREA</b> 0.0016 10.764 1.195 2.47 0.386 <b>VOLUME</b> 0.034 0.264 35.314 1.307 <b>MASS</b> 0.035 2.202 1.103 <b>EMPERATURE (exac</b> 1.8C+32	feet yards miles  square inches square feet square yards acres square miles  fluid ounces gallons cubic feet cubic yards  ounces pounds short tons (2000lb)  ct degrees) Fahrenheit	ft yd mi  in² ft² yd² ac mi²  oz gal ft³ yd³  oz lb T

<sup>\*</sup>SI is the symbol for the International System of Units

#### **ACKNOWLEDGMENTS**

This research project was performed under a pooled fund program between the following States and Agencies. The authors acknowledge and appreciate their guidance and assistance.

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Revised February 2020

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TR No. 612831-01 2020-05-07

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TR No. 612831-01 2020-05-07

# TABLE OF CONTENTS

		Page
Disclaimer		
O	es	
	S	
Chapter 1.	Introduction	
1.1	Problem Statement	
1.2	Objective and Scope	
Chapter 2.	Test Requirements and Evaluation Criteria	
2.1	Crash Test Performed / Matrix	
2.2	Evaluation Criteria	
Chapter 3.	Test Conditions	
3.1	Test Facility	
3.2	Vehicle Tow and Guidance System	5
3.3	Data Acquisition Systems	5
3.3.1	Vehicle Instrumentation and Data Processing	5
3.3.2	Anthropomorphic Dummy Instrumentation	6
3.3.3	Photographic Instrumentation Data Processing	
Chapter 4.	MASH Test 3-11 on Pinned F-shape Barrier with Drainage Scuppers	9
4.1	Test Article and Installation Details	9
4.2	Design Modifications during Tests	9
4.3	Material Specifications	9
4.4	Test Designation and Actual Impact Conditions	9
4.5	Weather Conditions	
4.6	Test Vehicle	13
4.7	Test Description	13
4.8	Damage to Test Installation	14
4.9	Damage to Test Vehicle	16
4.10	Occupant Risk Factors	17
Chapter 5.	MASH Test 3-11 on Free-Standing F-shape Barrier with Drainage	
•	Scuppers	19
5.1	Test Article and Installation Details	19
5.2	Design Modifications during Tests	19
5.3	Material Specifications	19
5.4	Test Designation and Actual Impact Conditions	
5.5	Weather Conditions.	
5.6	Test Vehicle	22
5.7	Test Description	23
5.8	Damage to Test Installation	
5.9	Damage to Test Vehicle	
5.10	Occupant Risk Factors	
Chapter 6.	MASH Test 4-12 on Embedded Single-Slope Barrier with Drainage	
F	Scuppers	29
6.1	Test Article and Installation Details	
6.2	Design Modifications during Tests	

# TABLE OF CONTENTS (CONTINUED)

6.2	Matarial Crossifications	Page
6.3 6.4	Material Specifications Test Designation and Actual Impact Conditions	
6.5	Weather Conditions	
6.6	Test Pessenistics	
6.7	Test Description	
6.8	Damage to Test Installation	
6.9	Damage to Test Vehicle	
6.10		
Chapter 7.	Summary and Conclusions	
7.1	Assessment of Test Results	
7.1.1		37
	MASH Test 3-11 on Free-Standing F-Shape Barrier with Drainage	27
	pers	3/
	MASH Test 4-12 on the Embedded Single-Slope Barrier with Drainage	27
	pers	
7.2	Conclusions	
	MASH Test 3-11 on Pinned F-Shape Barrier with Drainage Scuppers	41
7.2.2	MASH Test 3-11 on Free-Standing F-Shape Barrier with Drainage	4.1
	pers	41
	MASH Test 4-12 on the Embedded Single-Slope Barrier with Drainage	4.1
	pers	
Chapter 8.	Implementation	
References	D.4-164- T-441-	
	Details of the Test articles	
	ed F-shape barrier with drainage scuppers	
	standing F-shape barrier with drainage scuppers	
	edded Single-Slope Barrier with Drainage Scuppers	
	Supporting Certification Documents	
	. MASH Test 3-11 (Crash Test No. 612831-01-1)	
C1	Vehicle Properties and Information	
C2	Sequential Photographs	
C3	Vehicle Angular Displacements	
C4	Vehicle Accelerations	
	. MASH Test 3-11 (Crash Test No. 612831-01-2)	
D1	Vehicle Properties and Information	
D2	Sequential Photographs	
D3	Vehicle Angular Displacements	
D4	Vehicle Accelerations	
	. MASH Test 4-12 (Crash Test No. 612831-01-3)	
E1	Vehicle Properties and Information	
E2	Sequential Photographs	
E3	Vehicle Angular Displacements	
E4	Vehicle Accelerations	125

# LIST OF FIGURES

		Page
Figure 4.1.	Details of Pinned F-Shape Barrier with Drainage Scuppers	10
Figure 4.2.	Pinned F-Shape Barrier with Drainage Scuppers prior to Testing	12
Figure 4.3.	Barrier/Test Vehicle Geometrics for Test No. 612831-01-1	13
Figure 4.4.	Test Vehicle before Test No. 612831-01-1.	14
Figure 4.5.	Barrier after Test No. 612831-01-1	15
Figure 4.6.	Test Vehicle after Test No. 612831-01-1	16
Figure 4.7.	Interior of Test Vehicle after Test No. 612831-01-1.	17
Figure 4.8.	Summary of Results for MASH Test 3-11 on Pinned F-Shape Barrier with	
	Drainage Scuppers.	18
Figure 5.1.	Details of Free-Standing F-Shape Barrier with Drainage Scuppers	20
Figure 5.2.	Free-Standing F-Shape Barrier with Drainage Scuppers Prior to Testing	21
Figure 5.3.	Barrier/Test Vehicle Geometrics for Test No. 612831-01-2	22
Figure 5.4.	Test Vehicle before Test No. 612831-01-2.	22
Figure 5.5.	Barrier after Test No. 612831-01-2.	24
Figure 5.6.	Field Side of Barrier after Test No. 612831-01-2	25
Figure 5.7.	Test Vehicle after Test No. 612831-01-2	26
Figure 5.8.	Interior of Test Vehicle after Test No. 612831-01-2.	26
Figure 5.9.	Summary of Results for MASH Test 3-11 on Free-Standing F-Shape	
	Barrier with Drainage Scuppers	28
Figure 6.1.	Details of Embedded Single-Slope Barrier with Drainage Scuppers	30
Figure 6.2.	Embedded Single-Slope Barrier with Drainage Scuppers Prior to Testing	31
Figure 6.3.	Embedded Single-Slope Barrier with Drainage Scuppers/Test Vehicle	
	Geometrics for Test No. 612831-01-3.	32
Figure 6.4.	Test Vehicle before Test No. 612831-01-3.	33
Figure 6.5.	Embedded Single-Slope Barrier with Scuppers after Test No.	
	612831-01-3	
Figure 6.6.	Test Vehicle after Test No. 612831-01-3.	35
Figure 6.7.	Interior of Test Vehicle after Test No. 612831-01-3.	35
Figure 6.8.	Summary of Results for MASH Test 4-12 on Embedded Single-Slope	
	Barrier with Drainage Scuppers	36
Figure C.1.	Sequential Photographs for Test No. 612831-01-1 (Overhead and Frontal	
	Views).	99
Figure C.2.	Sequential Photographs for Test No. 612831-01-1 (Rear View)	
Figure C.3.	Vehicle Angular Displacements for Test No. 612831-01-1	102
Figure C.4.	Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-1	
	(Accelerometer Located at Center of Gravity).	103
Figure C.5.	Vehicle Lateral Accelerometer Trace for Test No. 612831-01-1	
	(Accelerometer Located at Center of Gravity).	104
Figure C.6.	Vehicle Vertical Accelerometer Trace for Test No. 612831-01-1	
	(Accelerometer Located at Center of Gravity).	105
Figure D.1.	Sequential Photographs for Test No. 612831-01-2 (Overhead and Frontal	
	Views).	
Figure D.2.	Sequential Photographs for Test No. 612831-01-2 (Rear View)	113

# TABLE OF FIGURES (CONTINUED)

	Page
Vehicle Angular Displacements for Test No. 612831-01-2	114
Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-2	
(Accelerometer Located at Center of Gravity).	115
Vehicle Lateral Accelerometer Trace for Test No. 612831-01-2	
(Accelerometer Located at Center of Gravity).	116
Vehicle Vertical Accelerometer Trace for Test No. 612831-01-2	
(Accelerometer Located at Center of Gravity).	117
Sequential Photographs for Test No. 612831-01-3 (Overhead and Frontal	
Views).	121
Sequential Photographs for Test No. 612831-01-3 (Rear View)	123
Vehicle Angular Displacements for Test No. 612831-01-3	124
Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-3	
(Accelerometer Located at Center of Gravity).	125
Vehicle Lateral Accelerometer Trace for Test No. 612831-01-3	
(Accelerometer Located at Center of Gravity).	126
Vehicle Vertical Accelerometer Trace for Test No. 612831-01-3	
(Accelerometer Located at Center of Gravity).	127
	Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-2 (Accelerometer Located at Center of Gravity).  Vehicle Lateral Accelerometer Trace for Test No. 612831-01-2 (Accelerometer Located at Center of Gravity).  Vehicle Vertical Accelerometer Trace for Test No. 612831-01-2 (Accelerometer Located at Center of Gravity).  Sequential Photographs for Test No. 612831-01-3 (Overhead and Frontal Views).  Sequential Photographs for Test No. 612831-01-3 (Rear View).  Vehicle Angular Displacements for Test No. 612831-01-3.  Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-3 (Accelerometer Located at Center of Gravity).  Vehicle Lateral Accelerometer Trace for Test No. 612831-01-3 (Accelerometer Located at Center of Gravity).  Vehicle Vertical Accelerometer Trace for Test No. 612831-01-3

# LIST OF TABLES

		Page
Table 2.1.	Test Conditions and Evaluation Criteria Specified for MASH TL-3 and	_
	TL-4 Longitudinal Barriers.	3
Table 2.2.	Evaluation Criteria Required for MASH TL-3 and TL-4 Longitudinal	
	Barriers.	
Table 4.1.	Events during Test No. 612831-01-1.	14
Table 4.2.	Barrier Segment Movement for Test No. 612831-01-1	16
Table 4.3.	Barrier Segment Pin Lift for Test No. 612831-01-1	16
Table 4.4.	Occupant Risk Factors for Test No. 612831-01-1	17
Table 5.1.	Events during Test No. 612831-01-2	23
Table 5.2.	Barrier Segment Movement for Test No. 612831-01-2	25
Table 5.3.	Occupant Risk Factors for Test No. 612831-01-2	
Table 6.1.	Events during Test No. 612831-01-3	33
Table 7.1.	Performance Evaluation Summary for MASH Test 3-11 on Pinned F-	
	Shape Barrier with Drainage Scuppers.	38
Table 7.2.	Performance Evaluation Summary for MASH Test 3-11 on Free-Standing	
	F-Shape Barrier with Drainage Scuppers.	39
Table 7.3.	Performance Evaluation Summary for MASH Test 4-12 on Embedded	
	Single-Slope Barrier with Drainage Scuppers	40
Table C.1.	Vehicle Properties for Test No. 612831-01-1.	95
Table C.2.	Measurements of Vehicle Vertical CG for Test No. 612831-01-1	96
Table C.3.	Exterior Crush Measurements for Test No. 612831-01-1	97
Table C.4.	Occupant Compartment Measurements for Test No. 612831-01-1	98
Table D.1.	Vehicle Properties for Test No. 612831-01-2.	
Table D.2.	Measurements of Vehicle Vertical CG for Test No. 612831-01-2	108
Table D.3.	Exterior Crush Measurements for Test No. 612831-01-2	109
Table D.4.	Occupant Compartment Measurements for Test No. 612831-01-2	110
Table E.1.	Vehicle Properties for Test No. 612831-01-3.	
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TR No. 612831-01 2020-05-07

# **Chapter 1. INTRODUCTION**

# 1.1 PROBLEM STATEMENT

Washington State Department of Transportation (WSDOT) wanted to analyze and test three concrete barrier systems with drainage scuppers. These systems included a pinned F-shape barrier, a freestanding F-shape barrier, and an embedded single-slope barrier. All three barriers have 6-inch tall drainage scuppers when installed. The F-shape barrier systems needed to be compliant to Test Level 3 (TL-3) criteria of American Association of State Highway and Transportation Official's (AASHTO) *Manual for Assessing Safety Hardware (MASH)* (1). The single-slope barrier system needed to be compliant to Test Level 4 (TL-4) criteria of *MASH*. For each of the three barrier systems, only essential tests were to be performed.

#### 1.2 OBJECTIVE AND SCOPE

The objective of this project was to evaluate the crash performance of two F-shape concrete barrier systems under *MASH* TL-3 criteria by performing *MASH* Test 3-11 for each system. Additionally, a single-slope barrier system was evaluated under *MASH* TL-4 criteria by performing *MASH* Test 4-12. More specifically, the following three systems were tested.

- 1. 32-inch tall F-shape barrier with drainage scuppers and pin-and-loop connections, restrained to underlying asphalt pavement with anchoring pins. The barrier was installed adjacent to 1V:1.5H slope with a 1-ft offset on the field side. This system was evaluated by performing *MASH* Test 3-11.
- 2. The above-mentioned system while unpinned and free-standing on concrete. This system was also evaluated by performing *MASH* Test 3-11.
- 3. 42-inch tall single slope barrier with grouted rebar-grid connections and drainage scuppers, embedded 4 inches in asphalt for an above-grade height of 38 inches. This barrier system was evaluated by performing *MASH* Test 4-12.

Prior to crash testing, TTI researchers evaluated the reinforcement details, barrier connections, barrier anchoring, barrier embedment, and scupper height of the barrier systems provided by WSDOT. New reinforcement details that incorporated the drainage scuppers were developed by the researchers. The design details of the barriers are presented in this report. Also presented are detailed documentation of the crash tests and results, and an assessment of the performance of each barrier per the *MASH* evaluation criteria.

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# Chapter 2. TEST REQUIREMENTS AND EVALUATION CRITERIA

#### 2.1 CRASH TEST PERFORMED / MATRIX

Table 2.1 shows the test conditions and evaluation criteria for *MASH* TL-3 and TL-4 longitudinal barriers.

Table 2.1. Test Conditions and Evaluation Criteria Specified for MASH TL-3 and TL-4 Longitudinal Barriers.

Test Article	Test	Test	Impact Conditions		Evaluation	
	<b>Designation</b> Vehicle		Speed	Angle	Criteria	
	3-10 4-10	1100C	62 mi/h	25°	A, D, F, H, I	
Longitudinal Barrier	3-11 4-11	2270P	62 mi/h	25°	A, D, F, H, I	
	4-12	10000S	56 mi/h	15°	A, D, G	

It should be noted that *MASH* TL-3 criteria also requires testing with the small passenger car (1100C vehicle). This test, however, is not critical for all three systems due to the successfully performed Test 3-10 for similar systems in the past (2,3,4). Furthermore, the lighter small car will not impart a greater load into the barrier systems and their restraint mechanisms in comparison to the heavier pickup truck of Test 3-11 (for the F-shape barriers) or the single unit truck of Test 4-12 (for the single slope barrier).

Similarly, Test 4-11 with the pickup truck is not critical for the single slope barrier due to the successfully performed past testing of a similar single slope barrier (5). It is also not expected to impart greater lateral load in the barrier compared to Test 4-12 with the single unit truck.

Thus, only Test 3-11 will be performed for the F-shape barriers and Test 4-12 for the single slope barrier. The crash tests and data analysis procedures were in accordance with guidelines presented in *MASH*. Chapter 3 presents brief descriptions of these procedures.

#### 2.2 EVALUATION CRITERIA

The appropriate safety evaluation criteria from Tables 2-2 and 5-1 of *MASH* were used to evaluate the crash tests reported herein. The test conditions and evaluation criteria required for *MASH* TL-3 and TL-4 are listed in Table 2.1, and the substance of the evaluation criteria in Table 2.2. An evaluation of the crash test results is presented in detail under the section Assessment of Test Results.

Table 2.2. Evaluation Criteria Required for MASH TL-3 and TL-4 Longitudinal Barriers.

Evaluation Factors	Evaluation Criteria	
Structural Adequacy	A. Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.	
	<ul> <li>Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present undue hazard to other traffic, pedestrians, or personnel in a work zone.</li> <li>Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.2.2 and Appendix E of MASH.</li> </ul>	3-10, 4-10, 3-11, 4-11, 4-12
Occupant Risk	F. The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.	3-10, 4-10, 3-11, 4-11
	G. It is preferable, although not essential, that the vehicle remain upright during and after the collision.	4-12
	H. Occupant impact velocities (OIV) should satisfy the following limits: Preferred value of 30 ft/s, or maximum allowable value of 40 ft/s.	3-10, 4-10, 3-11, 4-11
	I. The occupant ridedown accelerations should satisfy the following: Preferred value of 15.0 g, or maximum allowable value of 20.49 g.	3-10, 4-10, 3-11, 4-11

# Chapter 3. TEST CONDITIONS

# 3.1 TEST FACILITY

The full-scale crash tests reported herein were performed at Texas A&M Transportation Institute (TTI) Proving Ground, an International Standards Organization (ISO)/International Electrotechnical Commission (IEC) 17025-accredited laboratory with American Association for Laboratory Accreditation (A2LA) Mechanical Testing Certificate 2821.01. The full-scale crash tests were performed according to TTI Proving Ground quality procedures, and according to the *MASH* guidelines and standards.

The test facilities of the TTI Proving Ground are located on the Texas A&M University System RELLIS Campus, which consists of a 2000-acre complex of research and training facilities situated 10 miles northwest of the flagship campus of Texas A&M University. The site, formerly a United States Army Air Corps base, has large expanses of concrete runways and parking aprons well suited for experimental research and testing in the areas of vehicle performance and handling, vehicle-roadway interaction, durability and efficacy of highway pavements, and evaluation of roadside safety hardware and perimeter protective devices. The site selected for construction and testing of the barriers was along the edge of an out-of-service apron. The apron consists of an unreinforced jointed-concrete pavement in 12.5-ft × 15-ft blocks nominally 6 inches deep. The aprons were built in 1942, and the joints have some displacement, but are otherwise flat and level.

#### 3.2 VEHICLE TOW AND GUIDANCE SYSTEM

Each test vehicle was towed into the test installation using a steel cable guidance and reverse tow system. A steel cable for guiding the test vehicle was tensioned along the path, anchored at each end, and threaded through an attachment to the front wheel of the test vehicle. An additional steel cable was connected to the test vehicle, passed around a pulley near the impact point, through a pulley on the tow vehicle, and then anchored to the ground such that the tow vehicle moved away from the test site. A 2:1 speed ratio between the test and tow vehicle existed with this system. Just prior to impact with the installation, the test vehicle was released and ran unrestrained. The vehicle remained freewheeling (i.e., no steering or braking inputs) until it cleared the immediate area of the test site, after which the brakes were activated, if needed, to bring the test vehicle to a safe and controlled stop.

# 3.3 DATA ACQUISITION SYSTEMS

# 3.3.1 Vehicle Instrumentation and Data Processing

Each test vehicle was instrumented with a self-contained, on-board data acquisition system. The signal conditioning and acquisition system is a 16-channel, Tiny Data Acquisition System (TDAS) Pro produced by Diversified Technical Systems, Inc. The accelerometers, which measure the x, y, and z axis of vehicle acceleration, are strain gauge type with linear millivolt output proportional to acceleration. Angular rate sensors, measuring vehicle roll, pitch, and yaw rates, are ultra-small, solid state units designed for crash test service. The TDAS Pro hardware

and software conform to the latest SAE J211, Instrumentation for Impact Test. Each of the 16 channels is capable of providing precision amplification, scaling, and filtering based on transducer specifications and calibrations. During the test, data are recorded from each channel at a rate of 10,000 samples per second with a resolution of one part in 65,536. Once data are recorded, internal batteries back these up inside the unit should the primary battery cable be severed. Initial contact of the pressure switch on the vehicle bumper provides a time zero mark and initiates the recording process. After each test, the data are downloaded from the TDAS Pro unit into a laptop computer at the test site. The Test Risk Assessment Program (TRAP) software then processes the raw data to produce detailed reports of the test results.

Each of the TDAS Pro units is returned to the factory annually for complete recalibration and all instrumentation used in the vehicle conforms to all specifications outlined by SAE J211. All accelerometers are calibrated annually by means of an ENDEVCO® 2901, precision primary vibration standard. This standard and its support instruments are checked annually and receive a National Institute of Standards Technology (NIST) traceable calibration. The rate transducers used in the data acquisition system receive a calibration via a Genisco Rate-of-Turn table. The subsystems of each data channel are also evaluated annually, using instruments with current NIST traceability, and the results are factored into the accuracy of the total data channel, per SAE J211. Calibrations and evaluations are also made any time data are suspect. Acceleration data are measured with an expanded uncertainty of  $\pm 1.7$  percent at a confidence factor of 95 percent (k=2).

TRAP uses the data from the TDAS Pro to compute occupant/compartment impact velocities, time of occupant/compartment impact after vehicle impact, and the highest 10-millisecond (ms) average ridedown acceleration. TRAP calculates change in vehicle velocity at the end of a given impulse period. In addition, maximum average accelerations over 50-ms intervals in each of the three directions are computed. For reporting purposes, the data from the vehicle-mounted accelerometers are filtered with an SAE Class 180-Hz low-pass digital filter, and acceleration versus time curves for the longitudinal, lateral, and vertical directions are plotted using TRAP.

TRAP uses the data from the yaw, pitch, and roll rate transducers to compute angular displacement in degrees at 0.0001-s intervals, then plots yaw, pitch, and roll versus time. These displacements are in reference to the vehicle-fixed coordinate system with the initial position and orientation of the vehicle-fixed coordinate systems being initial impact. Rate of rotation data is measured with an expanded uncertainty of  $\pm 0.7$  percent at a confidence factor of 95 percent (k=2).

# 3.3.2 Anthropomorphic Dummy Instrumentation

According to *MASH*, use of a dummy in the 2270P vehicle is optional, and no dummy was used in the tests. *MASH* does not recommend or require use of a dummy in the 10000S vehicle, and thus no dummy was used.

#### 3.3.3 Photographic Instrumentation Data Processing

Photographic coverage of each test included three digital high-speed cameras:

- One overhead with a field of view perpendicular to the ground and directly over the impact point;
- One placed behind the installation at an angle; and
- A third placed to have a field of view parallel to and aligned with the installation at the downstream end.

A flashbulb on the impacting vehicle was activated by a pressure-sensitive tape switch to indicate the instant of contact with the barrier. The flashbulb was visible from each camera. The video files from these digital high-speed cameras were analyzed to observe phenomena occurring during the collision and to obtain time-event, displacement, and angular data. A digital camera recorded and documented conditions of each test vehicle and the installation before and after the test.

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# Chapter 4. MASH TEST 3-11 ON PINNED F-SHAPE BARRIER WITH DRAINAGE SCUPPERS

#### 4.1 TEST ARTICLE AND INSTALLATION DETAILS

The installation consisted of eight reinforced F-shape concrete barriers, each 12.5 ft long, with an approximately 1-inch wide gap between barriers, for a total installation length of approximately 100 ft-7 inches. The barriers were placed on a 4-inch thick layer of TxDOT Type D Asphalt, which was in turn placed on a 12-inch layer of compacted crushed limestone base. The barrier system was installed adjacent to 1V:1.5H slope with a 1-ft offset from the top edge of the slope to the field-side toe of the barrier.

Adjacent barrier segments were connected with a 1-inch diameter pin that extended through three loops cast into each end of each barrier. Each barrier segment had two 6-inch tall drainage scuppers at the bottom, and three inclined slots on each side to receive anchor pins. The barrier segments were anchored to the underlying pavement by installing a 48-inch long, 1½-inch diameter anchor pin in each of the three inclined slots from the impact (traffic) side of the barrier segments. Plate washers welded to the top of the anchoring pins were installed flush to the toe of the barrier segments.

Figure 4.1 presents overall information on the pinned F-shape barrier with drainage scuppers, and Figure 4.2 provides photographs of the installation. Appendix A1 provides further details of the pinned F-shape barrier with drainage scuppers.

# 4.2 DESIGN MODIFICATIONS DURING TESTS

No modification was made to the installation during the testing phase.

#### 4.3 MATERIAL SPECIFICATIONS

The specified minimum compressive strength of the concrete used in the barrier was 5000 psi. A total of eight barriers were constructed for this test. On the day of the test, barriers 1 through 4 had an average compressive strength of 5957 psi, and barriers 5 through 8 had an average compressive strength of 6555 psi. Soil compaction of the base was 100%.

Appendix B provides material certification documents for the materials used to install/construct the pinned F-shape barrier with drainage scuppers.

#### 4.4 TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS

MASH Test 3-11 involves a 2270P vehicle weighing 5000 lb  $\pm 110$  lb impacting the target critical impact point (CIP) of the barrier at an impact speed of 62 mi/h  $\pm 2.5$  mi/h and an angle of 25°  $\pm 1.5$ °. The CIP for MASH Test 3-11 on the pinned F-shape barrier was 4.3 ft  $\pm 1$  ft upstream of the center of the joint between segments 3 and 4. Figure 4.3 depicts the target impact setup.

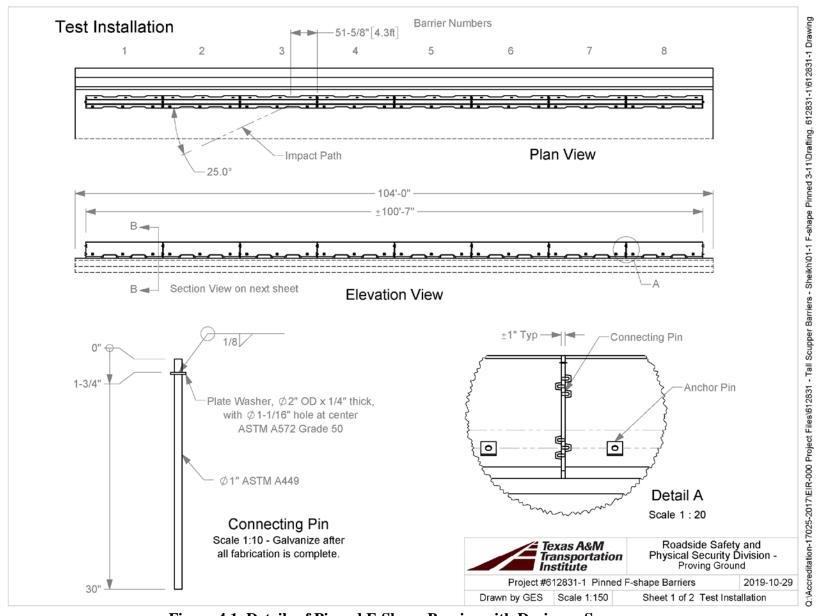


Figure 4.1. Details of Pinned F-Shape Barrier with Drainage Scuppers.

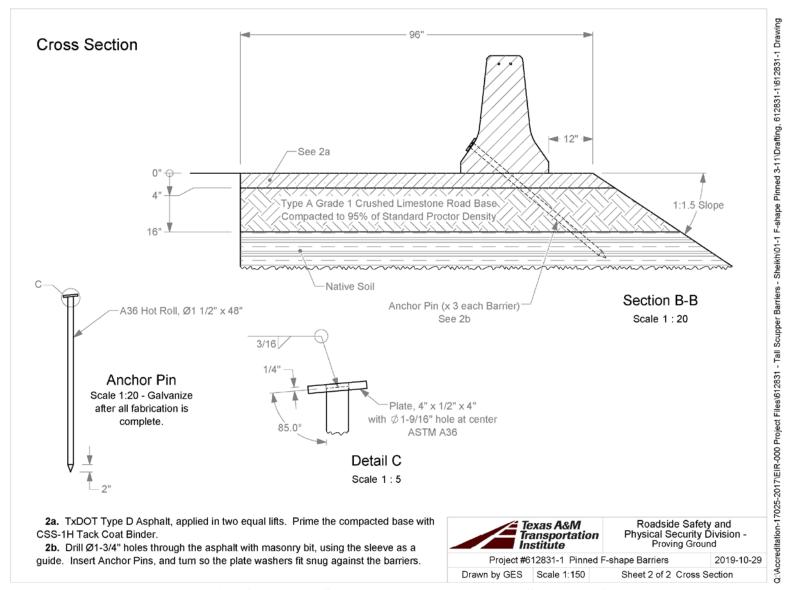


Figure 4.1. Details of Pinned F-Shape Barrier with Drainage Scuppers (Continued).



Figure 4.2. Pinned F-Shape Barrier with Drainage Scuppers prior to Testing.



Figure 4.3. Barrier/Test Vehicle Geometrics for Test No. 612831-01-1.

The 2014 RAM 1500 pickup truck used in the test weighed 5000 lb, and the actual impact speed and angle were 63.2 mi/h and 25.5°. The actual impact point was 4.4 ft upstream of the center of the joint between segments 3 and 4. Minimum target impact severity (IS) was 106 kip-ft, and actual IS was 124 kip-ft.

#### 4.5 WEATHER CONDITIONS

The test was performed on the morning of January 7, 2020. Weather conditions at the time of testing were as follows: wind speed: 5 mi/h; wind direction: 49° (vehicle was traveling at magnetic heading of 335°); temperature: 58°F; relative humidity: 42 percent.

#### 4.6 TEST VEHICLE

Figure 4.4 shows the 2014 RAM 1500 pickup truck used for the crash test. The vehicle's test inertia weight was 5000 lb, and its gross static weight was 5000 lb. The height to the lower edge of the vehicle bumper was 11.75 inches, and height to the upper edge of the bumper was 27.00 inches. The height to the vehicle's center of gravity was 28.5 inches. Tables C.1 and C.2 in Appendix C1 give additional dimensions and information on the vehicle. The vehicle was directed into the installation using a cable reverse tow and guidance system. It was released to be freewheeling and unrestrained just prior to impact.

# 4.7 TEST DESCRIPTION

Table 4.1 lists events that occurred during Test No. 612831-01-1. Figures C.1 and C.2 in Appendix C2 present sequential photographs during the test.

For longitudinal barriers, it is desirable that the vehicle redirects and exits the barrier within the exit box criteria (not less than 32.8 ft downstream from loss of contact for cars and pickups). The test vehicle exited within the exit box criteria defined in *MASH*. Brakes on the vehicle were applied at 2.4 s after impact, and the vehicle came to rest 170 ft downstream of the impact and 32 ft toward traffic lanes.





Figure 4.4. Test Vehicle before Test No. 612831-01-1.

**Table 4.1. Events during Test No. 612831-01-1.** 

TIME (s)	EVENTS
0.000	Vehicle contacts barrier while traveling at 63.2 mi/h and 25.5°
0.015	Left front tire lifts off pavement, climbs barrier
0.029	Barrier segments 3 and 4 begin to slide toward protected side
0.043	Vehicle begins to redirect
0.089	Right front tire lifts off pavement
0.118	Right rear tire lifts off pavement
0.216	Vehicle traveling parallel with barrier
0.230	Left rear bumper and quarter panel impact barrier 3
0.467	Vehicle loses contact with barrier while traveling at 50.6 mi/h,
	trajectory of 3.0° and heading of 9.7°.
0.535	Front left tire contacts pavement

# 4.8 DAMAGE TO TEST INSTALLATION

Figure 4.5 shows the damage to the barrier. Table 4.2 shows transverse barrier movement at the joints, and Table 4.3 shows how much the anchoring pins protruded after the test. Barriers 1 through 3 each moved downstream 1½-inches. No movement was noted at barriers 6 through 8. There was some spalling at the downstream scupper of barrier 3, and there was some damage to the toes of barriers 3 and 4 on the traffic side. There was a crack running approximately vertical radiating from the corner of the upstream scupper on barrier 4, and the upper field side upstream corner of barrier 5 was broken off approximately 4 inches down from the top of barrier and 4 inches downstream barrier.

Working width\* was 39.6 inches, and height of working width was 32.0 inches. Maximum dynamic deflection during the test was 21.7 inches, and maximum permanent deformation was 10.0 inches.



Figure 4.5. Barrier after Test No. 612831-01-1.

TR No. 612831-01 15 2020-05-07

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<sup>\*</sup> Per *MASH*, "The working width is the maximum dynamic lateral position of any major part of the system or vehicle. These measurements are all relative to the pre-impact traffic face of the test article." In other words, working width is the total barrier width plus the maximum dynamic intrusion of any portion of the barrier or test vehicle past the field side edge of the barrier.

Table 4.2. Barrier Segment Movement for Test No. 612831-01-1.

Joint	Movement	Direction
1-2	½ inch	Field side
2-3	1½ inches	Traffic side
3-4	10 inches	Field side
4-5	3 inches	Field side

Table 4.3. Barrier Segment Pin Lift for Test No. 612831-01-1.

Barrier	Pin Lift
2	1½ inches
3	2 inches – 3 inches
4	2 inches – 7 inches (head pulled off one pin and bent over 90°)
5	1 inch $-3-\frac{1}{2}$ inches

# 4.9 DAMAGE TO TEST VEHICLE

Figure 4.6 shows the damage sustained by the vehicle. The front bumper, hood, radiator and support, grill, left front fender, left front tire and rim, left frame rail, left front control arm, sway bar, tie rod, left front floor pan, left front door, left rear tire and rim, and rear bumper were damaged. Maximum exterior crush to the vehicle was 14.0 inches in the side plane at the left front corner at bumper height. Maximum occupant compartment deformation was 3.0 inches in the left front firewall. Figure 4.7 shows the interior of the vehicle. Tables C.3 and C.4 in Appendix C1 provide exterior crush and occupant compartment measurements.





Figure 4.6. Test Vehicle after Test No. 612831-01-1.



Figure 4.7. Interior of Test Vehicle after Test No. 612831-01-1.

# 4.10 OCCUPANT RISK FACTORS

Data from the accelerometers were digitized for evaluation of occupant risk, and the results are shown in Table 4.4. Figure C.3 in Appendix C3 shows the vehicle angular displacements, and Figures C.4 through C.6 in Appendix C4 show acceleration versus time traces. Figure 4.8 summarizes pertinent information from the test.

Table 4.4. Occupant Risk Factors for Test No. 612831-01-1.

Occupant Risk Factor	Value	Time
Occupant Impact Velocity (OIV)		
Longitudinal	18.4 ft/s	at 0.1053 s on left side of interior
Lateral	21.0 ft/s	at 0.1033 s off feft side of filterior
Occupant Ridedown Accelerations		
Longitudinal	7.8 g	0.2186 – 0.2286 s
Lateral	9.7 g	0.2390 – 0.2490 s
Theoretical Head Impact Velocity (THIV)	8.4 m/s	at 0.1018 s on left side of interior
Acceleration Severity Index (ASI)	1.44	0.0575 – 0.1075 s
Maximum 50-ms Moving Average		
Longitudinal	-8.7 g	0.0332 - 0.0832  s
Lateral	11.1 g	0.0336 – 0.0836 s
Vertical	-5.2 g	1.2633 – 1.3133 s
Maximum Roll, Pitch, and Yaw Angles		
Roll	27°	0.6717 s
Pitch	17°	0.7280 s
Yaw	43°	1.5000 s

Dummy...... No dummy Gross Static ...... 5000 lb

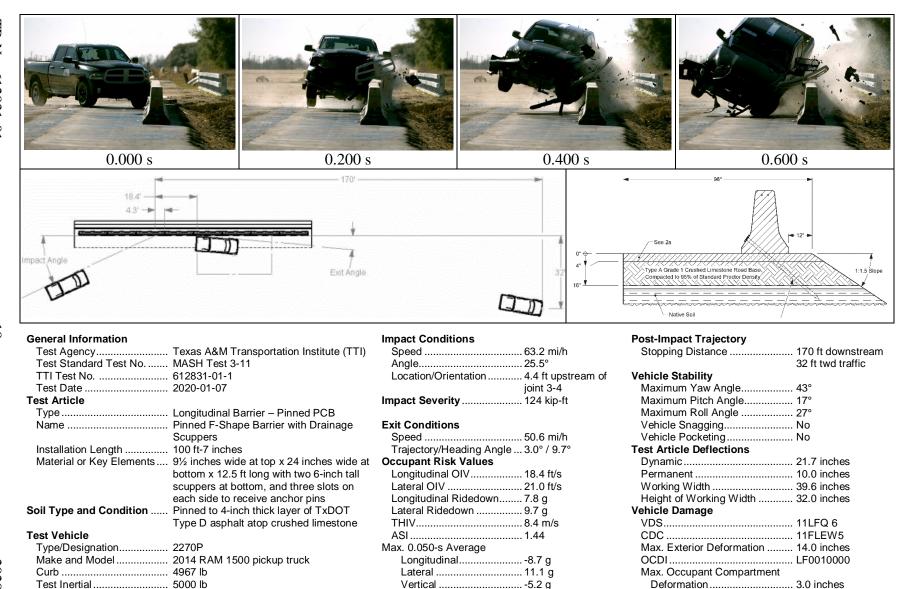


Figure 4.8. Summary of Results for MASH Test 3-11 on Pinned F-Shape Barrier with Drainage Scuppers.

# Chapter 5. MASH TEST 3-11 ON FREE-STANDING F-SHAPE BARRIER WITH DRAINAGE SCUPPERS

#### 5.1 TEST ARTICLE AND INSTALLATION DETAILS

The installation consisted of sixteen reinforced F-shape concrete barrier segments, each 12.5 ft long, with an approximately 1-inch wide gap between the segments, for a total installation length of approximately 201 ft-3 inches. Adjacent barrier segments were connected with a 1-inch diameter pin that extended through three loops cast into each end of the segments. The barrier segments were 9½ inches wide at the top and 24 inches wide at the bottom. Each segment had two 6-inch tall drainage scuppers at the bottom and three inclined slots on each side to receive anchor pins. No anchor pins were used for this test and the barrier was installed unrestrained on concrete pavement.

Figure 5.1 presents overall information on the free-standing F-shape barrier with drainage scuppers, and Figure 5.2 provides photographs of the installation. Appendix A2 provides further details of the free-standing F-shape barrier with drainage scuppers.

#### 5.2 DESIGN MODIFICATIONS DURING TESTS

No modification was made to the installation during the testing phase.

#### 5.3 MATERIAL SPECIFICATIONS

The specified minimum compressive strength of the concrete used in the barrier was 5000 psi. A total of sixteen barriers were constructed. On the day of the test, barriers 5 through 8 had an average compressive strength of 5813 psi, barriers 1-4, 15 and 16 had an average compressive strength of 5957 psi, and barriers 9 through 14 had an average compressive strength of 6555.

Appendix B provides material certification documents for the materials used to install or construct the free-standing F-shape barrier.

#### 5.4 TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS

MASH Test 3-11 involves a 2270P vehicle weighing 5000 lb  $\pm 110$  lb impacting the target CIP of the barrier at an impact speed of 62 mi/h  $\pm 2.5$  mi/h and an angle of  $25^{\circ} \pm 1.5^{\circ}$ . The CIP for MASH Test 3-11 on the free-standing F-shape barrier with drainage scuppers was 4.3 ft  $\pm 1$  ft upstream of the center of the joint between segments 6 and 7. Figure 5.3 depicts the target impact setup.

The 2014 RAM 1500 pickup truck used in the test weighed 5054 lb, and the actual impact speed and angle were 61.5 mi/h and 24.8°. The actual impact point was 4.3 ft upstream of the center of the joint between segments 6 and 7. Minimum target IS was 106 kip-ft, and actual IS was 112 kip-ft.

TR No. 612831-01 19 2020-05-07

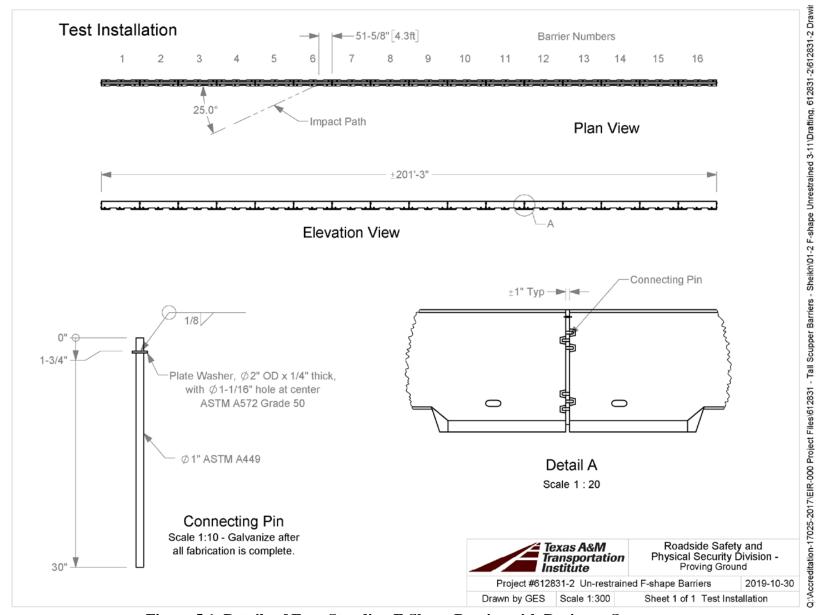


Figure 5.1. Details of Free-Standing F-Shape Barrier with Drainage Scuppers.



Figure 5.2. Free-Standing F-Shape Barrier with Drainage Scuppers Prior to Testing.





(photos taken after a rain event prior to the test)

Figure 5.3. Barrier/Test Vehicle Geometrics for Test No. 612831-01-2.

#### 5.5 WEATHER CONDITIONS

The test was performed on the afternoon of January 9, 2020. Weather conditions at the time of testing were as follows: wind speed: 9 mi/h; wind direction: 170° (vehicle was traveling at magnetic heading of 0°); temperature: 70°F; relative humidity: 99 percent.

#### 5.6 TEST VEHICLE

Figure 5.4 shows the 2014 RAM 1500 pickup truck used for the crash test. The vehicle's test inertia weight was 5054 lb, and its gross static weight was 5054 lb. The height to the lower edge of the vehicle bumper was 11.75 inches, and height to the upper edge of the bumper was 27.00 inches. The height to the vehicle's center of gravity was 28.75 inches. Tables D.1 and D.2 in Appendix D1 give additional dimensions and information on the vehicle. The vehicle was directed into the installation using a cable reverse tow and guidance system. The vehicle was released to be freewheeling and unrestrained just prior to impact.





Figure 5.4. Test Vehicle before Test No. 612831-01-2.

#### 5.7 TEST DESCRIPTION

Table 5.1 lists events that occurred during Test No. 612831-01-2. Figures D.1 and D.2 in Appendix D2 present sequential photographs during the test.

TIME (s)	EVENTS
0.000	Vehicle contacts barrier while traveling at 61.5 mi/h and 24.8°
0.012	Left front tire lifts off pavement
0.020	Barrier 6 begins to deflect toward field side
0.036	Barrier 7 begins to deflect toward field side
0.053	Vehicle begins to redirect
0.096	Right front tire lifts off pavement
0.128	Right rear tire lifts off pavement
0.218	Vehicle traveling parallel with barrier
0.273	Left rear quarter panel contacts barrier 7
0.404	Left front tire contacts pavement
0.492	Vehicle loses contact with barrier while traveling at 49.6 mi/h, trajectory
	of 6.0°, and heading of 21.6°.
0.966	Right front tire contacts pavement
1.104	Right rear tire contacts pavement

**Table 5.1. Events during Test No. 612831-01-2.** 

For longitudinal barriers, it is desirable that the vehicle redirects and exits the barrier within the exit box criteria (not less than 32.8 ft downstream from loss of contact for cars and pickups). The test vehicle exited within the exit box criteria defined in *MASH*. Brakes on the vehicle were applied after the vehicle exited the test site, and the vehicle came to rest 200 ft downstream of the impact and 28 ft toward field side.

#### 5.8 DAMAGE TO TEST INSTALLATION

Figures 5.5 and 5.6 show the damage to the barrier. Barriers 1 and 2 moved downstream 7 inches, and barrier 3 moved downstream 7½ inches. Barrier 4 moved downstream 8 inches, and its upstream end moved 2 inches toward the field side. Barriers 10 and 11 moved upstream 2 inches, barrier 12 moved upstream 1½ inches, barrier 13 moved upstream 1 inch, and barriers 14 and 15 moved upstream ½ inch. Barrier 16 did not move. Table 5.2 shows transverse movement at the joints.

There were multiple cracks in barrier 7, but it remained intact. There was minor damage to the lower corners of both barriers on the field side at joint 4-5. There was significant damage to both corners of both barriers on the traffic side at joint 6-7, and joint 7-8. There was also significant damage to the field side corners of both barriers at joints, 5-6, 7-8, 8-9 and 9-10. Barriers 1-3 and 11-16 were not damaged during this test (barriers 15 and 16 had minor damage from use in a previous test).

Working width\* was 86.9 inches, and height of working width was 3 inches. Maximum dynamic deflection during the test was 63.0 inches, and maximum permanent deformation was 63.0 inches.



Figure 5.5. Barrier after Test No. 612831-01-2.

<sup>\*</sup> Per MASH, "The working width is the maximum dynamic lateral position of any major part of the system or vehicle. These measurements are all relative to the pre-impact traffic face of the test article." In other words, working width is the total barrier width plus the maximum dynamic intrusion of any portion of the barrier or test vehicle past the field side edge of the barrier.



Figure 5.6. Field Side of Barrier after Test No. 612831-01-2.

Table 5.2. Barri	ier Segment Mo	vement for Tes	t No. 612831-01-2.
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Joint	Movement	Direction
4-5	3 inches	Field side
5-6	30 inches	Field side
6-7	63 inches	Field side
7-8	60 inches	Field side
8-9	30½ inches	Field side
9-10	4 inches	Traffic side
10-11	5 inches	Traffic side

# 5.9 DAMAGE TO TEST VEHICLE

Figure 5.7 shows the damage sustained by the vehicle. The front bumper, hood, grill, left front fender, left front tire and rim, left upper and lower control arms, left frame rail, left front floor pan, left front and rear doors, left rear cab corner, left rear exterior bed, and rear bumper were damaged. Maximum exterior crush to the vehicle was 15.0 inches in the side plane at the left front corner at bumper height. Maximum occupant compartment deformation was 1.5 inches

in the left front fire wall. Figure 5.8 shows the interior of the vehicle. Tables D.3 and D.4 in Appendix D1 provide exterior crush and occupant compartment measurements.



Figure 5.7. Test Vehicle after Test No. 612831-01-2.



Figure 5.8. Interior of Test Vehicle after Test No. 612831-01-2.

#### 5.10 OCCUPANT RISK FACTORS

Data from the accelerometers were digitized for evaluation of occupant risk, and the results are shown in Table 5.3. Figure D.3 in Appendix D3 shows the vehicle angular displacements, and Figures D.4 through D.9 in Appendix D4 show acceleration versus time traces. Figure 5.8 summarizes pertinent information from the test.

Table 5.3. Occupant Risk Factors for Test No. 612831-01-2.

Occupant Risk Factor	Value	Time
Occupant Impact Velocity (OIV)		
Longitudinal	1.3 ft/s	at 0.1745 s on left side of interior
Lateral	8.5 ft/s	at 0.1743 s on left side of filterior
Occupant Ridedown Accelerations		
Longitudinal	2.0 g	0.2488 - 0.2588  s
Lateral	2.6 g	0.2538 – 0.2638 s
Theoretical Head Impact Velocity (THIV)	2.6 m/s	at 0.1744 s on left side of interior
Acceleration Severity Index (ASI)	0.97	0.0020 – 0.0520 s
Maximum 50-ms Moving Average		
Longitudinal	-1.4 g	0.2327 – 0.2827 s
Lateral	4.7 g	0.0000 – 0.0500 s
Vertical	2.5 g	0.1258 - 0.1758  s
Maximum Roll, Pitch, and Yaw Angles		
Roll	20°	0.2919 s
Pitch	14°	0.4154 s
Yaw	19°	1.1800 s

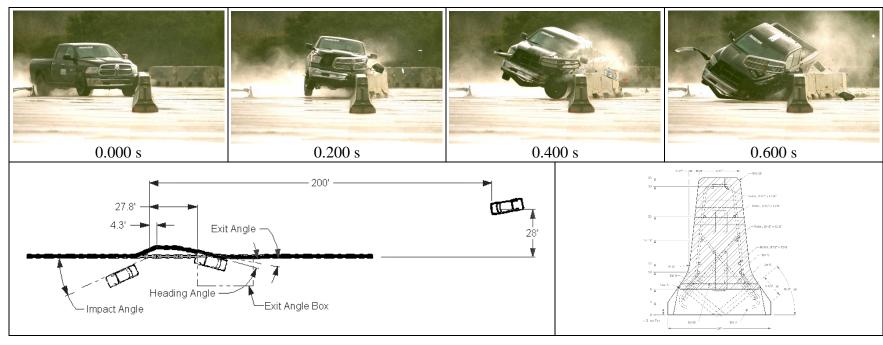


Figure 5.9. Summary of Results for MASH Test 3-11 on Free-Standing F-Shape Barrier with Drainage Scuppers.

# Chapter 6. MASH TEST 4-12 ON EMBEDDED SINGLE-SLOPE BARRIER WITH DRAINAGE SCUPPERS

#### 6.1 TEST ARTICLE AND INSTALLATION DETAILS

The installation consisted of twelve reinforced concrete single-slope barrier segments, each 12.5 ft long, with the ends butted together, for a total installation length of 150 ft. The barrier segments were 8 inches wide at the top and 24 inches wide at the base, with an equal constant slope on both the traffic and field sides. Each segment had three 10-inch tall drainage scuppers at the bottom. The barrier segments were 42 inches tall and were placed on 12 inches of compacted limestone base, set 4 inches below grade. Asphalt was then placed on the base in two 2-inch thick lifts. Due to the 4-inch embedment, the above grade barrier height was 38 inches and the above grade drainage scupper height was 6 inches.

The barrier segments contained vertical slots cast into each end to receive rebar-grids used to reinforce the joints. These slots were filled with grout after installation of the grids.

Figure 6.1 presents overall information on the embedded single-slope barrier with drainage scuppers, and Figure 6.2 provides photographs of the installation. Appendix A.3 provides further details of the embedded single-slope barrier with drainage scuppers.

#### 6.2 DESIGN MODIFICATIONS DURING TESTS

No modification was made to the installation during the testing phase.

#### 6.3 MATERIAL SPECIFICATIONS

The specified minimum compressive strength of the concrete used in the barrier was 4000 psi. Twelve barriers were constructed. Barriers 1 through 6 had an average compressive strength of 4367 psi, and barriers 7 through 12 had an average compressive strength of 5710 psi. Non-shrink grout was used between the joints of the barriers and its average compressive strength on the day of testing was 7620 psi. Soil compaction of the base was 100%.

Appendix B provides material certification documents for the materials used to install or construct the embedded single-slope barrier with drainage scuppers.

#### 6.4 TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS

MASH Test 4-12 involves a 10000S vehicle weighing 22,000 lb  $\pm 660$  lb impacting the CIP of the barrier at an impact speed of 56 mi/h  $\pm 2.5$  mi/h and an angle of 15°  $\pm 1.5$ °. The CIP for this test was 5 ft  $\pm 1$  ft upstream of the center of the joint between barrier segments 3 and 4. Figure 6.3 depicts the target impact setup.

TR No. 612831-01 29 2020-05-07

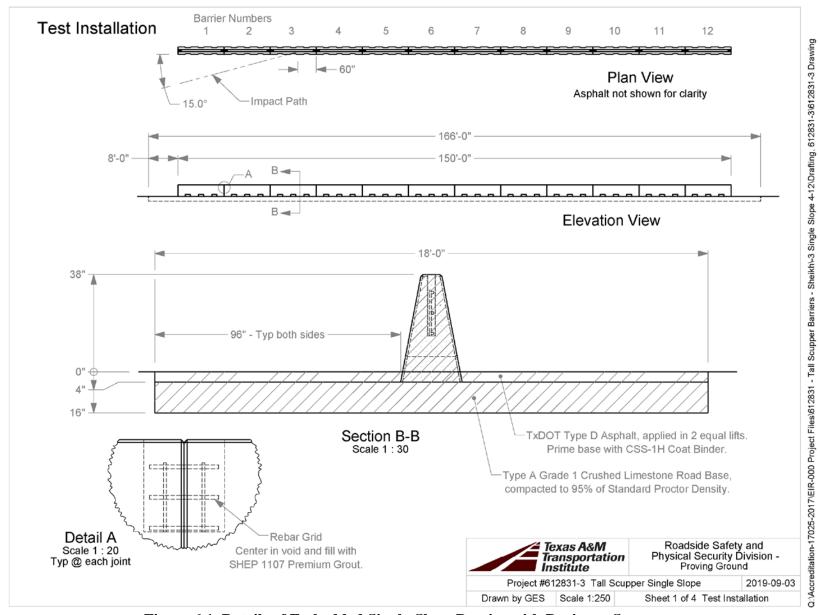


Figure 6.1. Details of Embedded Single-Slope Barrier with Drainage Scuppers.



Figure 6.2. Embedded Single-Slope Barrier with Drainage Scuppers Prior to Testing.





Figure 6.3. Embedded Single-Slope Barrier with Drainage Scuppers/Test Vehicle Geometrics for Test No. 612831-01-3.

The 2012 International 4300 single-unit truck used in the test weighed 22,370 lb, and the actual impact speed and angle were 57.7 mi/h and 16.7°. The actual impact point was 6.5 ft upstream of the center of the joint between barriers 3 and 4. Minimum target IS was 142 kip-ft, and actual IS was 206 kip-ft.

The point of impact in the test was 0.5 ft outside the target impact tolerance, and the impact angle was 0.2° greater than the tolerance specified in *MASH*. Both these tolerance exceedances, however, did not adversely affect the evaluation of the barrier for Test 4-12 compliance. The target impact point was selected to be 5-ft upstream of a barrier joint for consistency with past testing practice. The offset from the joint itself was not considered to be critical for evaluating the barrier system. The 0.2° exceedance in impact angle also did not adversely affect the evaluation of the barrier since it imparted greater impact energy to the barrier. Successful performance of the barrier with slightly higher impact angle implies that the barrier is able to perform acceptably for impact within the *MASH* impact angle tolerances.

#### 6.5 WEATHER CONDITIONS

The test was performed on the morning of December 17, 2019. Weather conditions at the time of testing were as follows: wind speed: 14 mi/h; wind direction: 337° (vehicle was traveling at magnetic heading of 345°); temperature: 44°F; relative humidity: 71 percent.

#### 6.6 TEST VEHICLE

Figure 6.4 shows the 2012 International 4300 single-unit truck used for the crash test. The vehicle's test inertia weight was 22,370 lb and its gross static weight was 22,370 lb. The height to the lower edge of the vehicle bumper was 18.25 inches, and height to the upper edge of the bumper was 33.25 inches. The height to the vehicle's ballast center of gravity was 62.0 inches. Table E.1 in Appendix E1 gives additional dimensions and information on the vehicle. The vehicle was directed into the installation using a cable reverse tow and guidance system and was released to be freewheeling and unrestrained just prior to impact.





Figure 6.4. Test Vehicle before Test No. 612831-01-3.

#### 6.7 TEST DESCRIPTION

Table 6.1 lists events that occurred during Test No. 612831-01-3. Figures E.1 and E.2 in Appendix E2 present sequential photographs during the test.

TIME (s)	EVENTS	
0.000	Vehicle contacts barrier while traveling at 57.7 mi/h and 16.7°	
0.023	Vehicle begins to redirect	
0.108	Right front tire leaves the pavement	
0.221	Right rear tire leaves the pavement	
0.251	Rear left corner of vehicle contacts barrier	
0.252	Vehicle traveling parallel with test article	
0.312	Left rear tire leaves the pavement	
0.898	0.898 Right front tire lands on the pavement	
1.701	Right rear tire lands on the pavement	

**Table 6.1. Events during Test No. 612831-01-3.** 

For longitudinal barriers, it is desirable that the vehicle redirects and exits the barrier within the exit box criteria (not less than 65.6 ft downstream from loss of contact for heavy vehicles). The test vehicle exited within the exit box criteria defined in *MASH*. Brakes on the vehicle were applied at 2.4 s after impact. After loss of contact with the barrier, the vehicle came to rest 218 ft downstream of the impact and 32 ft toward the field side.

#### 6.8 DAMAGE TO TEST INSTALLATION

Figure 6.5 shows the damage to the barrier. Barrier segment 3 had a crack on the traffic side at top near each end, and there were scrapes and gouges up to one inch deep on barrier segments 3 and 4. There was no visible movement in the asphalt, and no measurable static deflection.

Working width\* was 70.8 inches, and height of working width was 153.0 inches. Maximum dynamic deflection during the test was 1.8 inches, and no permanent deformation was observed. Maximum lateral intrusion of the passenger cab, measured from the pre-impact trafficside face of the barrier was 26.3 inches at a height of 87.8 inches.



Figure 6.5. Embedded Single-Slope Barrier with Scuppers after Test No. 612831-01-3.

TR No. 612831-01 34 2020-05-07

<sup>\*</sup> Per MASH, "The working width is the maximum dynamic lateral position of any major part of the system or vehicle. These measurements are all relative to the pre-impact traffic face of the test article." In other words, working width is the total barrier width plus the maximum dynamic intrusion of any portion of the barrier or test vehicle past the field side edge of the barrier.

#### 6.9 DAMAGE TO TEST VEHICLE

Figure 6.6 shows the damage sustained by the vehicle. The front bumper, hood, left front tire and rim, left door, left side steps, left front springs left floor pan, left front corner of box, left rear outer tire, and left rear lower corner of box were damaged. Maximum exterior crush to the vehicle was 12.0 inches in the side plane at the left front corner at bumper height. Maximum occupant compartment deformation was 5.0 inches in the left center floor pan. Figure 6.7 shows the interior of the vehicle.





Figure 6.6. Test Vehicle after Test No. 612831-01-3.





Figure 6.7. Interior of Test Vehicle after Test No. 612831-01-3.

#### **6.10 VEHICLE INSTRUMENTATION**)

Data from the accelerometers were digitized for informational purposes only. Figure E.3 in Appendix E3 shows the vehicle angular displacements, and Figures E.4 through E.6 in Appendix E4 show acceleration versus time traces. Figure 6.8 summarizes pertinent information from the test.

Curb ...... 13,940 lb

Test Inertial ...... 22.370 lb

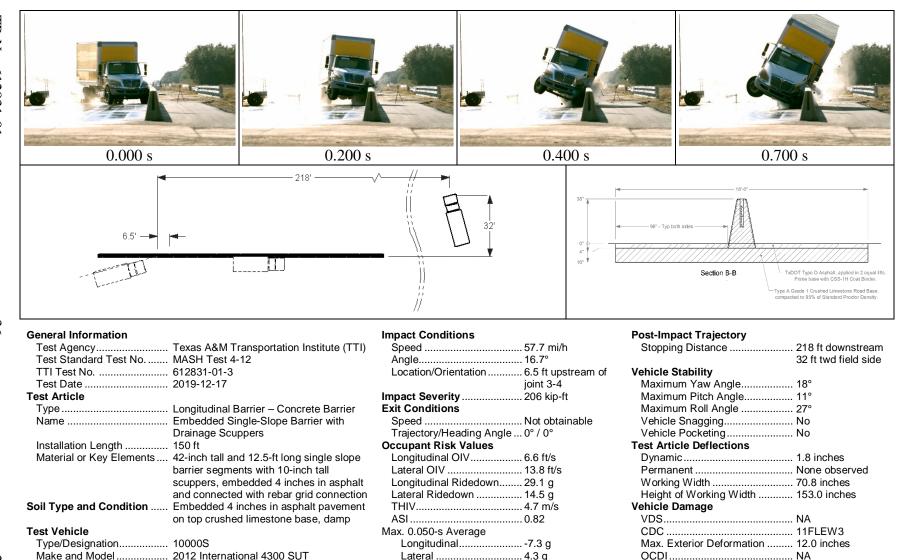


Figure 6.8. Summary of Results for MASH Test 4-12 on Embedded Single-Slope Barrier with Drainage Scuppers.

Vertical .....-3.0 g

Max. Occupant Compartment

Deformation ...... 5.0 inches

## Chapter 7. SUMMARY AND CONCLUSIONS

#### 7.1 ASSESSMENT OF TEST RESULTS

An assessment of each test based on the applicable safety evaluation criteria for *MASH* longitudinal barriers is provided below and in Tables 7.1 through 7.3.

#### 7.1.1 MASH Test 3-11 on Pinned F-Shape Barrier with Drainage Scuppers

The barrier contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 21.7 inches. Permanent deformation of the barrier was 10.0 inches. No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or to present hazard to others in the area. Maximum occupant compartment deformation was 3.0 inches in the left front firewall. The 2270P vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 27° and 17°. Occupant risk factors were within the preferred limits of *MASH*.

#### 7.1.2 *MASH* Test 3-11 on Free-Standing F-Shape Barrier with Drainage Scuppers

The barrier contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 63.0 inches. Permanent deformation of the barrier was also 63.0 inches. A few small pieces of debris were present; however, this debris did not penetrate or show potential for penetrating the occupant compartment, or to present hazard to others in the area. Maximum occupant compartment deformation was 1.5 inches in the left front fire wall. The 2270P vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 20° and 14°. Occupant risk factors were within the preferred limits of *MASH*.

# 7.1.3 *MASH* Test 4-12 on the Embedded Single-Slope Barrier with Drainage Scuppers

The barrier contained and redirected the 10000S vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 1.8 inches. No permanent deformation of the barrier could be observed. No detached elements, fragments, or other debris was present to penetrate or show potential for penetrating the occupant compartment, or present undue hazard to others in the area. Maximum occupant compartment deformation was 5.0 inches in the left center floor pan. The 10000S vehicle remained upright during and after the collision event.

Table 7.1. Performance Evaluation Summary for MASH Test 3-11 on Pinned F-Shape Barrier with Drainage Scuppers.

Test .	Agency: Texas A&M Transportation Institute	Test No.: 612831-01-1	Test Date: 2020-01-07
	MASH Test 3-11 Evaluation Criteria	Test Results	Assessment
	ctural Adequacy	The besides a sectional and and discount discount discount	
	Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.	The barrier contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 21.7 inches. Permanent barrier deformation was 10.0 inches.	Pass
<u>Occu</u>	pant Risk		
,	Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone.	No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or to present hazard to others in the area.	Pass
	Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.2.2 and Appendix E of MASH.	Maximum occupant compartment deformation was 3.0 inches in the left front firewall.	
F.	The vehicle should remain upright during and after collision. The maximum roll and pitch angles are not to exceed 75 degrees.	The 2270P vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 27° and 17°.	Pass
	Occupant impact velocities (OIV) should satisfy the following limits: Preferred value of 30 ft/s, or maximum allowable value of 40 ft/s.	Longitudinal OIV was 18.4 ft/s, and lateral OIV was 21.0 ft/s.	Pass
	The occupant ridedown accelerations should satisfy the following limits: Preferred value of 15.0 g, or maximum allowable value of 20.49 g.	Longitudinal occupant ridedown acceleration was 7.8 g, and lateral occupant ridedown acceleration was 9.7 g.	Pass

the following limits: Preferred value of 15.0 g, or

maximum allowable value of 20.49 g.

Table 7.2. Performance Evaluation Summary for *MASH* Test 3-11 on Free-Standing F-Shape Barrier with Drainage Scuppers.

Test No.: 612831-01-2 Test Agency: Texas A&M Transportation Institute Test Date: 2020-01-09 **MASH** Test 3-11 Evaluation Criteria **Test Results** Assessment **Structural Adequacy** Test article should contain and redirect the vehicle or The barrier contained and redirected the 2270P vehicle. The vehicle did not penetrate, underride, bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the or override the installation. Maximum dynamic **Pass** installation although controlled lateral deflection of deflection during the test was 63.0 inches. the test article is acceptable. Permanent barrier deformation was 63.0 inches. **Occupant Risk** D. Detached elements, fragments, or other debris from A few small pieces of debris were present; however, this debris did not penetrate or show the test article should not penetrate or show potential for penetrating the occupant compartment, or present potential for penetrating the occupant an undue hazard to other traffic, pedestrians, or compartment, or to present hazard to others in **Pass** personnel in a work zone. the area. Deformations of, or intrusions into, the occupant Maximum occupant compartment deformation compartment should not exceed limits set forth in was 1.5 inches in the left front fire wall. Section 5.2.2 and Appendix E of MASH. The vehicle should remain upright during and after The 2270P vehicle remained upright during and collision. The maximum roll and pitch angles are not after the collision event. Maximum roll and pitch Pass to exceed 75 degrees. angles were  $20^{\circ}$  and  $14^{\circ}$ . H. Occupant impact velocities (OIV) should satisfy the Longitudinal OIV was 1.3 ft/s, and lateral OIV following limits: Preferred value of 30 ft/s, or was 8.5 ft/s. Pass maximum allowable value of 40 ft/s. The occupant ridedown accelerations should satisfy Longitudinal occupant ridedown acceleration

was 2.0 g, and lateral occupant ridedown

acceleration was 2.6 g.

Pass

Table 7.3. Performance Evaluation Summary for *MASH* Test 4-12 on Embedded Single-Slope Barrier with Drainage Scuppers.

Test Agency: Texas A&M Transportation Institute Test No.: 612831-01-3 Test Date: 2019-12-17

Tes	t Agency: Texas A&M Transportation Institute	Test No.: 612831-01-3	l'est Date: 2019-12-17
	MASH Test 4-12 Evaluation Criteria	Test Results	Assessment
Str	uctural Adequacy		
<i>A</i> .	Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.	The barrier contained and redirected the 10000S vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 1.8 inches. There was no permanent barrier deformation.	Pass
Occ	cupant Risk		
D.	Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone.	No detached elements, fragments, or other debris was present to penetrate or show potential for penetrating the occupant compartment, or present undue hazard to others in the area.	Pass
	Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.2.2 and Appendix E of MASH.	Maximum occupant compartment deformation was 5.0 inches in the left center floor pan.	
G.	It is preferable, although not essential, that the vehicle remain upright during and after collision.	The 10000S vehicle remained upright during and after the collision event.	Pass

### 7.2 CONCLUSIONS

#### 7.2.1 *MASH* Test 3-11 on Pinned F-Shape Barrier with Drainage Scuppers

The F-shape barrier with drainage scuppers, pinned on 4 inches of asphalt, passed the performance criteria for *MASH* Test 3-11 for longitudinal barriers.

## 7.2.2 MASH Test 3-11 on Free-Standing F-Shape Barrier with Drainage Scuppers

The free-standing F-shape barrier with drainage scuppers passed the performance criteria for *MASH* Test 3-11 for longitudinal barriers.

#### 7.2.3 *MASH* Test 4-12 on the Embedded Single-Slope Barrier with Drainage Scuppers

The embedded single-slope barrier with drainage scuppers passed the performance criteria for *MASH* Test 4-12 for longitudinal barriers.

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# **Chapter 8. IMPLEMENTATION\***

Based on the results of the tests performed in this project, it can be concluded that the free-standing and pinned-down F-shape barrier systems with 6-inch drainage scuppers are *MASH* TL-3 compliant. Similarly, it can be concluded that the embedded single slope barrier with drainage scuppers and 12.5-ft long segments is *MASH* TL-4 compliant. Both F-shape barrier systems passed *MASH* Test 3-11, and the single slope barrier system passed *MASH* Test 4-12.

While the *MASH* small car Test 3-10 was not performed for all three systems, successful past testing of similar systems shows that these tests are not critical (2,3,4). Furthermore, the lighter small car will not impart greater load into the barrier systems and their restraint mechanisms in comparison to the heavier pickup truck of Test 3-11 (for the F-shape barriers) or the single unit truck of Test 4-12 (for the single slope barrier). Based on past testing of similar systems, Test 4-11 with the pickup truck is also not critical for the single slope barrier system (5). Thus, only Test 3-11 was performed for the F-shape barriers and Test 4-12 for the single slope barrier.

The free-standing F-shape barrier was tested with a segment length of 12.5 ft. Longer segment lengths may also be used for this system since increasing segment length reduces the number of connections per unit length of the system, leading to reduced relative rotation of the barrier segments at joints – resulting in reduction in barrier deflection. It also increases the segment mass that must be moved by the vehicle to deflect the barrier.

The pinned-down F-shape barrier was also tested with a segment length of 12.5 ft. In this case, however, increase in segment length may need to accompany an increase in the number of anchor pins installed per segment to adequately restrain the barrier. Segment lengths up to 15 ft may be restrained with three anchor pins per segment. Segments lengths greater than 15 ft and up to 20 ft may be restrained with four anchor pins per segment.

The test installation of pinned-down F-shape barrier was comprised of a 12-inch thick Type-A Grade-1 crushed limestone road base, over which a 4-inch thick asphalt pavement was constructed. This road base was primarily used to meet *MASH* requirements for the type of soil that should be used for testing, and to be able to compact the 4-inch thick asphalt pavement on top. In a field installation, it may not always be feasible to have a 12-inch thick road base. Furthermore, native soil conditions may vary from one site to another. It should be noted that the primary resistance to the deflection of the barrier comes from the asphalt pavement. While differences in soil properties underneath the asphalt layer can have some influence on the lateral deflection of the barrier, their effect is expected to be minimal as long as the sub-base is stable enough to roll and compact the asphalt pavement on top of it. Thus, smaller thickness of road base may also be used in combination with native soil if the sub-base can be stabilized to achieve proper compaction of the 4-inch thick asphalt pavement on top.

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<sup>\*</sup> The opinions/interpretations identified/expressed in this section of the report are outside the scope of TTI Proving Ground's A2LA Accreditation.

The embedded single slope barrier was tested with 4-inch embedment and a segment length of 12.5 ft. This barrier may be used with longer segment lengths since increasing segment length increases its mass, which makes the barrier more difficult to move by the impacting vehicle. The barrier segments tested in this project were 42 inches tall, and with 4-inch embedment, the effective above grade barrier height was 38 inches. Past testing has shown that barrier height of 36 inches performs acceptably for *MASH* Test 4-12 (6). Thus the 42-inch tall barrier segments may be embedded up to 6 inches in asphalt for a minimum above grade height of 36 inches. Furthermore, a taller barrier segment that is embedded 4 inches or greater in asphalt may also be used.

In the test installation of the embedded single slope barrier, asphalt backfill was used under the drainage scuppers. This backfill material does not have a meaningful interaction with the barrier during vehicle impact, and therefore, its material properties do not influence the lateral restraint of the barrier. In a field installation, a different backfill material may be used under the scuppers if desired.

#### REFERENCES

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- 3. K.A. Polivka, R.K. Faller, D.L. Sicking, J.R. Rohde, B.W. Bielenberg, J.D. Reid, and B.A. Coon, *Performance Evaluation of the Permanent New Jersey Safety Shape Barrier Update to NCHRP 350 Test No. 3-10 (2214NJ-1)*. Report No. TRP-03-177-06, Midwest Roadside Safety Facility, University of Nebraska-Lincoln, Lincoln, Nebraska, 2006.
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- 6. N.M. Sheikh, R.P. Bligh, J Holt, *Minimum Rail Height and Design Impact Load for MASH TL-4 Longitudinal Barriers*. Transportation Research Record, Journal of the Transportation Research Board 2309(2309):135-143, December 2012

TR No. 612831-01 45 2020-05-07

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# TR No. 612831-01 **Test Installation** Barrier Numbers 51-5/8" [4.3ft] 2 6 8 Plan View Impact Path 25.0° 104'-0" ±100'-7' Section View on next sheet **Elevation View** 47 ±1" Typ -Connecting Pin 1/8 0" 1-3/4" Anchor Pin Plate Washer, Ø2" OD x 1/4" thick, with Ø 1-1/16" hole at center ASTM A572 Grade 50 ि 0

Ø1" ASTM A449

2020-05-07

30"

Connecting Pin

Scale 1:10 - Galvanize after

all fabrication is complete.

# APPENDIX A. **DETAILS OF THE TEST ARTICLES**

A1. PINNED F-SHAPE BARRIER WITH DRAINAGE SCUPPERS

Q:/dccreditation-17025-2017/EIR-000 Project Files/612831 - Tall Scupper Barriers - Sheikh\01-1 F-shape Pinned 3-11\Drafting, 612831-1/612831-1 Drawing

Detail A Scale 1:20

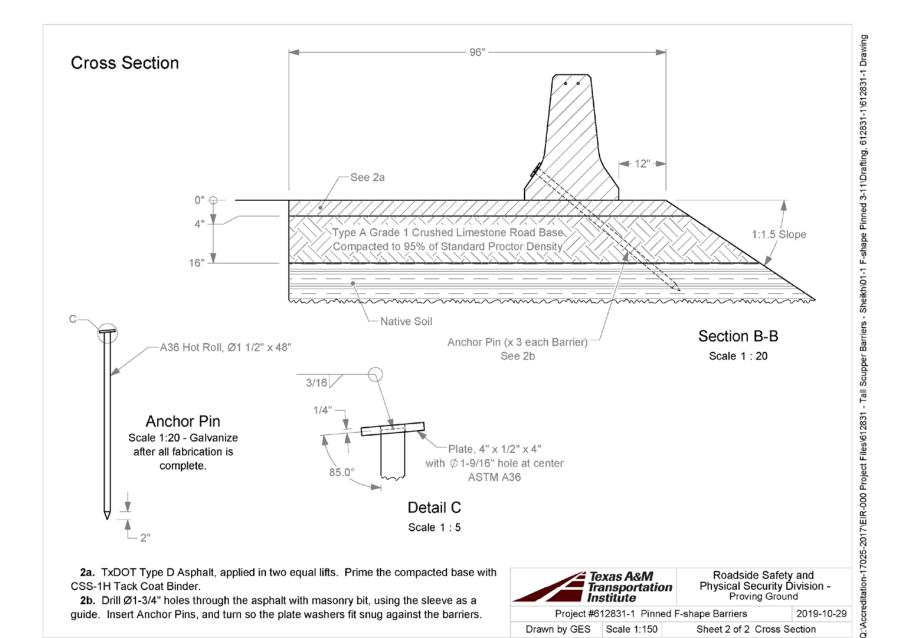
Texas A&M Transportation Institute

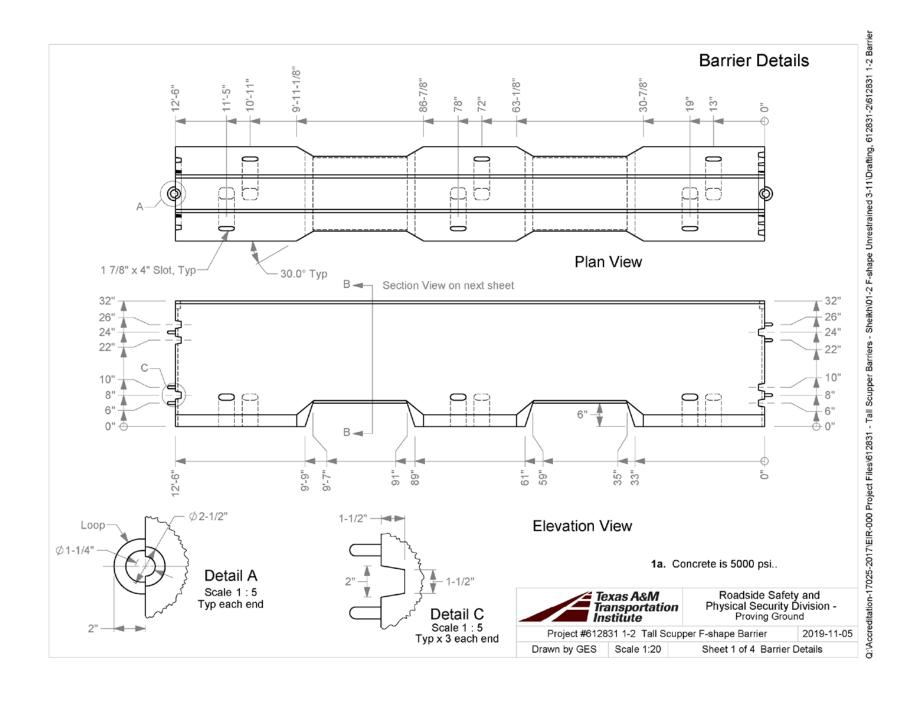
Drawn by GES | Scale 1:150

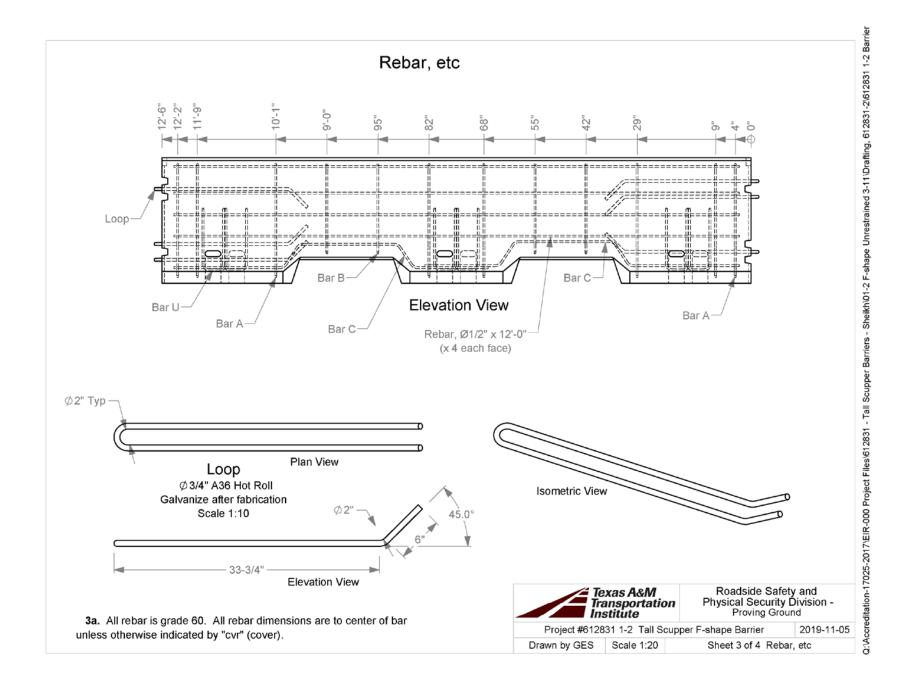
Project #612831-1 Pinned F-shape Barriers

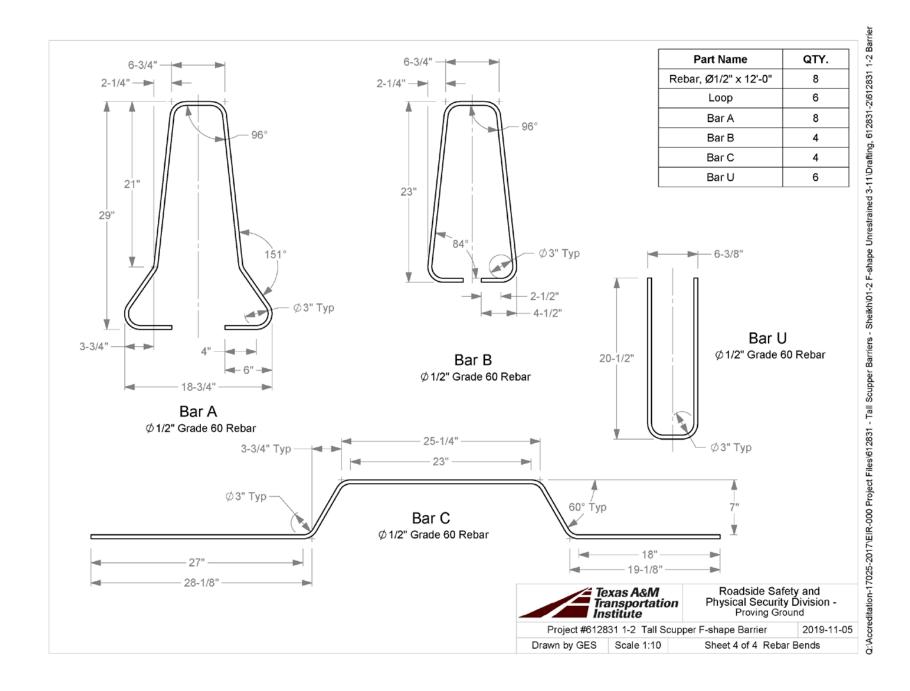
Roadside Safety and Physical Security Division -Proving Ground

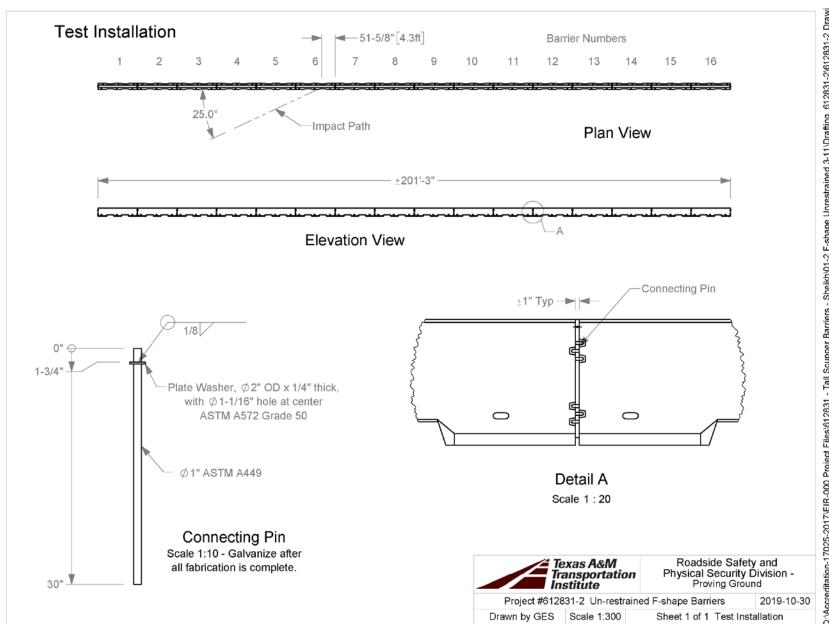
Sheet 1 of 2 Test Installation



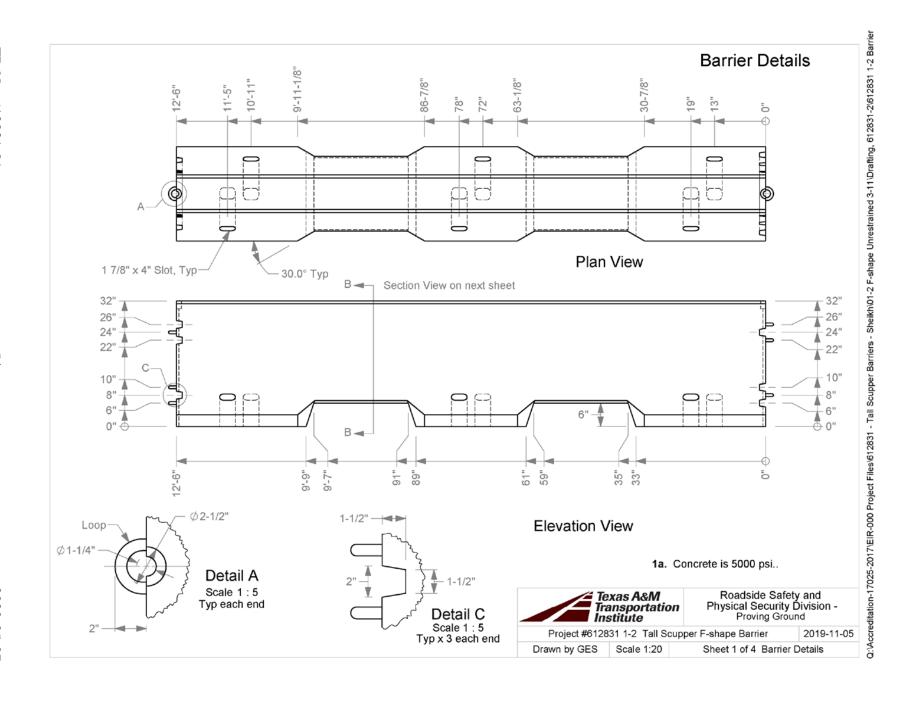


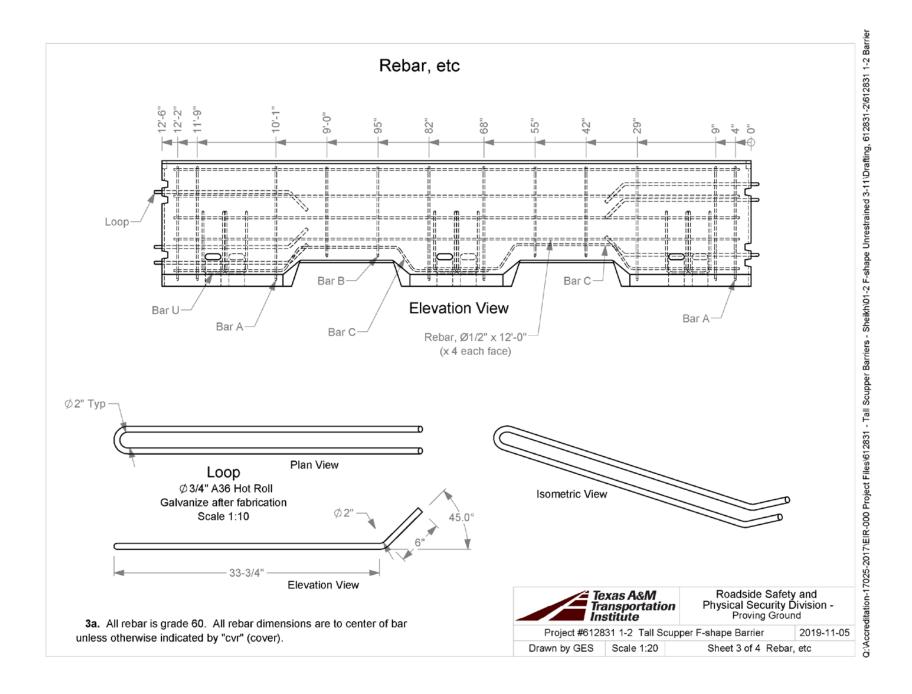


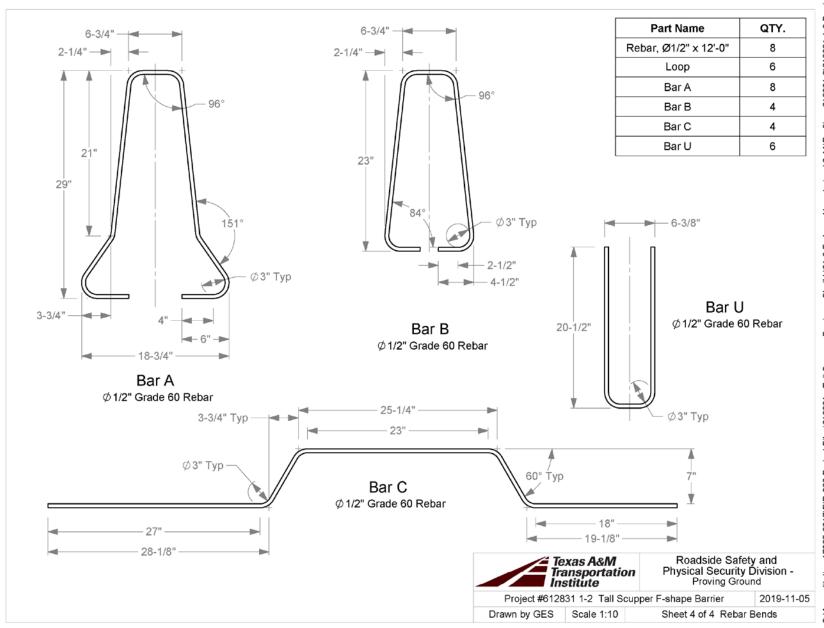




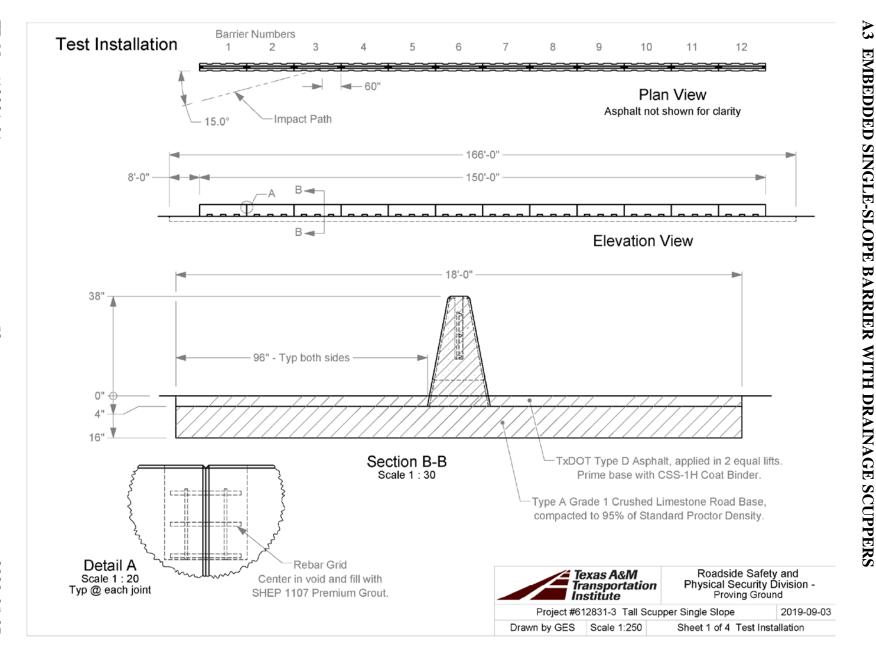
Q:\Accreditation-17025-2017\EIR-000 Project Files\612831 - Tall Scupper Barriers - Sheikh\01-2 F-shape Unrestrained 3-11\Drafting, 612831-2\612831-2\Drawi A2. FREE-STANDING F-SHAPE BARRIER WITH DRAINAGE SCUPPERS

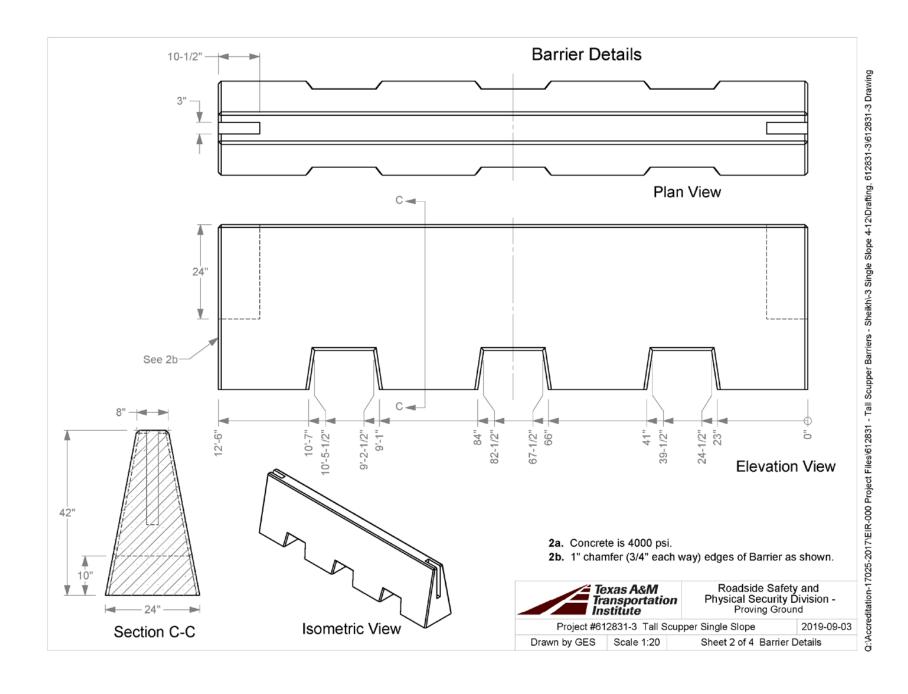


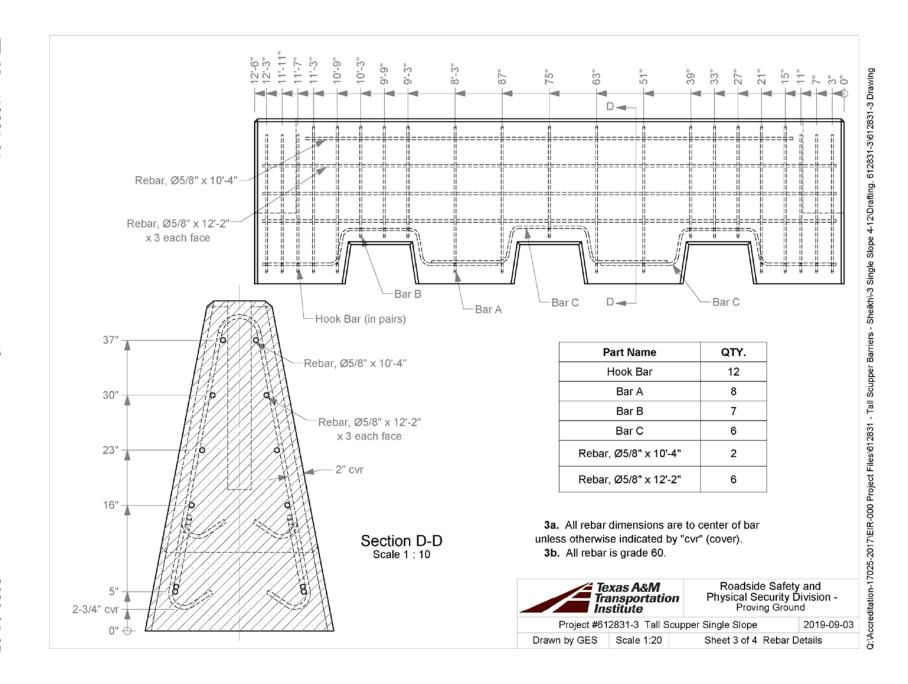


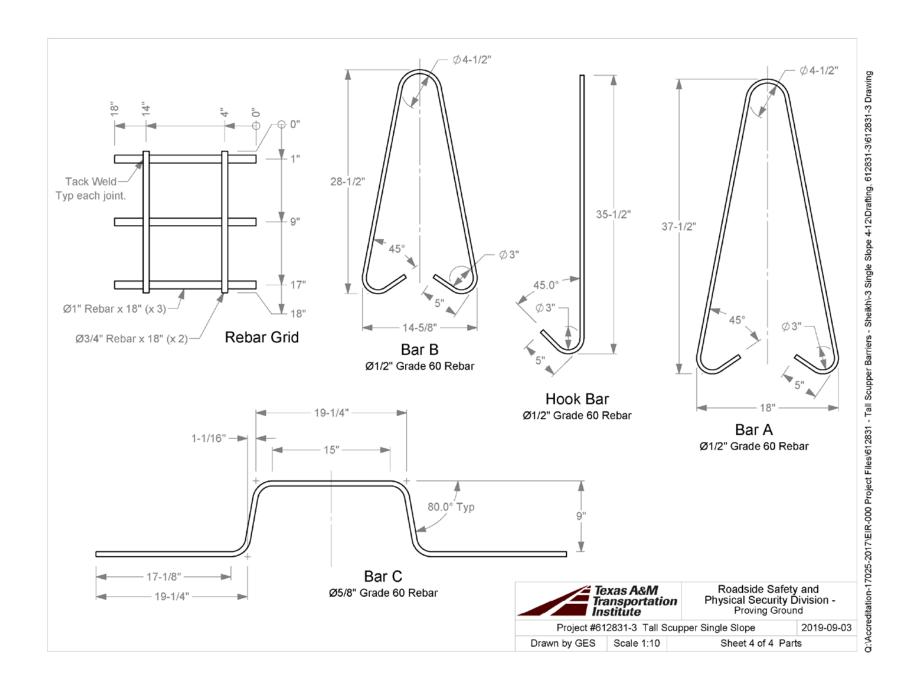


Q:\dccreditation-17025-2017\EIR-000 Project Files\612831 - Tall Scupper Barriers - Sheikh\01-2 F-shape Unrestrained 3-11\Drafting, 612831-2\612831 1-2 Barrier









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#### CERTIFIED MILL TEST REPORT For additional copies call 830-372-8771

We hereby certify that the test results presented here are accurate and conform to the reported grade specification

APPENDIX B. SUPPORTING CERTIFICATION DOCUMENTS

			Quality Assurance Manager							
HEAT NO.:3089816 SECTION: REBAR 13MM (#4) 20'0' GRADE: ASTM A615-18e1 Gr 420// ROLL DATE: 07/16/2019 MELT DATE: 07/16/2019 Cert. No.: 82794205 / 089816A130		O L 10650 Sta		S H I P T O	CMC Construction Svcs 10650 State Hwy 30 College Station TX US 77845-7950 979 774 5900	s College Stati	Delivery#: 82794205 BOL#: 73120574 CUST PO#: 823592 CUST P/N: DLVRY LBS / HEAT: 2191.000 LB DLVRY PCS / HEAT: 164 EA			
Characteristic	Value		Characteristic		Value		Characteristic Value			
C Mn P S Si Cu Cr Ni Mo V Cb Sn Al Yield Strength test 1	0.44% 0.79% 0.013% 0.031% 0.11% 0.20% 0.074% 0.000% 0.001% 0.0014% 0.000%		Bend Test Diame		(0.000 - 0.00	*Material is fully kit *100% melted and *EN10204:2004 3.: *Contains no weld *Contains no Merc *Manufactured in a	rolled in the USA 1 compliant repair ury contamination ccordance with the latest version			
Elongation test 1 Elongation Gage Lgth test 1 Tensile to Yield ratio test1 Bend Test 1	15% 8IN 1.60 Passed					*Warning: This pro known to the State	merica" requirements of 23 CFR635 410, 49 CFR 661 bduct can expose you to chemicals which are te of California to cause cancer, birth defects tive harm. For more information go			

REMARKS :





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Fecha / Date:	27/06/2019

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7.184725 TH UK4085

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Cliente / Customer: DEACERO USA IN	C (HOUSTON DISTRIBUTION CENTER)	CLIENTE CONSIGNADO / SHIP TO	DATOSDEL ENDARGUETA
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	Pais / Country: U.S.A. C.P.JZIP 77022-3	25ta007 State: , 1X	Pedido / Customer Order No: 21973241
Correo Electrónico / eMail:			Núm. Plan / Shipping Plan: 158261
			Fecha Embarque / Date: 26/06/2019
			Orden de Compra / Purchase Order.

olada /	Secuencia /	Clave /	Producto / Description of Goods	% C	% Mo						1111							- 11
Heat	Sequence	Code			76 MII	% Si	% P	% S	% Cu	% Cr	% Ni	% Mo	% Sn	% Ti	% V	% Nb	% N	CE
6135	9541	13158	ROUND BAR 3/4" A-36 P 20" 1.5T	0.14	AVG	AVG	AVG	AVG	AVG	AVG	AVG	AVG	AVG	AVG	AVG			1
6240	42241	13158	10000 000 0000	0.14	0.67	0.18	0.007	0.003	0.24	0.066	0.083	0.020	0.011	0.001	0.004	0.003	0.009	0.2
	101000000	10100	ROUND BAR 3/4" A-36 P 20" 1.5T	0.21	0.78	0.18	0.010	0.008	0.30	0.119	0.114	0.025	0.014	0,001	3	6	1	
6243	42228	13158	ROUND BAR 3/4" A-36 P 20' 1.5T	0.21	0.73	0.19	0.008		_	5	5	5	5	4	0	0.000	0.008	0.3
246	42224	13158	ROUND BAR 3/4" A-36 P 20' 1.5T	-	0.75	0.15	0.008	0.003	0.25	0.117	0.100	0.022	0.012	0.001	0.005	0.000	0.008	0.3
		10.00	1.50 P 20 1.5T	0.21	0.72	0.20	0.009	0.014	0.26	0,095	0 118	0.027	0.014	4	0.005	0.000	6	9

LF kg/mm² AVG	YS PSI AVG	P. Doblez / Bend Test
AVG	1	bend lest
-		
39.34	55980,82	
33.54	33900.82	Cumple / Successfull
35.14	50004.22	
		Successfull
35.01	49819.23	
20 10	44502.02	Successfully Cumple /
	35.14 35.01 29.19	35.01 49819.23

CE = [C] + ([Cu)/40) + ([Mn]/6) + ([Ni]/20) + ([Cr]/10) + ([Mo)/50) + ([V]/10)



Certificamos que este material ha sido producido, inspeccionado y probado de acuerdo a las normas de fabricación del acero aplicables a la ASTM A38-2008, A529-2005 (re aprobada el 2009), A572-2012 y A992-2011 y a las normas dimensionales NAX B252, ASTM A6/A6M-2012. / We certify hast this material has been produced hot-rolled carbon, inspected and tested according to standards applicable steelmaking to ASTM A38-2008, A529-2005 (Reapproved 2008), A572-2012 y A992-2011, and the dimensional standards NAX B252, ASTM A6/A6M-2012.

JUAN ALEJANDRO GALVAN GARCIA Gerente de Aseguramiento de Calidad / Quality Assurance Manager 65

2020-05-07



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MX 01 800 021 3322, USA 1800 332 2376

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Certificate No:	135714 - 2184	7/33
echa / Date:	22/01/2019	

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Cludad / City: HOUSTON	Estado / State: , TX	Dirección / Address: 1755 FEDERAL RD	Núm Fachim (In view)
eléfono / Phone: 332 2376	Pais / Country: U.S.A. C.P./ZIP 77022-3	Ciudad / City: HOUSTON Estado / State: , TX	Núm. Factura / Invoice No: FQ77449 Padido / Customer Order No: 21847733
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			Fecha Embarque / Date: 21/01/2019
Consideration of the American Constitution of the Constitution of			Orden de Compra / Purchase Order:

3530 39 3507 7		60683 60584	ROUND BAR 1 1/2" A-36 P 20' 2.0T	AVG	% Mn	% Si	% P	% S	% Cu	% Cr	% N	% Mo	21.0					
3507 7		10000000	ROUND BAR 1 1/2" A-36 P 20' 2.0T		AVG						70 FW	76 MO	% Sn	% Ti	% V	% Nb	% N	CE
	743	60594	1.00 1 20 2.01			AVG	AVG	AVG	AVG	AVG	AVG	AVG	AVG	AVG	AVG		100.00	1000
3500 7	(A.A.2222)		FLAT BAR 1 1/2" x 3/16" A-36 P 20' 2.0T	0.22	0.75	0.20	0.009	0.009	0.24	0.074	0.074	0.017	0.014	0.001	-	0.002	0.010	0.3
	7.0			0.21	0.73	0.21	0.012	0.004	0.24	0.065	0.084	0.019	0.012	0.001	0.004	0.001	2	2
	740	60584	FLAT BAR 1 1/2" x 3/16" A-36 P 20" 2.0T	0.21	0.77	0.24	0.010	0.005	0.21	0.074	0.079	5	0	1	4	5	0.009	0.3
3510 7	736	60584	FLAT BAR 1 1/2" x 3/16" A-36 P 20" 2.0T	0.21	0.76	0.00				0	0.079	0.018	0.012	0.001	0.004	0.001	0.008	0.3
3512 7	739	60584		0.21	0.70	0,23	0.011	0.010	0.24	0.076	0.088	0.025	0.012	0.001	0.004	0.001	0.008	0.3
		1000000	FLAT BAR 1 1/2" x 3/16" A-36 P 20" 2.0T	0.20	0.76	0.23	0.014	0.011	0.21	0.081	0.062	0.017	0.009	0.001	0.004	6	6	3
3514 7	734	60584	FLAT BAR 1 1/2" x 3/16" A-38 P 20' 2.0T	0.21	0.76	0.22	0.014	0.005	0.27	0	5	0	0	1	4	0.001	0.008	0.3
5080 39	39839	80611	ANGLE 1 1/2" x 1 1/2" x 1/8" A36/529-50 20"	0.20	0.72	0.20	0.007	0.020	0.27	0.104	0.079	0.016	0.014	0.001	0.004	0.001	0.008	0.3

33530	Sequence	Code	Producto / Description of Goods								
33530		<b>斯里利里</b>		Calibre / Diameter	Cantidad / Bundle	RT kg/mm²	TS PSI	% Elong / Elong	LF kg/mm²	YS PSI	P. Doblez /
	39468					AVG	AVG	AVG	AVG	AVG	Bend Test
45-1000	39468	50683	ROUND BAR 1 1/2" A-36 P 20"	1 1/2"	5	52.42	74593.66				
33507	743	0000	2.0T			32,42	74593.66	32.58	32.24	45877.52	Cumple /
	/13	80584	FLAT BAR 1 1/2" x 3/16" A-36 P	1 1/2" x	4	43.72	62213.56	30.57	-		Successfully
33509	740	50504	20° 2.0T	3/16*			022 13.30	30.57	30.44	43316.12	Cumple /
30000	740	60584	FLAT BAR 1 1/2" x 3/16" A-36 P	1 1/2° x	2	50.85	72359.55	31.06			Successfully
33510	736	60584		3/16*			. 2000.00	31.00	36.79	52352.17	Cumple /
	.00	00004	FLAT BAR 1 1/2" x 3/16" A-36 P 20" 2.0T	1 1/2" x	1	52.77	75091.71	33.54		-	Successfully
33512	739	60584		3/16"	1		1.0001.71	33.54	37.22	52984.06	Cumple /
		00004	FLAT BAR 1 1/2" x 3/16" A-38 P	1 1/2" x	1	52.11	74152.53	31.57	20.00	-	Successfully
3514	734	60584		3/16*		2000000	14152.55	31.57	36.20	51512.60	Cumple /
	1.00	00304	FLAT BAR 1 1/2" x 3/16" A-36 P 20" 2.0T	1 1/2" x 3/16"	2	42.26	60135.98	32.38	36,97	52608.31	Successfully Cumple /



Certificamos que este material ha sido producido, inspeccionado y probado de acuerdo a las normas de fabricación del acero aplicables a la ASTM A36-2008, A529-2005 (re aprobada el 2009), A572-2012 y A992-2011 y a las normas dimensionales NMX B252, ASTM A6IA6M-2012. / We certify that this material has been produced hot-rolled carbon, inspected and tested according to standards applicable steefnaking to ASTM A36-2008, A529-2005 (Reapproved 2009), A572-2012 y A992-2011, and the dimensional standards NMX B252, ASTM A6IA6M-2012.

GUSTAVO GABRIEL MANCILLA GARZA Gerente de Aseguramiento de Calidad / Quality Assurance Manager

ROUND BAR A 36/A 529 GR50 1-1/2 X 20\*

ROUND BAR A-36/A 529 GR50 1-1/2 X 20'



Aceria Ramos Arizpe CARRETERA MONCLOVA KM 4 NUMERO 2125 TRAMO SANTA CRUZ CJO CALIENTE C.P./ZIP RAMOS ARIZPE, COAHUILA Tel/Phone (+52) 01 818 368 1111

/ CERTIFICATE OF TEST AN ANALYSIS

Colada /	Secuencia /	Clave /	Producto / Description of Goods			CONTRACTOR OF THE PARTY OF THE	OI LIVINE		700	I STATE	100
Heat	Sequence	Code	STATE OF THE PARTY	Diameter	Cantidad / Bundle	RT kg/mm³	TS PSI	% Elong / Elong	LF kg/mm²	YS PSI	P. Doblez
105080	39839	60611				AVG	AVG	AVG	AVG	AVG	Bend Tes
	25028		ANGLE 1 1/2" x 1 1/2" x 1/8" A38/529-50 20' 2.0T	1 1/2" x 1 1/2" x 1/8"	4	53.17	75660.91	34.29	37.24	52992.52	

No. Cartificado / Certificate No: 135714 - 21847733 Fecha / Date: 22/01/2019

Hecho en México / Made in Mexico

Certificamos que este material ha sido producido, inspeccionado y probado de acuerdo a las normas de fabricación del acero aplicables a la ASTM A38-2008, A529-2005 (re aprobada el 2009), A572-2012 y A992-2011 y a las normas dimensionales NMX B252, ASTM A6/A6M-2012. / We certify that this material has been produced not-rolled carbon, inspected and tested according to standards applicable steelmaking to ASTM A36-2008, AST92-2005 (Reapproved 2009), AS72-2012 y A992-2011, and the dimensional standards NMX B252, ASTM A6/A6M-2012.

GUSTAVO GABRIEL MANCILLA GARZA

Gerente de Aseguramiento de Calidad / Quality Assurance Manager

99

2020-05-07

09-23-2019 23:53 Load - 3420331

BL - 3870965

blr466

Custom Fabricators

Heat - 2910110

Cust. PO - CF19-24152

Order - 17857122

Kloeckner Metals Corporation

Kloeckner Metals Corp - HTX 7400 Mesa Drive Houston,TX 77028, United States (713) 633-7400

Material Certifications for Shipment August 13, 2019 Bl Num: 6.069683 Customer Florither Metals Corp - HOU 14244 Alecda Bond Houston,TX 770532510 Ship To Kloeckner Metals Corp-Hou 14200 Almeda Rd Houston, TX 77053-2510 Description/Part Number 96.000 Strip Mill Plate 1/2 Order No GA Ord 48.000

Cust PO: 7415466 Heat: 2910110 Mill ID:2910110-4 Vendor: NUCOR STEEL - BERKELEY Certification # 4724465 Insue Date: 26-JUL-19

Carbon Equivalent: .29

Chenical Properties

C Mn P S Si Al B
171. .496 012 031 .046 .025 .

M6 N ND N1 S5 T1 V
.316 .006 .000 .036 .006 .001 .000 .036

Physical Properties Property Mill Test Internal Test Head Internal Test Middle Internal Test Tail Charpy Properties

Value
Internal Test Head
Internal Test Middle
Internal Test Tail PT-LBS1 FT-LBS2 FT-LBS3 LBS

MDTRS] 1474451
We certify that the listed information is correct as contained in the records of the cospany and that all results meet the requirements of the specification cited above. All ABME Section II Pert A specifications are 2017 Edition. All Charpy Leats are at -70 F unless otherwise indicated.



09-23-2019 23:53

Load - 3420331

BL - 3870965

blr466

Custom Fabricators

Cust. PO - CF19-24152

Heat - 2910110

Order - 17857122

08-13-2019 16:25

Load - 3386911

BL - 81069683

bir466

Kloeckner Metals Corp - HOU

Cust. PO - 7418486

Heat - 2910110

Order - 17709462

HASS Heque Avenue Horracturector ITST FERGET More stated Decisions where 50 25450 a division of horracture concernion and accordant Accordant Corporation Ship Control Corporation 105 700 COLOGNIA CTR PREF. Too. 18500 EACH THE MUD. STEE 500 COLOGNIA CTR PREF. Too. 18500 EACH THERETORY. PROFESSION OF CORP. THE STATE ST

Gauge x Width .4900 HIN I 48.0000 HIN ASIM ASONS / SS GR36 TYPE 2 / REV: 2018 SUITABLE FOR CONVERSION TO ASTH A35

.006 .001 .009 19.55 .000 .006 ₽ 5

Z 6.

Mn P 2 2

2910110 .19

VIELO XPENGTR IDISTLE SPENGTE E (Ast) (Ast

2910110-4 2910110-5 2910110-6 (47080.00 tB) (47160.00

All naterial is sold subject to the description, specifications and terms on the face and reverse side of Papor Steel - Borkeley's salos order acker

We because cartisfy the above inforestion is secret as contained in the Seconds of the Computation.

For I She to the Computation of the Second Secon



### **Material Test Report**

7953 Washington Woods Dr.

Dayton, OH 45459 Phone. (937)573-4675 Fax: (702)926-4128

5/05/2019 0000065775

Ref No 108380

PO/Line# 144313-8

TEST CERTIFICATE ACC TO DIN 50049 : 3.1 B/EN: 10204: 3.1

Customer:

Part Number:2233-N

PO Number:146313

Description: ASTM A513T5-14, SRA-(BKS); DOM; Grade=1020; Category=CAR8ON WELDED; Round; OD=3.125"; ID=2.935"; (WT=0.095"); Deburred, Length=240" to 288"

Product: TUBE

Heat No: 18C6983

Steel Grade: 1020

Ends: Deburred

DIMENSIONS TOLERANCE QUANTITY MM -VE +VE UOM NUMBER METER / FT WEIGHT TON 0.0090 IN 0.0000 IN 79.375 3.125 0 0000 804.672 2,640 000 2.935 0.095 0.0000 74.549 6.096.000 7,315.200 MM 240.000 - 288.000 IN

	CHE	EMICAL	ANALYSI	S			MECH	ANICAL PR	OPERTIE	S
ELEMENTS Carbon (C) Manganese (Mn)	UOM %	MIN 0.1800 0.3000	MAX 0.2300 0.6000	MILL 0.2010 0.4510			Y.S Psi	U.T.S Psi	% E	HARDNES HRB
Silicon (Si)	%			0.0270	)	MIN	55,000	65,000	10.00	75 00
Aluminum (AL)	%			0.0370	E.	MAX				
Phosphorus (P)	%		0.0350	0.0150	E.	MILL	72.020	81.490	12 00	85.00
Sulfur (S)	%		0.0350	0.0060	6	PROD	100000000			00.00
Chromium (Cr)	%					11100				
Nickel (Ni)	%					Sample	Orientation			
Molybdenum (Mo)	%					Tensile	Spec Type:			
Vanadium (V)	%					Gauge	Width (MM):			
Copper (Cu)	%						+			
Columbium (CB)	%					Remark	S:			
Calcium (Ca)	%							0.00		
Titanium (Ti)	%						MITO	u 1.5	38.4	711
Nitragen (N)	%						MTR	#		17
Boron (B)	9/4									

1864983 Heat#\_

PASSED (AS PER S8 OF ASTM A513)

1/3% OF OD PASSED

15 % of ID PASSED

(1/1000 MM MAX) PASSED

FLARING TEST

100% EDDY CURRENT TESTING- E309

FLATTENING TEST

STRAIGHTNESS Remarks:

Antimony (Sb) Tin (Sn)

Melted In: INDIA Product In: INDIA Product is free of weld repair and has not come in contact with Mercury or any of its components.

We hereby Certify that the material described above conforms to Specification and Purchase Order.

Bill Johnston

Quality Co-ordinator

Any questions, Please email: TestReports@aaris-llc.com

ALTOS HORNOS DE MEXICO

# 1/4" A572-50 plate

MILL TEST CERTIFICATE AHMSA: QUALITY WITH THE STRENGTH OF STEEL PROLONGACION JUAREZ SIN NUMERO COLONIA LA LOMA MONCLOVA COAHUILA 25770

B024046B

										OF ISSUED		AGE	WE HEREBY SHOW IN TH	THAT CHEM	ICAL AND / OR TEST INE CONNECT AS US OF THE COMPAN
	AHMSA INT/ TRIPLE-	S HOUSTON							12.	07.2017		1	CONTAINED	THE RECOR	US OF THE COMPAN
ADDRESS	5150 N LOOP1604 W	SAN ANTONIO, TX													7
PRODUCT		***************************************											ING. KA	MIHO CISALE	OS MENCHACA
	WIDE PLATE												MECHANICA	ALLES AN	U CERTIFICATION
+				CHE	MICA	AL CO	MPOSIT	ION							
HEAT	SPECIFICATION	C	Mn	P	S		Si	Cu	Cr	Ni	Mo	Alt	V	Cb	Ti
173583	ASTM A572-50/A709-50	0.170	0.850	0.022	0.	003	0.120	0.019	0.028	0.027	0.006	0.034	0.004	0.007	0.002
173584	ASTM A572-50/A709-50	0.160	0.840	0.023	0.	003	0.120	0.014	0.023	0.022	0.005	0.034	0.005	0.007	0.002
173585	ASTM A572-50/A709-50	0.170	0.850	0.024	0.	003	0.120	0.034	0.023	0.022	0.005	0.031	0.004	0.007	0.002
+				TE	ST O	F TH	E PRODU	JCT							
HEAT	SLAB	PLATE NO.	THICK	NESS(Ir	ich)	Y.ST	TRENGTH		T.STRENG	GTH	%ELON.		T.EL	ONG.	
173583	3170	11144791	0.250	0		64.7	776 (KSI	)	80.464(1	KSI)	32(%)		2		
173583	3220	11145011	0.250	0		64.8	357 (KSI	)	80.160(I	(SI)	33(%)		2		
173584	3080	11145051	0.250	0		64.5	577 (KSI	)	79.498(1	KSI)	33(%)		2		
173584	3110	11145041	0.250	0		64.7	785 (KSI	)	80.550(1	KSI)	34(%)		2		
173585	3030	11145071	0.250	0		64.7	799(KSI	)	80.938(1	KSI)	34(%)		2		
173585	4070	11145251	0.250	0		63.4	188 (KSI	)	80.527(1	(SI)	38(%)		2		
+					SHIP	PED	PRODUCT								
HEAT	PLANCHON	PLATE NO.	THICK	NESS (Ir	ich)	WIDT	H(Inch	) LAI	RGE (Inch)	OF	DER		ITEM	Т	ELIVERY
173583	3140	11145081	0.250	0		96.0	0000	240	0.000	0.0	0020148	9	000010		.002477499
173583	3190	11145033	0.250	0		96.0	0000	240	0.0000		0020148		000010		.002477499
173583	3220	11145011	0.250	0		96.0	0000	240	0.000		0020148		000010		.002477499
173583	3220	11145012	0.250	0		96.0	0000	240	0.000	0.0	0020148	9	000010		.002477499
173584	4120	11145061	0.250	0		96.0	0000	240	0.000		0020148		000010		.002477499
173584	4120	11145062	0.250	0		96.0	0000		0.000		0020148		000010		.002477499
173584	4120	11145063	0.250	0		96.0	0000	240	0.000		0020148		000010		.002477499
173585	3030	11145071	0.250	0		96.0	0000		0.000		0020148		000010		002477499
173585	3030	11145072	0.250	0		96.0	0000		0.000		0020148		000010		002477499
173585	3030	11145073	0.250	0		96.0	0000	240	0.000		0020148		000010		002477499
HEAT	PLANCHON	PLATE NO.	CUSTO	MER ORI	).				TANDARD				000010	_	002477499
173583	3140	11145081	AHI48	20 (WLY-	2016	51)/N	1		4-6						
173583	3190	11145033	AHI48	20 (WLY-	2016	51)/N	1		4-6						
173583	3220	11145011		20 (WLY-					4-6						
173583	3220	11145012		20 (WLY-		- 10			4-6						
173584	4120	11145061		20 (WLY-					4-6						
173584	4120	11145062		20 (WLY-					1-6						
エリココロエ	4120	11145063		20 (WLY-		. ,			. 6						
173584					2016	,			-6						



### MILL TEST CERTIFICATE AHMSA: QUALITY WITH THE STRENGTH OF STEEL PROLONGACION JUAREZ SIN NUMERO COLONIA LA LOMA MONCLOVA COAHUILA 25770

B024046B

The second secon				1 0	ATE OF ISSUED	PAGE	ME HEEL CONTROL
USTOMER	AHMSA INT/ TRIPLE-S	HOUSTON			2.07.2017	2	WE HEREBY THAT CHEMICAL AND FOR TEST SHOW IN THIS REPORT ARE CORRECT AS CONTAINED THE RECORDS OF THE COMPAN
DDRESS							
	5150 N LOOP1604 W S	SAN ANTONIO, TX.					ING. RAMINO CISTEROS MENCHAÇA
RODUCT	WIDE PLATE						
							MECHANICALIES SAND CERTIFICATION
			SHIPPED PRODUCT				
EAT	PLANCHON	PLATE NO.	CUSTOMER ORD.	STANDA	RD		
73585	3030		AHI4820(WLY-20161)/M	A-6			
73585	3030		AHI4820(WLY-20161)/M	A-6			
DIN EN1020	4 3.1 COMPLIANT	ALUMINUM KILLED	WITH FINE GRAIN PRACTICE.				
	ORIGIN: MÉXICO		END OF DATA				
140410 00HDF21110	OF IC ONLY WITH THE CHOTOMES	ENTIONED IN THEORET : TO CO	E, AHMSA WILL ONLY ACCEPT THE ORIGINAL DOCUMEN		ISSUED :	C01AI10	



## MILL TEST CERTIFICATE AHMSA: QUALITY WITH THE STRENGTH OF STEEL PROLONGACION JUAREZ SIN NUMERO COLONIA LA LOMA MONCLOVA COAHUILA 25770

B024047B

CUSTOMER	AHMSA INT/ TRIPLE-	S HOTISTON						OF ISSUED		AGE	SHOW IN TH	THAT CHEM	ICAL AND / OR TEST ARE CORRECT AS US OF THE COMPAN
ADDRESS	THE	5 1100510N					12.	07.2017		1	CONTAINED	THE RECOR	US OF THE COMPAN
IDDRESS	5150 N LOOP1604 W	SAN ANTONIO, TX.											P
PRODUCT											ING. KA	MIKO CISNEL	OS MENCHACA
	WIDE PLATE										MECHANIC	ALIES AN	U CERTIFICATION
+			C	CHEMICAL C	COMPOSITI	ON							
HEAT	SPECIFICATION	C	Mn P	S	Si	Cu	Cr	Ni	Mo	Alt	V	Cb	Ti
173583	ASTM A572-50/A709-50	0.170	0.850 0.02	2 0.003	0.120	0.019	0.028	0.027	0.006	0.034		0.007	0.002
173584	ASTM A572-50/A709-50	0.160	0.840 0.02	3 0.003	0.120	0.014	0.023	0.022	0.005	0.034		0.007	0.002
+				TEST OF T	HE PRODUC	CT							0.002
HEAT	SLAB	PLATE NO.	THICKNESS (	Inch) Y.S	TRENGTH	T	.STRENG	TH	%ELON.		T.EL	ONG	
173583	3170	11144791	0.2500	64.	776 (KSI)	8	30.464 (K	SI)	32(%)		2	.0110.	
173583	3220	11145011	0.2500	64.	857 (KSI)	8	30.160(K	SI)	33(%)		2		
173584	3080	11145051	0.2500	64.	577 (KSI)	7	79.498 (K	SI)	33(%)		2		
173584	3110	11145041	0.2500	64.	785 (KSI)	8	30.550(K	SI)	34(%)		2		
+				SHIPPED	PRODUCT								
HEAT	PLANCHON	PLATE NO.	THICKNESS (	Inch) WID	TH (Inch)	LARG	E(Inch)	ORI	DER		ITEM	г	ELIVERY
173583	3180	11145021	0.2500	96.	0000	240.	0000	000	0020148	9	000010		.002477497
173583	3180	11145022	0.2500	96.	0000	240.	0000	000	0020148	9	000010		.002477497
173583	3180	11145023	0.2500	96.	0000	240.	0000		0020148		000010		.002477497
173583	3220	11145013	0.2500	96.	0000	240.	0000	000	0020148	9	000010		.002477497
173584	3080	11145051	0.2500	96.	0000	240.	0000	000	0020148	9	000010		.002477497
173584	3080	11145052	0.2500	96.	0000	240.	0000	000	0020148	9	000010		.002477497
173584	3080	11145053	0.2500	96.	0000	240.	0000	000	0020148	9	000010		002477497
173584	3110	11145041	0.2500	96.	0000	240.	0000	000	0020148	9	000010		002477497
173584	3110	11145042	0.2500	96.	0000	240.	0000	000	0020148	9	000010		002477497
173584	3110	11145043	0.2500	96.	0000	240.	0000	000	020148	9	000010		002477497
HEAT	PLANCHON	PLATE NO.	CUSTOMER O	RD.		ST	ANDARD						
173583	3180	11145021	AHI4820(WL	Y-20161)/	M	A-	6						
173583	3180	11145022	AHI4820(WL	Y-20161)/	M	A-	6						
173583	3180	11145023	AHI4820(WL	Y-20161)/	M	A-	6						
173583	3220	11145013	AHI4820(WL	Y-20161)/	M	A-	6						
173584	3080	11145051	AHI4820(WL	Y-20161)/	M	A-	6						
173584	3080	11145052	AHI4820(WL	Y-20161)/	M	A-	6						
173584	3080	11145053	AHI4820(WL	Y-20161)/	M	A -	6						
173584	3110	11145041	AHI4820(WL	Y-20161)/	M	A-	6						
173584	3110	11145042	AHI4820(WL	Y-20161)/	M	A -	6						
173584	3110	11145043	AHI4820 (WL			A-	6						
ALL HE	ATS AND SIZES ARE FULLY	ALUMINUM KILLEI	WITH FINE	GRAIN PRA	CTICE.								
								7.00mm					
HMSA'S COMP	ROMISE IS ONLY WITH THE CUSTOMER M	ENTIONED IN THISCERTIFICA	TE, AHMSA WILL ON	LY ACCEPT THE	ORIGINAL DOCI	JMENT.		ISSUED	: CO1AI	10			



### MILL TEST CERTIFICATE AHMSA: QUALITY WITH THE STRENGTH OF STEEL PROLONGACION JUAREZ SIN NUMERO COLONIA LA LOMA MONCLOVA COAHUILA 25770

B024047B

CUSTOMER		DATE OF ISSUED	PAGE	WE HEREBY THAT CHEMICAL AND LOD TEST
COSTOMER	AHMSA INT/ TRIPLE-S HOUSTON	12.07.2017	2	WE HEREBY THAT CHEMICAL AND FOR TEST SHOW IN THIS REPORT ARE CORRECT AS CONTAINED THE RECORDS OF THE COMPANY
ADDRESS	5150 N LOOP1604 W SAN ANTONIO, TX.			7
PRODUCT	WIDE PLATE			MECHANICAL LES AND CERTIFICATION
DIN EN102	04 3.1 COMPLIANT			
	F ORIGIN: MÉXICO			
+	END OF DATA			
AUMONIO COMPTE	SE IS ONLY WITH THE CUSTOMED MENTIONED IN THIS CEPTIFICATE ANNISA WILL ONLY ACCEST THE ORIGINAL DOCUMENT	ISSUED :	C01AI10	

Proving Ground¶ 3100 SH-47, Bldg-7091¶ Bryan, TX-77807	Texas A&M Transportation Institute Texas-A&M-University[ College-Station, TX-77843¶ Phone-979-845-6376]	QF·7.3-01··Concrete· Sampling¤	Doc.·No.¶ ¶ <i>QF-7.3-01</i> □	Issue Date: ← ← 2018-06-18¤
Quali	ity·Form¤	Prepared by: Wanda-L. Menges¶ Approved by: Darrell L. Kuhn¤	Revision: ↔ 6¤	Page:¶ 1 of 1□

Project No:	612831	Casting Date:	11/26/2019	Mix Design (psi):	5000
Technician Taking Sample	TERR/	TERRACON		TERRACON	
Signature of Technician Taking Sample	TERRACON		Signature of Technician Breaking Sample		
Load No.	Truck No.	Ticket No.	Locat	ion (from concrete map)	
T1	7131	5793157	6 of the F-shape B	arriers	

Lood No	Break Date	Culinday Ass	Total Load (lbs)	Decole (mail	A
Load No.	Break Date	Cylinder Age	Total Load (Ibs)	Break (psi)	Average
	T1-	SEE ATTACHED SH	EETS FROM TERRAC	ON	
				_	
	<del> </del>				

TR No. 612831-01 74 2020-05-07

			CUSTOMER	R'S COPY	( ) (	TICK	CET NO.
	A conditions listed	Mai	tin N	lari	otta	ner tea si terrettea	
M	Martin		1503 LBJ	-		5793	
	Marietta	is product to the r	Suite		and strength to		
			Dallas, T	x 75234	h la enmures	remoteus will Treo	tho
			elvona ed l	Charleman	A STATE OF THE PROPERTY OF T	(Aleksenia)	A CONTRACTOR OF THE PROPERTY O
LOAD TIME	то јов	ARRIVE JOB SITE	BEGIN P	OUR	FINISH POUR	LEAVE JOB SITE	ARRIVE PLANT
9:28	9:41	10:04	10	10	O SQUARED	art harrong team	1
WATER ADDED ON	N JOB AT CUSTOMER'	S REQUEST	GAL.		R SIGNATURE	+	
ALLOWABLE WATE	ER (withheld from batch	1) 15.3 D BY + escal	GAL.	DELIVE	BY OF THESE M	ATERIALS IS SUBJEC	CT TO THE TERMS ANI
CYLINDER TAKEN			Duwing o	CONDIT	TIONS ON THE		EOF AS ACCEPTED B
	ATER ADDED TO TH	Company of the control of the contro		SIGNAT	URE ABOVE.		
	STOMER'S RISK.	D IN EXCESS OF S	PECIFIED				
TEXAS A	ND DELIVERY ADDRESS	ment car area and r		PLANT 517	TRUCK ORDER	R NO. SLUMP P	O. #/JOB/LOT GRID
TTI-Rive	rside Campus			DRIVER NAM	ven Albrec	a steema tartic a	DATE 25/19
				CUSTOMER		ECT CUM. QTY	ORDERED QTY
LOAD QUANTITY P	PRODUCT CODE DES	na lilly daine and	mish side	783	559 79		9.80
	VD925 R9Z5	CRIPTION	98, 2, 500		ATT SHEET THE PARTY OF	UNITPRICE	AMOUNT
	answallen 1898		SHT CHAR				mark -
SPECIAL DELIVERY INS	STRUCTIONS	W) BEFORE SILVE WORKE		TO HE SHE	THE VIOLENCE OF		DDIE
SOUTH 28	18, RIGHT ON LIS CAMPUS TH	LEONARD RR. F	RIGHT ON	HWY	47 LEFT	SALES TAX	
			-			TOTAL	
	JSE ALKALI BURNS. ON REVERSE SIDE.				500.0	FIRE USE ONLY FOR	2004
SEE WATININGS O	IN REVERSE SIDE.		3/		FOR O	FFICE USE ONLY FOF	TIVI:
			- Parkers		AND THE PERSON NAMED IN		
Truck 7131	Driver 934547		5793157	cket 1	Num Ticke 80621	t ID Time I 9:28 1	)ate* 11/25/19
OH MINI REVE	934547 # Mix Code DS R9Z50547	Returned			Mix Age		ID
Material D	esign Qty Requ		% Var -0.23%	% Moist 6.40%	ure Actual Wa	t	
	525 15 4742 1302 15 12387	1b 11720 1b 1b 4720 1b 1b 12400 1b	-0.46%	0.35% 5.40%	M 2 61		
Material D 1MRG 3/8MPG SAND-1 CMT-1/II FLYMSH-C He0 TY-610 Actual	1300 lb 11747 525 lb 4742 1302 lb 12387 528 lb 4752 127 lo 1143 256 lb 1444	1b 4750 1b 1b 1140 1b 1b 1443 1b	0,11% -0,04% -0,26%				
7Y-610 Actual	21 oz 190 Num Batches: 1		-0.05% -0.04%		173 91		
- Load Total: 3	36185 lb Design	0.391 Water/Cement uck: 0.0 gl Adjust	0.391 T Water: 0.00	Desi	gn 276.1 gl	Actual 260.8 gl	To Add: 15.3 gl
			16		- 17 AV 104XLT X	en ger und	
Eliter to pe			A THE				

#### **CONCRETE COMPRESSIVE STRENGTH TEST REPORT**

Report Number: A1171057.0071

Service Date: 11/25/19

Report Date: 01/07/20 Revision 2 - 43-day results

PO #612831-01 Task:

College Station, TX 77845-5765 979-846-3767 Reg No: F-3272

Client **Project** 

Texas Transportation Institute Riverside Campus Attn: Gary Gerke Riverside Campus TTI Business Office Bryan, TX

3135 TAMU

College Station, TX 77843-3135 Project Number: A1171057

Material Information

Specified Strength: 5,000 psi @ 28 days

Mix ID: R9Z50547 Supplier: Martin Marietta

Batch Time: 0928 Plant: 617 Truck No.: 7131 Ticket No.: 5793157

Field Test Data

Yield (Cu. Yds.):

Test Specification Result Slump (in): 4 3/4 Not Specified 1.0 Air Content (%): Not Speciified Concrete Temp. (F): 72 40 - 95 40 - 95 Ambient Temp. (F): 69 Plastic Unit Wt. (pcf): 148.2 Not Specified

#### Sample Information

Sample Date: 11/25/19 Sample Time: 1020

Sampled By: David Thompson Weather Conditions: Clear, no wind

Accumulative Yards: Batch Size (cy): Direct Discharge

Placement Method: Water Added Before (gal): 15

Water Added After (gal): Sample Location: Traffic barricade

Maximum

Traffic barricade (PO #612831-01) Placement Location:

Compressive

#### **Laboratory Test Data**

Set No.	Specimen ID	Avg Diam. (in)	Area (sq in)	Date Received	Date Tested	Test (days)	Load (lbs)	Strength (psi)	Fracture Type	Tested By
1	A	4.00	12.57	11/26/19	12/02/19	7 F	62,170	4,950		JCM
1	В	4.00	12.57	11/26/19	01/07/20	43 F	81,920	6,520	1	BJA
1	C	4.00	12.57	11/26/19	01/07/20	43 F	83,210	6,620	1	BJA
1	D	4.00	12.57	11/26/19	01/07/20	43 F	87,260	6,940	1	BJA
1	F	4.00	12.57	11/26/19	01/07/20	43 F	77,210	6,140	1	BJA
1	E			11/26/19		Hold				
Initial	Cure: Cure	Blanket		Final C	ure: Field Cu	red				

Comments: F = Field Cured

Samples Made By: Terracon

Obtain samples of fresh concrete at the placement locations (ASTM C 172), perform required field tests and cast, cure, and Services:

test compressive strength samples (ASTM C 31, C 39, C 1231).

Terracon Rep.: David Thompson Start/Stop: \*\*

Reported To: Contractor:

Report Distribution:
(1) Texas Transportation Institute, Gary Gerke (1) Terracon Consultants, Inc., Andrea Allen

R	eviev	ved	$\mathbf{R}\mathbf{v}$	

Andrea Allen Project Manager

Test Methods: ASTM C 31, ASTM C143, ASTM C231, ASTM C1064

The tests were performed in general accordance with applicable ASTM, AASHTO, or DOT test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

CR0001, 11-16-12, Rev.6

<sup>\*\*</sup>Time and mileage shown on Report No. A1171057.0070.

Proving-Ground¶ Texas A&M Transportation Institute Proving-Ground¶ 7811   Texas A&M University¶ 3100-SH-47. Bilds 7901   College-Station. TX-77843   Phone 970-946-9375   Thome 970-946-9375	QF·7.3-01··Concrete∙ Sampling¤	Doc.·No.¶ ¶ <i>QF-7.3-01</i> □	Issue Date: ← ← 2018-06-18¤
Quality·Form <sup>12</sup>	Prepared by: Wanda-L. Menges¶	Revision: ↔	Page:¶
	Approved by: Darrell L. Kuhn¤	6¤	1 of 1□

The information conf	tained in this document is c	onfidential-to-TTI-Proving-0	Ground.		
Project No:	612831	Casting Date:	12/9/2019	Mix Design (psi): 50	000
Technician Taking Sample	TERRA	4CON	Technician Breaking Sample	TERRACON	
Signature of Technician Taking Sample	TERRA	ACON	Signature of Technician Breaking Sample	TERRACON	
raning campic.			. Droaming campic		
Load No.	Truck No.	Ticket No.		on (from concrete map)	
		Ticket No.		on (from concrete map)	

Load No.	Break Date	Cylinder Age	Total Load (lbs)	Break (psi)	Average
	T1-		EETS FROM TERRAG		
		SEE ATTACHED SIT	EETS TROM TERRISA		
	<u> </u>				
	ļ				

TR No. 612831-01 77 2020-05-07

CUST	OMER'S COPY	TICKET NO.
Martir	Marietta	
MORTID	LBJ Freeway	5817467
	Suite 400	
	as, Tx 75234	
	10000000000000000000000000000000000000	INTERNACIONAL DE LA CONTRACTOR DE LA CON
LOAD TIME TO JOB ARRIVE JOB SITE E	EGIN POUR FINISH POUR	4 6 7 *
7.23 7.45 8.00	- Privish Pour	LEAVE JOB SITE ARRIVE PLANT
WATER ADDED ON JOB AT CUSTOMER'S REQUEST 15	GAL. CUSTOMER SIGNATURE	•
ALLOWABLE WATER (withheld from batch)	GAL. X	Den dans de restau
TEST CYLINDER TAKEN Q YES Q NO BY	DELIVERY OF THESE MATE	RIALS IS SUBJECT TO THE TERMS AN
CYLINDER TAKEN BEFORE AFTER WATER  ADDITIONAL WATER ADDED TO THIS CONCRETE WILL REDI	CONDITIONS ON THE REV	ERSE SIDE HEREOF AS ACCEPTED B
ITS STRENGTH. ANY WATER ADDED IN EXCESS OF SPECIF SLUMP IS AT CUSTOMER'S RISK.	IED	
CUSTOMER NAME AND DELIVERY ADDRESS	PLANT TRUCK ORDER NO.	SLUMP P.O. #/JOB/LOT GRID
- TTI-Riverside Campus	617 7165 201	- INTERESTER -
	Demonites Willam	12/9/19
The state of the second st	CUSTOMER NUMBER PROJECT 7954	CUM. QTY ORDERED QTY
LOAD QUANTITY PRODUCT CODE DESCRIPTION		UNIT PRICE AMOUNT
9.00 CYDS R9Z30547 COM, RG, Z,	HARGE MAN MAN AND AND AND AND AND AND AND AND AND A	an addition yn den an englig in it banken der a graften fyll gronner foedern a accordance policy
SPECIAL DELIVERY INSTRUCTIONS  ABABART UN LEUNARD RT ON HWY-47-LFT 1	NTO DISCRETE SALES	TAX
CAMPUS WILL MEET AT BATE		
	ТОТА	AL .
DANGER! MAY CAUSE ALKALI BURNS. SEE WARNINGS ON REVERSE SIDE.	EOR OFFICE	USE ONLY FORM:
	TOTOTTICE	OSC ONLY TOTAWI.
HOR SHALL TO THE STATE AND A TOUCH	/ar % Moisture Actual Wat 3% 0.25% M 4 gl 3% 0.25% M 1 gl 3% 4.00% M 59 gl	81905
275-10	Design 276.1 gl 0.0 gl / Load Trim Water: -1.7	Actual 253.1 gl To Add: 15.0 gl

#### **CONCRETE COMPRESSIVE STRENGTH TEST REPORT**

Report Number: A1171057.0078

Service Date: 12/09/19

Report Date: 01/07/20 Revision 1 - 29-day results

PO #612831 Task:

College Station, TX 77845-5765 979-846-3767 Reg No: F-3272

Client **Project** 

Texas Transportation Institute Riverside Campus Attn: Gary Gerke Riverside Campus TTI Business Office Bryan, TX

3135 TAMU

College Station, TX 77843-3135 Project Number: A1171057

Material Information Sample Information

Specified Strength: 5,000 psi @ 28 days Sample Date: 12/09/19 Sample Time: 0825

Sampled By: Alexander Dunigan Mix ID: R9Z50547 Weather Conditions: Cloudy, no wind

Supplier: Martin Marietta Accumulative Yards: Batch Size (cy):

Batch Time: 0723 Plant: 617 Placement Method: Direct Discharge

Truck No.: Ticket No.: 5817467 Water Added Before (gal): 0 Water Added After (gal):

Field Test Data Sample Location: South end of 3rd barrier from the south Test Result Specification Placement Location: Concrete barriers

Slump (in): 6 1/2 Not Specified

Air Content (%): Not Specified Concrete Temp. (F): 75 40 - 95 70 40 - 95 Ambient Temp. (F): Plastic Unit Wt. (pcf): 147.4 Not Specified

Yield (Cu. Yds.):

**Laboratory Test Data** Maximum Compressive

Set	Specimen	Avg Diam.	Area	Date	Date	Test	Load	Strength	Fracture	Tested
No.	ID	(in)	(sq in)	Received	Tested	(days)	(lbs)	(psi)	Type	By
1	A	4.00	12.57	12/10/19	01/07/20	29 F	75,380	6,000	3	BJA
1	В	4.00	12.57	12/10/19	01/07/20	29 F	73,690	5,860	3	BJA
1	C	4.00	12.57	12/10/19	01/07/20	29 F	75,470	6,010	3	BJA
1	D			12/10/19		Hold				

Final Cure: Field Cured

Comments: F = Field Cured

Initial Cure: Outside

Samples Made By: Terracon

Obtain samples of fresh concrete at the placement locations (ASTM C 172), perform required field tests and cast, cure, and Services:

test compressive strength samples (ASTM C 31, C 39, C 1231).

Terracon Rep.: Alexander Dunigan Start/Stop: 0715-0930

Reported To: Contractor:

Report Distribution:
(1) Texas Transportation Institute, Gary Gerke
(1) Terracon Consultants, Inc., Andrea Allen Reviewed By:

Andrea Allen Project Manager

Test Methods: ASTM C 31, ASTM C143, ASTM C231, ASTM C1064

The tests were performed in general accordance with applicable ASTM, AASHTO, or DOT test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

CR0001, 11-16-12, Rev.6

Texas A&M Transportation Institute Proving Ground   Texas A&M University   Texas A&M University   Texas A&M University   College-Station, TX-77643   Phone 979-9445-9375   Texas A&M University   Texas A&M Un	QF·7.3-01···Concrete· Sampling¤	Doc.·No.¶ ¶ <i>QF-7.3-01</i> □	Issue-Date: ← ↓ ← ↓ 2018-06-18¤
	Prepared·by:·Wanda·L.·Menges¶	Revision: ↔	Page:¶
	Approved·by:·Darrell·L.·Kuhn¤	6¤	1-of-1¤

_	ality·Form¤	Approved by:	Wanda L. Menges¶ Darrell L. Kuhn¤	Kevision: • 6□	1-of-1¤
The information conf	ained-in-this-document-is-d	confidential to TTI Proving	Ground.		
Project No:	612831	Casting Date:	12/18/2019	Mix Design (psi):_	5000
Name of Technician Taking Sample	TERR	ACON	Name of Technician Breaking Sample	TERRA	CON
Signature of Technician Taking Sample	TERR		Signature of Technician Breaking Sample	TERRA	CON
Load No.	Truck No.	Ticket No.	Locat	ion (from concrete	map)
T <b>1</b>	8130	5836268	Remaining F-shape	e Barriers	
Load No.	Break Date	Cylinder Age	Total Load (lbs)	Break (psi)	Average
	<b>⊤1</b> -	SEE ATTACHED SHE	EETS FROM TERRAC	ON	

			CUSTOMER	'S COPY			TIC	CKET NO.	
		Mar	tio N/	loria	otto		al Pullinden		
AA	Martin		tin M				58		
	Martin	Service of	503 LBJ F		У				
	Mariotta		Dallas, Tx						
			118	DINE NULL	11 15 18 8 11111	S HILLS	11919 91191 191	[ [ ] [ ] [	
LOAD TIME	то Јов	ARRIVE JOB SITE	BEGIN PO	OUR	FINISH POL	UR	LEAVE JOB SIT	E AF	RRIVE PLANT
11.54	17.04	12.30	1/2.	34	Capital Cal	L. Bil	ggirtater team		: 2
	1100		10+5	CUSTOME	R SIGNATURE			TO SHARE	
	JOB AT CUSTOMER'S	State of the state	GAL.	X		Number of	place safet to	191911	
TEST CYLINDER TA	ER (withheld from batch AKEN YES NO		UAL.	DELIVE	RY OF THES	E MATE	RIALS IS SUBJ	ECT TO TH	E TERMS AN
CYLINDER TAKEN	□ BEFORE □	The state of the s			TIONS ON TI		ERSE SIDE HE	REUF AS A	ACCEPTED B
ADDITIONAL WA	TER ADDED TO TH	IS CONCRETE WILL	REDUCE	SIGNAL	UNE ABOVE				
SLUMP IS AT CU	ANY WATER ADDE	D IN EXCESS UF S	PECIFIED						
CUSTOMER NAME A	ND DELIVERY ADDRESS		Lander Like	PLANT	**********	RDER NO.	SLUMP	P.O. #/JOB/LO	
	& M LINITERSI			DRIVER NAI	8130	201	Any Delino	DATE	-PV
				Ray	William			1/18/19	
				CUSTOMER	NUMBER F	PROJECT	CUM. QTY	ORD	ERED CITY
LOAD QUANTITY F	PRODUCT CODE DES	SCRIPTION					UNITPRICE	AN	IOUNT
	VDS 8975			MO. RE					
1.00 e	a 1298						~ /		
					711	10	51		
0	A CONTRACTOR				610	40-			
V. A	1/1 10								
Roel	Kt gurang		min of the	The last			Salita Salita	annua 1	
SPECIAL DELIVERY IN	STRUCTIONS ON LEGNARD R	T ON HWY-47-	LFY INTE			SALE	S TAX		
						то	DTAL		
	USE ALKALI BURNS. ON REVERSE SIDE.				FC	OR OFFIC	CE USE ONLY F	ORM:	
SEE WARNINGS	ON REVERSE SIDE.								- milita
								-	N. Caralle
Truck	Driver 726612 e Mix Code 'DS R9750547			icket				12/18	
Load Siz	e Mix Code				Mix Age				
7,00 CY	DS R9750547	uired Batched	4 Var	% Mai					
Material 1"RG 3/8"PS	1300 16 911 525 1b 366	8 16 9100 lb 2 15 3500 lb	+ -0.96%	% Mei 0.20% 0.20%					
SAMO-1			0.07% -0.16%	4,00%					
FLYASH-L H20 ZY-610	528 16 369 127 16 88 256 16 122	2 1b 880 1b 2 1b 1220 1b	-0.86%-						
			-1.24%						
Lead Tetal:	Num Batches: 1 28075 1b Desi	gn 0.391 Water/Ceme	nt 0,392 T		ign 214,7 gt		Actual 194.8	gl To Ado	i: 14.9 gl
Sluip: 5.00		ruck: 5.0 gl Adju		e dr / r	oad Trie Wa				
The William									The state of the s

TR No. 612831-01 81 2020-05-07

#### **CONCRETE COMPRESSIVE STRENGTH TEST REPORT**

Report Number: A1171057.0081

Service Date: 12/18/19

Report Date: 01/07/20 Revision 1 - 20-day results

PO #612831 Task:

College Station, TX 77845-5765 979-846-3767 Reg No: F-3272

1243

Client **Project** 

Texas Transportation Institute Riverside Campus Attn: Gary Gerke Riverside Campus TTI Business Office Bryan, TX

3135 TAMU

College Station, TX 77843-3135

Project Number: A1171057

Maximum

Compressive

Material Information Sample Information

Specified Strength: 4,000 psi @ 28 days Sample Date: 12/18/19 Sample Time: Sampled By: Cullen Turney

Mix ID: R9Z50547 Weather Conditions: Clear, moderate wind

Supplier: Martin Marietta Accumulative Yards: Batch Size (cy): 7 Batch Time: 1154 Direct Discharge Plant: 617 Placement Method:

Truck No.: Ticket No.: 5836268 Water Added Before (gal): 10

Water Added After (gal): Field Test Data Sample Location: Third F beam

Apron 4 (PO #612831) Test Result Specification Placement Location: Not Specified

Air Content (%): Not Specified Concrete Temp. (F): 40 - 95 40 - 95 Ambient Temp. (F): 55 Plastic Unit Wt. (pcf): Not Specified

Yield (Cu. Yds.):

**Laboratory Test Data** 

Set No.	Specimen ID	Avg Diam. (in)	Area (sq in)	Date Received	Date Tested	Test (days)	Load (lbs)	Strength (psi)	Fracture Type	Tested By
1	A	6.00	28.27	12/19/19	01/07/20	20 F	162,820	5,760	1	BJA
1	В	6.00	28.27	12/19/19	01/07/20	20 F	166,840	5,900	1	BJA
1	C	6.00	28.27	12/19/19	01/07/20	20 F	163,510	5,780	1	BJA
1	D			12/19/19		Hold				
Initial	Cure: Outsi	ide		Final C	ure: Field Cur	red				

Comments: Not tested for plastic unit weight. F = Field Cured

Samples Made By: Terracon

Obtain samples of fresh concrete at the placement locations (ASTM C 172), perform required field tests and cast, cure, and Services:

test compressive strength samples (ASTM C 31, C 39, C 1231).

Terracon Rep.: Cullen Turney Reported To: Contractor:

Report Distribution:
(1) Texas Transportation Institute, Gary Gerke (1) Terracon Consultants, Inc., Andrea Allen

R	evi	ew	ed	Bv:	

Start/Stop: \*\*

Andrea Allen Project Manager

Test Methods: ASTM C 31, ASTM C143, ASTM C231, ASTM C1064

The tests were performed in general accordance with applicable ASTM, AASHTO, or DOT test methods. This report is exclusively for the use of the client indicated above and shall not be reproduced except in full without the written consent of our company. Test results transmitted herein are only applicable to the actual samples tested at the location(s) referenced and are not necessarily indicative of the properties of other apparently similar or identical materials.

CR0001, 11-16-12, Rev.6

<sup>\*\*</sup>Time and mileage shown on Report No. A1171057.0080.



#### CERTIFIED MILL TEST REPORT For additional copies call 830-372-8771

We hereby certify that the test results presented here are accurate and conform to the reported grade specification

Quality Assurance Manager

HEAT NO.:3090295 S CMC Construction Svcs College Stati S CMC Construction Svcs College Stati Delivery#: 82812501 SECTION: REBAR 16MM (#5) 20'0" 420/60 BOL#: 73150643 GRADE: ASTM A615-18e1 Gr 420/60 L 10650 State Hwy 30 I 10650 State Hwy 30 CUST PO#: 825407 ROLL DATE: 08/13/2019 College Station TX P College Station TX CUST P/N: MELT DATE: 08/04/2019 US 77845-7950 US 77845-7950 DLVRY LBS / HEAT: 24090.000 LB Cert. No.: 82812501 / 090295A371 979 774 5900 T 979 774 5900 DLVRY PCS / HEAT: 1155 EA 0 0

Characteristic	Value	Characteristic	Value	Characteristic Value
С	0.42%	Bend Test Diameter	2.188IN	Value
Mn	0.90%		2.100114	
P	0.010%			
S	0.039%	612831 - 3		
Si	0.23%	6/21/1-/		
Cu	0.32%			
Cr	0.13%			
Ni	0.19%			
Mo	0.054%			The Following is true of the material represented by this MTR:
V	0.000%			*Material is fully killed
Сь	0.001%			*100% melted and rolled in the USA
Sn	0.011%			*EN10204:2004 3.1 compliant
AI	0.000%			*Contains no weld repair
				*Contains no Mercury contamination
Yield Strength test 1	67.7ksi			*Manufactured in accordance with the latest version
Tensile Strength test 1	105.1ksi			of the plant quality manual
Elongation test 1	16%			*Meets the "Buy America" requirements of 23 CFR635.410, 49 CFR 66
Elongation Gage Lgth test 1	8IN			*Warning: This product can expose you to chemicals which are
Tensile to Yield ratio test1	1.55			known to the State of California to cause cancer, birth defects
Bend Test 1	Passed			or other reproductive harm. For more information go
				to www.P65Warnings.ca.gov

REMARKS:



#### CERTIFIED MILL TEST REPORT For additional copies call 830-372-8771

We hereby certify that the test results presented here are accurate and conform to the reported grade specification

**Quality Assurance Manager** 

HEAT NO.:3090116 SECTION: REBAR 25MM (#8) 20'0" 420/60 GRADE: ASTM A615-18e1 Gr 420/60 ROLL DATE: 08/03/2019 MELT DATE: 07/27/2019 Cert. No.: 82799443 / 090116A041	S CMC Construction Svcs College State O L 10650 State Hwy 30 D College Station TX US 77845-7950 T 979 774 5900 O	H I 10650 State Hwy 30 P College Station TX US 77845-7950	Delivery#: 82799443 BOL#: 73129113 CUST PO#: 824142 CUST P/N: DLVRY LBS / HEAT: 12816.000 LB DLVRY PCS / HEAT: 240 EA
--	--	---	---

Characteristic	Value	Characteristic Value	Characteristic Value
С	0.42%	Bend Test Diameter 5.000IN	
Mn	1.25%	V V	
P	0.013%		
S	0.043%		
Si	0.22%	1 10001 7	
Cu	0.31%	612831-3	
Cr	0.07%		
Ni	0.08%		
Мо	0.027%		The Following is true of the material represented by this MTR:
V	0.000%		*Material is fully killed
Cb	0.001%		*100% melted and rolled in the USA
Sn	0.010%		*EN10204:2004 3.1 compliant
AI	0.000%		*Contains no weld repair
			*Contains no Mercury contamination
Yield Strength test 1	64.6ksi		*Manufactured in accordance with the latest version
Tensile Strength test 1	107.1ksi		of the plant quality manual
Elongation test 1	15%		*Meets the "Buy America" requirements of 23 CFR635.410, 49 CFR 66
Elongation Gage Lgth test 1	8IN		*Warning: This product can expose you to chemicals which are
Tensile to Yield ratio test1	1.66		known to the State of California to cause cancer, birth defects
Bend Test 1	Passed		or other reproductive harm. For more information go
			to www.P65Warnings.ca.gov

REMARKS:



#### CERTIFIED MILL TEST REPORT For additional copies call 830-372-8771

We hereby certify that the test results presented here are accurate and conform to the reported grade specification

Quality Assurance Manager

HEAT NO.:3088704 S CMC Construction Svcs College Stati S CMC Construction Svcs College Stati Delivery#: 82794206 SECTION: REBAR 19MM (#6) 20'0" 420/60 BOL#: 73120575 GRADE: ASTM A615-18e1 Gr 420/60 L 10650 State Hwy 30 I 10650 State Hwy 30 CUST PO#: 823593 ROLL DATE: 05/31/2019 D College Station TX P College Station TX CUST P/N: MELT DATE: 05/21/2019 US 77845-7950 US 77845-7950 DLVRY LBS / HEAT: 12978.000 LB Cert. No.: 82794206 / 088704A619 979 774 5900 T 979 774 5900 DLVRY PCS / HEAT: 432 EA 0 0

Characteristic	Value	Characteristic Value	Characteristic Value
С	0.44%		
Mn	0.88%		
P	0.010%		
S	0.036%	_	
Si	0.17%	612831-3	
Cu	0.29%	0,000	
Cr	0.10%		
Ni	0.25%		
Мо	0.092%		The Following is true of the material represented by this MTR:
V	0.000%		*Material is fully killed
Сь	0.001%		*100% melted and rolled in the USA
Sn	0.011%		*EN10204:2004 3.1 compliant
AI	0.000%		*Contains no weld repair
V: 116			*Contains no Mercury contamination
Yield Strength test 1	69.4ksi		*Manufactured in accordance with the latest version
Tensile Strength test 1	109.8ksi		of the plant quality manual
Elongation test 1	15%		*Meets the "Buy America" requirements of 23 CFR635.410, 49 CFR 66
Elongation Gage Lgth test 1	8IN		*Warning: This product can expose you to chemicals which are
Bend Test 1	Passed		known to the State of California to cause cancer, birth defects
Bend Test Diameter	3.750IN		or other reproductive harm. For more information go
			to www.P65Warnings.ca.gov

REMARKS :



**CMC STEEL TEXAS** 1 STEEL MILL DRIVE **SEGUIN TX 78155-7510** 

#### **CERTIFIED MILL TEST REPORT** For additional copies call 830-372-8771

We hereby certify that the test results presented here are accurate and conform to the reported grade specification

**Quality Assurance Manager** 

HEAT NO.:3089816 SECTION: REBAR 13MM (#4) 20'0 GRADE: ASTM A615-18e1 Gr 420 ROLL DATE: 07/16/2019 MELT DATE: 07/16/2019 Cert. No.: 82794205 / 089816A130	/60	O L D	CMC Construction Svcs College Stati 10650 State Hwy 30 College Station TX JS 77845-7950 979 774 5900	S H I P T O	CMC Construction Svo 10650 State Hwy 30 College Station TX US 77845-7950 979 774 5900	College Stati Delivery#: 82794205 BOL#: 73120574 CUST PO#: 823592 CUST P/N: DLVRY LBS / HEAT: 2191. DLVRY PCS / HEAT: 164 E		92 AT: 2191.000 LB
Characteristic	Value		Characteristic		Value		Characteristic	Value
С	0.44%		Bend Test Diame	eter	1.750IN			2000000
Mn	0.79%							
P	0.013%							
S	0.031%		1.10031-	1	2			
Si	0.18%		612831-1	1	0,0			
Cu	0.33%							
Cr Ni	0.11% 0.20%							
Mo	0.20%					AND YELL ON THE ST.		
V	0.000%						rue of the material repre	esented by this MTR:
Cb	0.000%					*Material is fully ki		
Sn	0.001%					*100% melted and		
Al	0.000%					*EN10204:2004 3.1		
55	000,0					*Contains no weld *Contains no Merc	·	
Yield Strength test 1	65.8ksi					Committee of the Commit	ccordance with the latest	orgia -
Tensile Strength test 1	105.1ksi					of the plant qualit		Ver SIO(1)
Elongation test 1	15%					The state of the s	E1 16 10 (15 (15 (15 (15 (15 (15 (15 (15 (15 (15	3 CFR635.410, 49 CFR 66
Elongation Gage Lgth test 1	8IN						nduct can expose you to c	
Tensile to Yield ratio test1	1.60						te of California to cause of	
Rond Toot 1	Decead							

REMARKS :

Bend Test 1

Passed

or other reproductive harm. For more information go

to www.P65Warnings.ca.gov



#### CERTIFIED MILL TEST REPORT For additional copies call 830-372-8771

We hereby certify that the test results presented here are accurate and conform to the reported grade specification

Rolando A Dav

Quality Assurance Manager

HEAT NO.:3090295 SECTION: REBAR 16MM (#5) 20'0" 420/60 GRADE: ASTM A615-18e1 Gr 420/60 ROLL DATE: 08/13/2019 MELT DATE: 08/04/2019 Cert. No.: 82812501 / 090295A371	S CMC Construction  L 10650 State II  D College Stati US 77845-799  T 979 774 5900	on TX	H I P	CMC Construction Svcs College Stati 10650 State Hwy 30 College Station TX US 77845-7950 979 774 5900	Delivery#: 828125 BOL#: 73150643 CUST PO#: 82540 CUST P/N: DLVRY LBS / HEADLVRY PCS / HEADLVRY P	7 XT: 24090.000 LB
Characteristic Value		Characteristic		Value	Characteristic	Value
C 0.42%		Bend Test Diame	eter			value

Characteristic	Value	Characteristic Value	Characteristic Value
С	0.42%	Bend Test Diameter 2.188IN	7 4140
Mn	0.90%	2.10011	
P	0.010%	1.0001	
S	0.039%	612831-3	
Si	0.23%		
Cu	0.32%		
Cr	0.13%		
Ni	0.19%		
Mo	0.054%		The Following is true of the material represented by this MTR:
V	0.000%		*Material is fully killed
СЬ	0.001%		*100% melted and rolled in the USA
Sn	0.011%		*EN10204:2004 3.1 compliant
Al	0.000%		*Contains no weld repair
			*Contains no Mercury contamination
Yield Strength test 1	67.7ksi		*Manufactured in accordance with the latest version
Tensile Strength test 1	105.1ksi		of the plant quality manual
Elongation test 1	16%		The state of the s
Elongation Gage Lgth test 1	8IN		*Meets the "Buy America" requirements of 23 CFR635.410, 49 CFR 66
Tensile to Yield ratio test1	1.55		*Warning: This product can expose you to chemicals which are
Bend Test 1	Passed		known to the State of California to cause cancer, birth defects
	1 CECTE		or other reproductive harm. For more information go to www.P65Warnings.ca.gov

REMARKS :

Proving-Ground¶ 3100-SH-47,-Bldg-7091¶ Bryan,-TX-77807	Texas A&M Transportation Institute Texas A&M-University[ Texas A&M-University[ Phone-979-845-63767]	QF·7.3- <u>01··Concrete</u> · Sampling¤	Doc.·No.¶ ¶ <i>QF-7.3-01</i> ¤	Issue-Date: ←
Qual	ity·Form¤	Prepared by: Wanda L. Menges¶ Approved by: Darrell L. Kuhn¤	Revision: ↔ 6¤	Page:¶ 1-of-1¤

Project No:_	612831 Ca	asting Date: 10/3/2019	Mix Design (psi):	4000
Name of Technician Taking Sample	TERRACON	Name of Technician Breaking Sample		
Signature of Technician Taking Sample	TERRACON	Signature of Technician Breaking Sample		

Load No.	Truck No.	Ticket No.	Location (from concrete map)
			Lower 3 ft of all Barrier, remaining for 3 barrier from
T1	7211	5695733	south end
			half of the top of the 4th barrier, top of remaining 2
T2	8162	5695885	barriers on north side

Load No.	Break Date	Cylinder Age	Total Load (lbs)	Break (psi)	Average
	T <b>1</b> -	SEE ATTACHED SH	EETS FROM TERRAC	ON	
	T <b>2</b> -	SEE ATTACHED SH	EETS FROM TERRAC	ON	

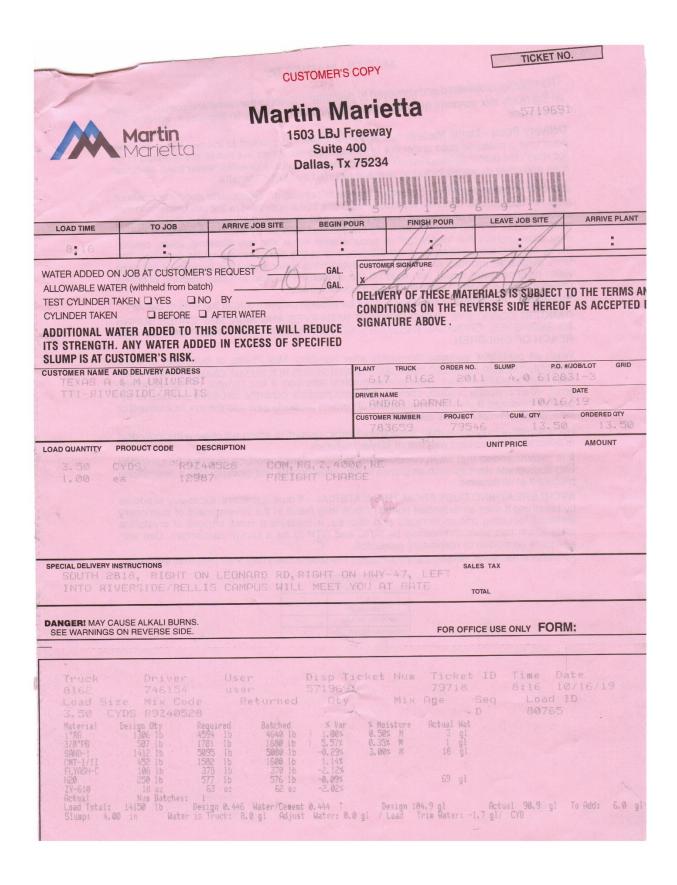
	MER'S COPY	10	TICKET	NO.
Martin Martin Marietta Marietta			\$6958	85
Marietta 1503 LBJ Suite				
Dallas, T	x 75234	esiences )	Messec a lavo lak	
Animal and a second a second and a second an				a series a
LOAD TIME TO JOB ARRIVE JOB SITE BEGIN F	##181 Pills Bills 18	5 8	8 5 *	
	POUR FINISH F	POUR	LEAVE JOB SITE	ARRIVE PLANT
10:17 // :/9 // :/9 :	Tayona da		-	•
WATER ADDED ON JOB AT CUSTOMER'S REQUEST GAL.  ALLOWABLE WATER (withheld from batch) GAL.	CUSTOMER SIGNATURE	69	A1	H. State of the second
TEST CYLINDER TAKEN DYES DNO BY	DELIVERY OF THE			
CYLINDER TAKEN BEFORE ARTER WATER	CONDITIONS ON SIGNATURE ABOV		ISE SIDE HEREO	F AS ACCEPTED E
ADDITIONAL WATER ADDED TO THIS CONCRETE WILL REDUCE ITS STRENGTH. ANY WATER ADDED IN EXCESS OF SPECIFIED	REJUNA DE ESTR	BEAR JABI		
SLUMP IS AT CUSTOMER'S RISK.  CUSTOMER NAME AND DELIVERY ADDRESS	IPLANT TRUCK	ODDED NO	natherican da dagg	15/
TEXAS A & M UNIVERSENT ,	3 617 B162	ORDER NO.	SLUMP P.O. #/	JOB/LOT GRID
TTI-RIVERGIDE/RELLIS a solesso unto abbotico pusta	DRIVER NAME ANDRA DARK		00 to 90 10/3 X	DATE
	CUSTOMER NUMBER	PROJECT	CUM. QTY	ORDERED QTY
LOAD QUANTITY PRODUCT CODE DESCRIPTION	Energy entitle of		NITPRICE	AMOUNT
- 5.00 - CVDS . Pozagisce - contine, z. 400	ao, RE,	potencial i		
1.00 ea 12987 FREIBHT CHAI	RGE	X220 04 07 4 07 00	· Street Sections	The same of the sa
We extend to the control of the cont				
SOUTH 2818, RIGHT ON LEONARD RD, RIGHT ON	The state of the s	SALES TA		
INTO RELLIS CAMPUS WILL MEET AT THE TURN	A AROUND	TOTAL	C-A	
	The second secon	IOIAL		
DANGER! MAY CAUSE ALKALI BURNS. SEE WARNINGS ON REVERSE SIDE.	FOR OF	FICE USE ONI	LY FORM: 25	73205
	the second difference of the ten of the second and		<u></u>	
Truck' Driver Wer Disp Ti	cket sum w	X 1 10	/ Time Dat	
8162 748154 User 195885 Load Size Mix Code (Returned ) Oty 5.00 CVDS 89740538			1 1 T KN E T ( T KN)	/3/19 1
			Load II 80431	
Material Design Oty Required Barrhed 7 Var 1786 1306 lb 6536 lb 6640 lb 1292 3.8706 507 lb 6536 lb 6640 lb 1292 9880-1 110 lb 7326 lb 73667lb 0.463	Moisture Acti	gl Wat 3 gl		
SAMPLE 1419 16 CONTRACT MACHINE TRACTE LANGE AND LONGER	0.40V H 0.25X M 3.50V M	1 gl 31 gl		
H20 50 1h	A TON COMME	Paris Por		
Oct 121 102 7 0.88%	1	1	1	
Load Total: 20175 1b Design 0.446 Water/Cement 0.448 T Slump: 5.00 in # Mater in Truck: 4.0 gl Adjust Water: 0.0	Design 149.8 of	Ac ater: -1.7 dl	tual 137.6 gl T	o Add: 8.2 gl
		N.		

	MER'S COPY	TICKET N	0.
Martin Martin Martin N  1503 LBJ Suite Dallas, T	Freeway 400	5695733	
LOAD TIME TO JOB ARRIVE JOB SITE BEGIN F	POUR FINISH POL	JR LEAVE JOB SITE	ARRIVE PLANT
9:43 10:02 10:25 10:	31 //	7 :1	
WATER ADDED ON JOB AT CUSTOMER'S REQUEST ALLOWABLE WATER (withheld from batch) TEST CYLINDER TAKEN YES NO BY CYLINDER TAKEN BEFORE AFTER WATER ADDITIONAL WATER ADDED TO THIS CONCRETE WILL REDUCE ITS STRENGTH. ANY WATER ADDED IN EXCESS OF SPECIFIED SLUMP IS AT CUSTOMER'S RISK.	DELIVERY OF THESE CONDITIONS ON THE SIGNATURE ABOVE	E MATERIALS IS SUBJECT TO TE REVERSE SIDE HEREOF A	THE TERMS AN S accepted e
CUSTOMER NAME AND DELIVERY ADDRESS TEXAS A 2 M LINIVERSI	PLANT TRUCK OR	DER NO. SLUMP P.O. #/JOB	LOT GRID
TTI RIVERBIDE/RELLIS	DRIVER NAME	2036 5.0 61283)	
	LARRY JANTZ	EN 200 10/3/19	
LOAD QUANTITY PRODUCT CODE DESCRIPTION		79346 Lia, 00	ADERED QTY
SPECIAL DELIVERY INSTRUCTIONS		FeUC Offer or every microscopic of the second position of the second	
SOUTH 2818. RIGHT ON LEGMOND ON PROUT OF	FINY-47-LEFT	SALES TAX	
SOUTH 2818, RIGHT ON LEGNARD RD, RIGHT ON INTO RELLIS CAMPUS WILL MEET AT THE TURN	HUY-47-LEFT AROUND	TOTAL	
SOUTH 2818. RIGHT ON LEGMOND ON PROUT OF	AROUND		3203

Texas A&M   Transportation   Institute   Proving Ground	QF·7.3-01··Concrete∙ Sampling¤	Doc.·No.¶ ¶ <i>QF-7.3-01</i> □	Issue-Date: ← ← 2018-06-18¤
• Quality·Form¤	Prepared by: Wanda L. Menges¶	Revision: ↔	Page:¶
	Approved by: Darrell L. Kuhn¤	6¤	1-of-1¤

The information con	tained-in-this-document-is-c	onfidential to TTI Proving	Ground.	00	1-01-12			
Project No:	612831	Casting Date:	10/16/2019	Mix Design (psi):	4000			
Name of Technician Taking Sample		ACON	Name of Technician Breaking Sample	TERR	ACON			
Signature of Technician Taking Sample		ACON	Signature of Technician Breaking Sample	TERR	ACON			
Load No.	Truck No.	Ticket No.	Location (from concrete map)					
T1	7124	5719600	Lower 3 ft of all Ba	rriers				
T2	8162	5719691	Top half of all barriers					
Load No.	Break Date	Cylinder Age	Total Load (lbs)	Break (psi)	Average			
Load No.			Total Load (lbs)		Average			
Load No.	T1-	SEE ATTACHED SHE		ON	Average			
Load No.	T1-	SEE ATTACHED SHE	EETS FROM TERRAC	ON	Average			
Load No.	T1-	SEE ATTACHED SHE	EETS FROM TERRAC	ON	Average			
Load No.	T1-	SEE ATTACHED SHE	EETS FROM TERRAC	ON	Average			
Load No.	T1-	SEE ATTACHED SHE	EETS FROM TERRAC	ON	Average			
Load No.	T1-	SEE ATTACHED SHE	EETS FROM TERRAC	ON	Average			
Load No.	T1-	SEE ATTACHED SHE	EETS FROM TERRAC	ON	Average			

		C	USTOMER'	SCOPY			TICKET	NO.
<b>/</b>	<b>Martin</b> Marietta	silve Lubria el	tin M 503 LBJ F Suite 4 Dallas, Tx	reeway 100	tta Projection of the Control of the	Management of the control of the con	57196	00
	only a severiment	ARRIVE JOB SITE	BEGIN PO	OUR	FINISH POUR	P 1	EAVE JOB SITE	ARRIVE PLANT
LOAD TIME	то Јов	V. 27	X.	31		and the	Note:	
7:55	8:06	0 .	S GAL.	CUSTOMERS	IGNATURE	Bulan	ASVARIORO, III	(8)
ALLOWABLE WATE TEST CYLINDER TO CYLINDER TAKEN		AFTER WATER  IS CONCRETE WILL	GAL.	CONDITIO	OF THESE NONS ON THE RE ABOVE .	NATERIA REVER	ALS IS SUBJECT Se side here	T TO THE TERMS AN OF AS ACCEPTED B
SI LIMP IS AT CU	ISTOMER'S RISK.	-waterway	Curenta	PLANT 1	RUCK ORD	ER NO.		D. #/JOB/LOT GRID
CUSTOMER NAME A	ND DELIVERY ADDRESS	District VIII I at		617	7124	2011	4.0 618	DATE
				DRIVER NAME			100	
TTI-RIVE	RSIDE/RELLIS			Stev	en Albre	cht	19/1	
LOAD QUANTITY	PRODUCT CODE DE	SCRIPTION COM.	, RG, Z, 40 IGHT CHA	CUSTOMER N 7/3/3/6	UMBER PRO	9546	CUM. GTY	ORDERED QTY
LOAD QUANTITY	PRODUCT CODE DE R974 1298	SCRIPTION  10528 COM. FRE	RIGHT CHA	CUSTOMER N 7/836	en Pibre	9546	CUM. QTY  JNIT PRICE  TAX	ORDERED CTY
SPECIAL DELIVERY INTO RI	PRODUCT CODE DE	SCRIPTION  10528 COM. FRE	RIGHT CHA	CUSTOMER N 7/836	en Albre  IUMBER PRO  S9 6	SALES TOTA	CUM. QTY  JNIT PRICE  TAX	AMOUNT



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# APPENDIX C. MASH TEST 3-11 (CRASH TEST NO. 612831-01-1)

### C1 VEHICLE PROPERTIES AND INFORMATION

Table C.1. Vehicle Properties for Test No. 612831-01-1.

Date:	2020-0	1-07	_ Test No	<sub>).:</sub> 612	831-01-1	_ VIN No.	: <u>10</u>	6RR6FT8	ES35	5468
Year:	201	4	_ Make	e:	RAM	_ Model	:	150	0	
Tire Siz	ze: <u>265</u> /	70 R 17			Tire	Inflation Pr	essure: _	3	35 ps	si
Tread	Type: <u>Hig</u> h	nway				Odd	ometer: _	152623		
Note a	ny damage t	o the ve	hicle prior t	to test: N	lone					
• Dend	otes acceler	ometer I	ocation.			X -   X -	_			
NOTES	S: None			_ 1 1		7/				1
Engine Engine	.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	7-8 7 liter		— A M	WHEEL					- N T
Transm	nission Type Auto or FWD	·	] Manual 4W	D .	R	\ <u>\</u>		→ TEST INERTIAL	С. М.	
Optiona None	al Equipmer e	nt:		_	P	•	<b>I</b>	•		B
Dummy Type: Mass: Seat F	<u> </u>	lo dumm	ny used 0 lb		F-	и н	L <sub>V</sub>	-s	D-	FK L
Geome	etry: inche	es			_	7 M FRONT	C	▼ M   REAR		
Α	78.50	F	40.00	) к	20.00	Р	3.0	00	U	26.75
В	74.00	G .	28.50	<u> </u>	30.00	_ Q	30.5	50	<sub>v</sub> –	30.25
c	227.50	н	62.21	<u>М</u>	68.50	R	18.0	00	w =	62.20
D	44.00	1	11.75	<u> </u>	68.00	_ s	13.0	00	× _	79.00
E	140.50	J	27.00		46.00	_ T _	77.0			
Н	neel Center eight Front		14.75	Wheel \ Clearance (Fr	ont)	6.00		Frame - Front		12.50
Н	neel Center leight Rear			Wheel \ Clearance (R	ear)	9.25	Heigh	rrame t - Rear		22.50
	.IMII: A=78 ±2 inch Ratings:	ies; C=237 ±	13 inches; E=148  Mass: I		±3 inches;		inches; 0=43 ± <u>Inertial</u>		,	1.5 inches <u>Static</u>
Front	3700		M <sub>front</sub>	D 7	2897	<u>1031</u>	2786		<u> </u>	2786
Back	3900		M <sub>rear</sub>		2070		2214	_		2214
Total	6700		M <sub>Total</sub>		4967		5000	_		5000
Mass D	 Distribution	:			(Allowable	Range for TIM an	d GSM = 5000 I	b ±110 lb)		
lb		LF:	1372	RF:	1414	LR:	1148	_ RR:	1	066

Table C.2. Measurements of Vehicle Vertical CG for Test No. 612831-01-1.

Date:2020-	01-07 <b>T</b>	est No.: _	612831-	01-1	VIN:	1C6RR6FT8ES355468		
Year:20^	14	Make: _	RAM	1	Model:	1500		
Body Style: G	Quad Cab				Mileage:	152623		
Engine: 5.7 liter	r \	<b>V-</b> 8		Trans	smission:	Automatic		
Fuel Level: E	mpty	Ball	<b>ast</b> :160_				(440	) lb max)
Tire Pressure:	Front: 3	35 <b>ps</b>	i Rea	r: <u>35</u>	psi S	<b>Size</b> : 265/70 F	₹ 17	
Measured Vel	hicle Wei	ghts: (II	0)					
LF:	1372		RF:	1414		Front Axle	e: 2786	
LR:	1148		RR:	1066		Rear Axl	e: 2214	
Left:	2520		Right:	2480		Tota	al: 5000	
						5000	±110 lb allowed	
VVh	l neel Base:	140.50	inches	Track: F:	68.50	inches F	R: 68.00	inches
	148 ±12 inch	es allowed			Track = (F+F	R)/2 = 67 ±1.5 incl	nes allowed	
Center of Gra	vity, SAE	J874 Sus <sub>l</sub>	pension M	ethod				
X:	62.21	inches	Rear of F	ront Axle	(63 ±4 inches	allowed)		
Y:	-0.27	inches	Left -	Right +	of Vehicle	Centerline		
Z:	28.50	inches	Above Gr	ound	(minumum 2	3.0 inches allowed	d)	
Hood Heig	jht:	46.00	inches	Front	Bumper H	eight:	27.00 i	nches
	43 ±4 i	nches allowed						
Front Overha	ng:	40.00	inches	Rear	Bumper H	eight:	30.00 i	nches
39 ±3 inches allo								
Overall Leng	ıth:	227.50	inches					
	237 ±1	3 inches allow	ed					

#### Table C.3. Exterior Crush Measurements for Test No. 612831-01-1.

Date:	2020-01-07	_ Test No.:	612831-01-1	VIN No.:	1C6RR6F18ES355468
Year:	2014	_ Make:	RAM	Model:	1500

#### VEHICLE CRUSH MEASUREMENT SHEET<sup>1</sup>

Complete When Applicable							
End Damage	Side Damage						
Undeformed end width	Bowing: B1 X1						
Corner shift: A1	B2 X2						
A2							
End shift at frame (CDC)	Bowing constant						
(check one)	X1+X2 _						
< 4 inches							
≥ 4 inches							

Note: Measure C<sub>1</sub> to C<sub>6</sub> from Driver to Passenger Side in Front or Rear Impacts – Rear to Front in Side Impacts.

a :a			Direct Damage								
Specific Impact Number	Plane* of C-Measurements	Width** (CDC)	Max*** Crush	Field L**	$C_1$	$C_2$	C <sub>3</sub>	C <sub>4</sub>	C <sub>5</sub>	C <sub>6</sub>	±D
1	Front plane at bmpr ht	14	11	28	-	1	-	-	-	-	-20
2	Side plane at bmpr ht	14	14	50	-	ı	-	-	1	1	+72
	Measurements recorded										
	✓ inches or ☐ mm										

<sup>&</sup>lt;sup>1</sup>Table taken from National Accident Sampling System (NASS).

Free space value is defined as the distance between the baseline and the original body contour taken at the individual C locations. This may include the following: bumper lead, bumper taper, side protrusion, side taper, etc. Record the value for each C-measurement and maximum crush.

Note: Use as many lines/columns as necessary to describe each damage profile.

<sup>\*</sup>Identify the plane at which the C-measurements are taken (e.g., at bumper, above bumper, at sill, above sill, at beltline, etc.) or label adjustments (e.g., free space).

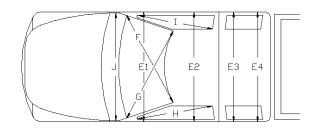
<sup>\*\*</sup>Measure and document on the vehicle diagram the beginning or end of the direct damage width and field L (e.g., side damage with respect to undamaged axle).

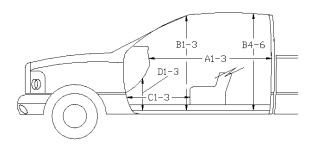
<sup>\*\*\*</sup>Measure and document on the vehicle diagram the location of the maximum crush.

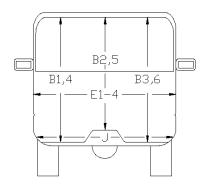
Table C.4. Occupant Compartment Measurements for Test No. 612831-01-1.

 Date:
 2020-01-07
 Test No.:
 612831-01-1
 VIN No.:
 1C6RR6FT8ES355468

 Year:
 2014
 Make:
 RAM
 Model:
 1500







<sup>\*</sup>Lateral area across the cab from driver's side kickpanel to passenger's side kickpanel.

# OCCUPANT COMPARTMENT DEFORMATION MEASUREMENT

f <b>ter Differ.</b> hes)
65.00 0.00
63.00 0.00
65.50 0.00
45.00 0.00
38.00 0.00
45.00 0.00
39.50 0.00
43.00 0.00
39.50 0.00
23.00 -3.00
0.00 0.00
26.00 0.00
11.00 0.00
0.00 0.00
11.50 0.00
58.50 0.00
63.50 0.00
63.50 0.00
63.50 0.00
59.00 0.00
59.00 0.00
37.50 0.00
37.50 0.00
25.00 0.00

## C2 SEQUENTIAL PHOTOGRAPHS

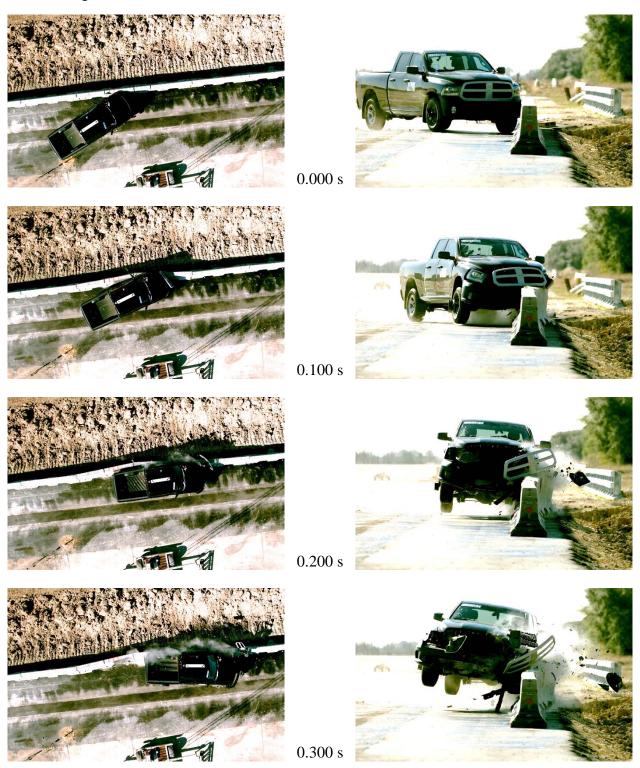


Figure C.1. Sequential Photographs for Test No. 612831-01-1 (Overhead and Frontal Views).

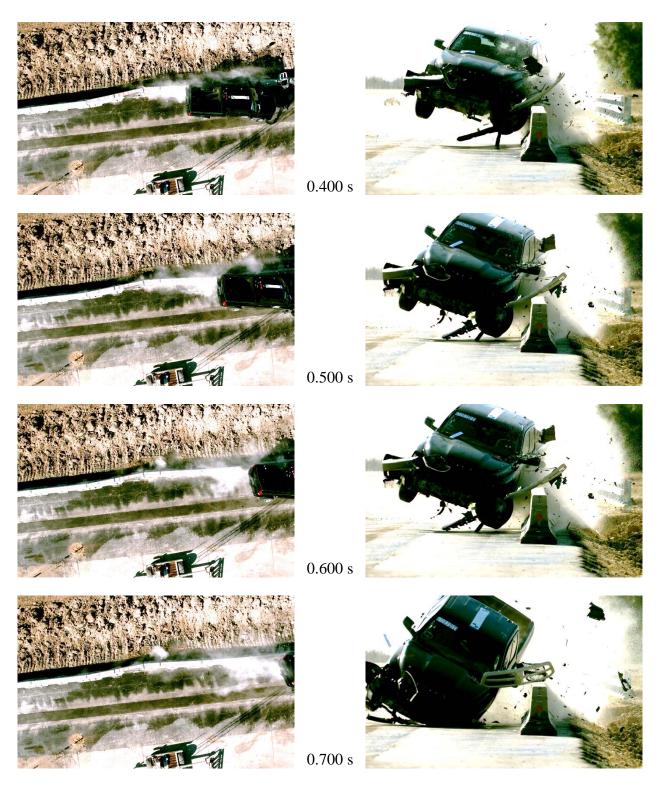


Figure C.1. Sequential Photographs for Test No. 612831-01-1 (Overhead and Frontal Views) (Continued).

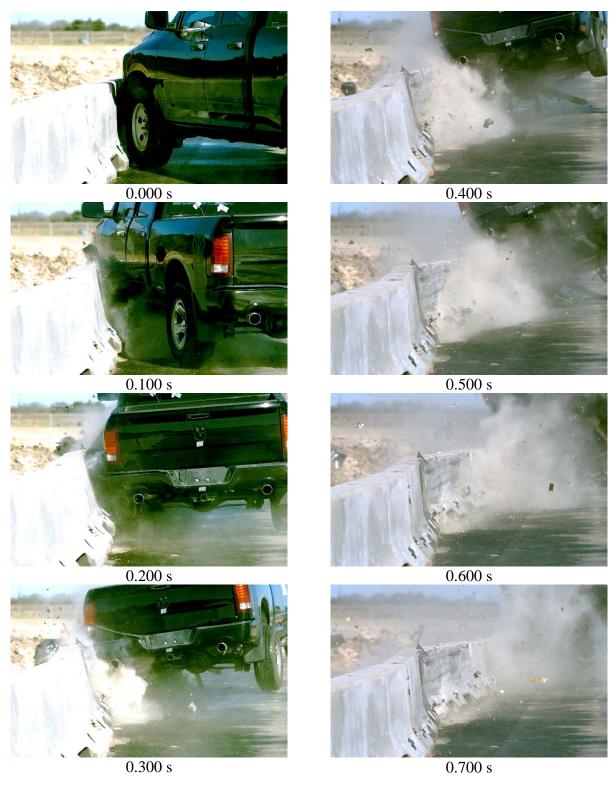


Figure C.2. Sequential Photographs for Test No. 612831-01-1 (Rear View).

Figure C.3. Vehicle Angular Displacements for Test No. 612831-01-1.

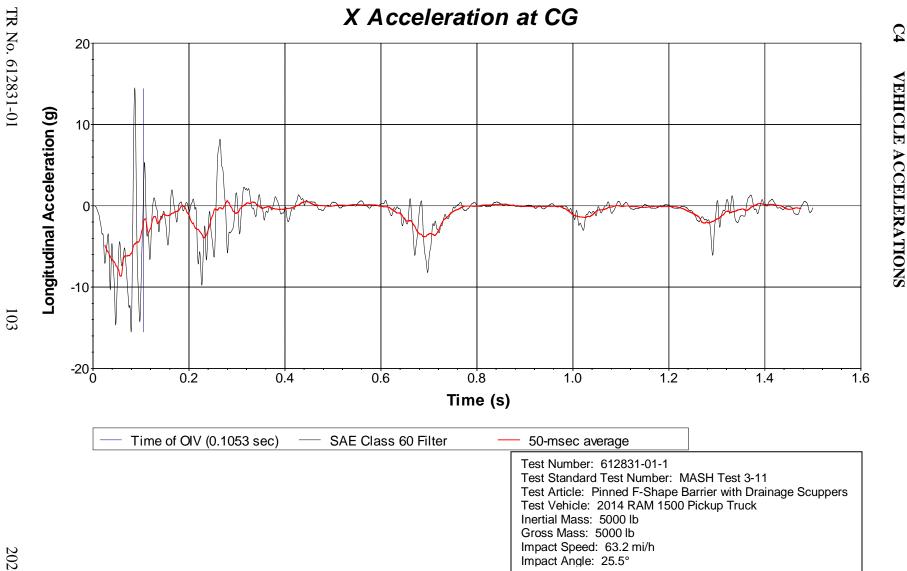


Figure C.4. Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-1 (Accelerometer Located at Center of Gravity).

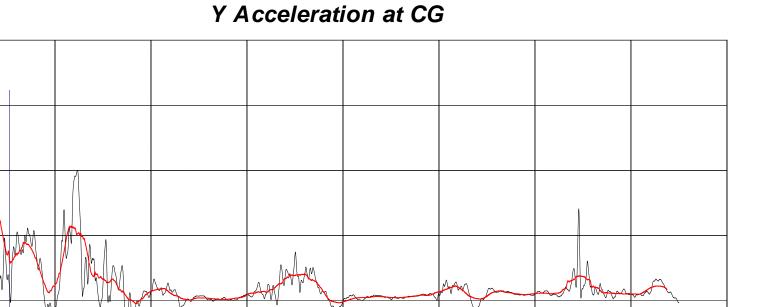
20

15

0.2

0.4

Lateral Acceleration (g)



Time of OIV (0.1053 sec) — SAE Class 60 Filter — 50-msec average

0.6

Test Standard Test Number: MASH Test 3-11

1.2

1.4

1.6

Test Article: Pinned F-Shape Barrier with Drainage Scuppers

Test Vehicle: 2014 RAM 1500 Pickup Truck

Inertial Mass: 5000 lb Gross Mass: 5000 lb Impact Speed: 63.2 mi/h Impact Angle: 25.5°

Test Number: 612831-01-1

1.0

Impact Angle: 25.5° ace for Test No. 612831-01

Figure C.5. Vehicle Lateral Accelerometer Trace for Test No. 612831-01-1 (Accelerometer Located at Center of Gravity).

0.8

Time (s)

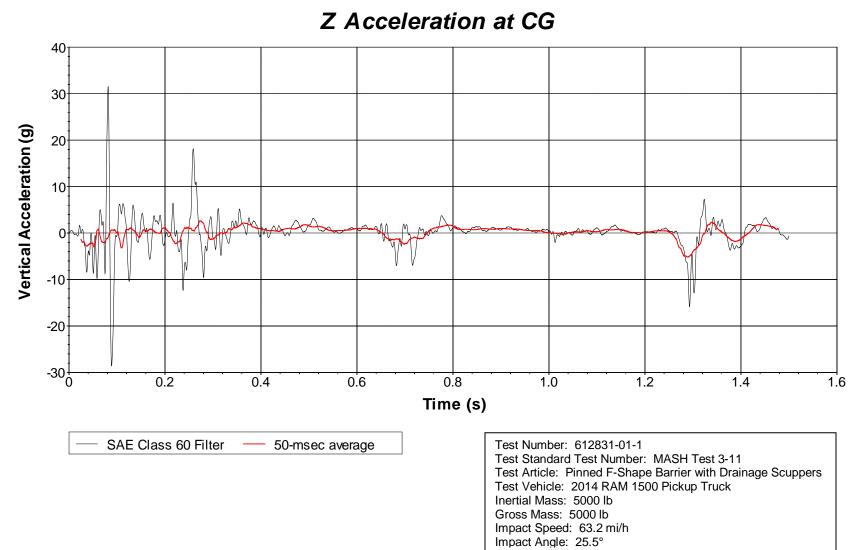


Figure C.6. Vehicle Vertical Accelerometer Trace for Test No. 612831-01-1 (Accelerometer Located at Center of Gravity).

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# APPENDIX D. MASH TEST 3-11 (CRASH TEST NO. 612831-01-2)

### D1 VEHICLE PROPERTIES AND INFORMATION

Table D.1. Vehicle Properties for Test No. 612831-01-2.

Date:20	020-01-09	_ Test No.:	612831-	01-2	_ VIN No.:	1C6RR6	GT2ES	477824
Year:	2014	Make:	RAM		Model		1500	
Tire Size:	265/70 R 17	7		Tire I	nflation Pre	essure:	35	osi
Tread Type:	Highway				Odo	meter: <u>14719</u>	98	
Note any dam	nage to the ve	ehicle prior to	test: None					
Denotes ac	:celerometer	location			X-	_		
NOTES: No		100411011.	A	*			Dı ——	1
NOTES. 140	i ic		-   [					
Engine Type:	V-8		A M -		<del>                                     </del>			H N T
Engine CID:	5.7 liter		_				<u> </u>	WHEEL TRACK
Transmission	· · · · -	7 Magual	-			TEST II	NERTIAL C. M.	
Auto FWD	or <u>L</u> _ <b>∏</b> RWD	<b>」</b> Manual 4WD		R - Q				
Optional Equi	pment:		₽ →	•				⇒
None	· 		- 🖟 🛌		W	<b>1</b>	(E	<i>b</i> − <b>B</b>
Dummy Data:		-	] J I					TK L
Type: Mass:	No dumr	0 lb	_	<b>←</b> F <b>→</b>	—H—►	L <sub>G</sub> L <sub>V</sub> L <sub>S</sub>	<b>■</b> D =	
Seat Position	n: NA		- -	-	M	Е ———	▼ M	
Geometry:	inches				FRONT	— C ———	REAR	•
A78.		40.00	_ K	20.00	_ P _	3.00	U .	26.75
B74.0		28.75	_ L	30.00	_ Q _	30.50	٧.	30.25
C 227.		60.43	_ M	68.50	_ R_	18.00	W <sub>-</sub>	60.40
D 44.0		11.75	_ N	68.00	_ S _	13.00	X <sub>-</sub>	79.00
E 140.5 Wheel Cen		27.00	_ O Wheel Well	46.00	_ T_	77.00 Bottom Fram	e -	
Height Fro Wheel Cen	ont	14.75 cı	earance (Front)		6.00	Height - Fror	nt	12.50
Height Re	ear		Wheel Well learance (Rear) _		9.25	Bottom Fram Height - Rea	ır	22.50
						nches; O=43 ±4 inches		
GVWR Rating	<b>gs:</b> 700	Mass: Ib	Curb	926	<u>i est</u>	<u>Inertial</u> 2880	Gros	ss Static 2880
	900	M <sub>front</sub> M <sub>rear</sub>		110		2174		2174
	700	M <sub>Total</sub>		036		5054		5054
Mass Distrib					Range for TIM and	I GSM = 5000 lb ±110 li	o)	
lp	LF	1430	_ RF:1	450	LR:	1110	RR:	1064

Table D.2. Measurements of Vehicle Vertical CG for Test No. 612831-01-2.

Date:2020	<u>-01-09                                  </u>	est No.: _	612831-	01-2	VIN:	1C6RR6	GT2ES47782	24
Year: 20	)14	Make: _	RAM	1	Model:		1500	
Body Style:	Quad Cab				Mileage:	147198		
Engine: 5.7 lite	er '	V-8		Trans	smission:	Automatic		
Fuel Level:	Empty	Bal	last: _100				(440	0 lb max)
Tire Pressure	: Front: 3	35 ps	i Rea	ır: <u>35</u>	psi S	Size: 265/70	R 17	
Measured Ve	hicle Wei	ghts: (l	b)					
LF	: 1430		RF:	1450		Front Axl	e: 2880	
LR	: 1110		RR:	1064		Rear Axl	e: 2174	
Left	: 2540		Right:	2514		Tota	al: 5054 0 ±110 lb allowed	
W	heel Base:	140.50	inches	Track: F:	68.50	inches I	R: 68.00	inches
	148 ±12 inch	es allowed			Track = (F+F	$R)/2 = 67 \pm 1.5 inc$	hes allowed	
Center of Gra	avity, SAE	J874 Sus	pension M	ethod				
Х	: 60.44	inches	Rear of F	ront Axle	(63 ±4 inches	s allowed)		
Υ	: -0.18	inches	Left -	Right +	of Vehicle	e Centerline		
Z	28.75	inches	Above Gr	ound	(minumum 2	8.0 inches allowe	d)	
Hood Hei	ght:	46.00	inches	Front	Bumper H	eight:	27.00	inches
	43 ±4 i	nches allowed						
Front Overha			_	Rear	Bumper H	eight:	30.00	inches
Overallian		nches allowed						
Overall Len		227.00	-					

#### Table D.3. Exterior Crush Measurements for Test No. 612831-01-2.

612831-01-2

VIN No.:

1C6RR6GT2ES477824

Year:	2014 Make:	RA	M	Model: _		1500
	VEHICLE (	CRUSH ME	ASUREM	ENT SHEET	$\Gamma^1$	
		Complete Who	en Applicab	le		
	End Damage			Side	Damage	
	Undeformed end width _			Bowing: B1 _	X1	
	Corner shift: A1 _			В2 _	X2 _	
	A2 _					
	End shift at frame (CDC)		Boy	ving constant		
	(check one)			X1+X2	_	
	< 4 inches _			=		
	≥ 4 inches _					

Note: Measure C<sub>1</sub> to C<sub>6</sub> from Driver to Passenger Side in Front or Rear Impacts – Rear to Front in Side Impacts.

G:G-		Direct Damage									
Specific Impact Number	Plane* of C-Measurements	Width** (CDC)	Max*** Crush	Field L**	$C_1$	$C_2$	C <sub>3</sub>	C <sub>4</sub>	C <sub>5</sub>	C <sub>6</sub>	±D
1	Front plane at bmpr ht	15	14	36	-	-	-	-	-	-	-18
2	Side plane at bmpr ht	15	15	58	-	-	-	-	-	-	+68
	Measurements recorded										
	√inches or □mm										
				·							

<sup>&</sup>lt;sup>1</sup>Table taken from National Accident Sampling System (NASS).

2020-01-09

Test No.:

Date:

Free space value is defined as the distance between the baseline and the original body contour taken at the individual C locations. This may include the following: bumper lead, bumper taper, side protrusion, side taper, etc. Record the value for each C-measurement and maximum crush.

Note: Use as many lines/columns as necessary to describe each damage profile.

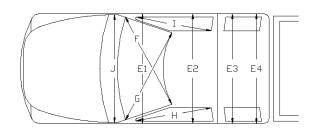
<sup>\*</sup>Identify the plane at which the C-measurements are taken (e.g., at bumper, above bumper, at sill, above sill, at beltline, etc.) or label adjustments (e.g., free space).

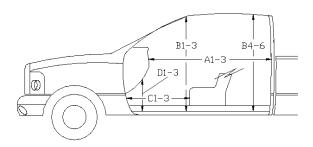
<sup>\*\*</sup>Measure and document on the vehicle diagram the beginning or end of the direct damage width and field L (e.g., side damage with respect to undamaged axle).

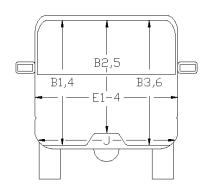
<sup>\*\*\*</sup>Measure and document on the vehicle diagram the location of the maximum crush.

Table D.4. Occupant Compartment Measurements for Test No. 612831-01-2.

Date:	2020-01-09	_ Test No.:	612831-01-2	VIN No.:	1C6RR6GT2ES477824
Year:	2014	Make:	RAM	Model:	1500







\*Lateral area across the cab from driver's side kickpanel to passenger's side kickpanel.

# OCCUPANT COMPARTMENT DEFORMATION MEASUREMENT

	Before	After (inches)	Differ.
A1	65.00	65.00	0.00
A2	63.00	63.00	0.00
А3	65.50	65.50	0.00
B1	45.00	45.00	0.00
B2	38.00	38.00	0.00
В3	45.00	45.00	0.00
B4	39.50	39.50	0.00
B5	43.00	43.00	0.00
В6	39.50	39.50	0.00
C1	26.00	24.50	-1.50
C2	0.00	0.00	0.00
C3	26.00	26.00	0.00
D1	11.00	11.00	0.00
D2	0.00	0.00	0.00
D3	11.50	11.50	0.00
E1	58.50	58.50	0.00
E2	63.50	63.50	0.00
E3	63.50	63.50	0.00
E4	63.50	63.50	0.00
F	59.00	59.00	0.00
G	59.00	59.00	0.00
Н	37.50	37.50	0.00
I	37.50	37.50	0.00
J*	25.00	25.00	0.00

# D2 SEQUENTIAL PHOTOGRAPHS

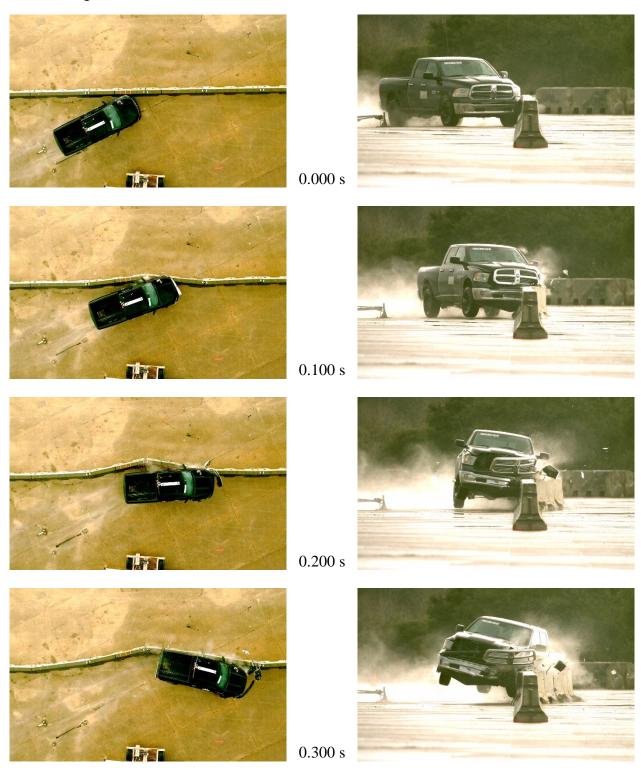


Figure D.1. Sequential Photographs for Test No. 612831-01-2 (Overhead and Frontal Views).

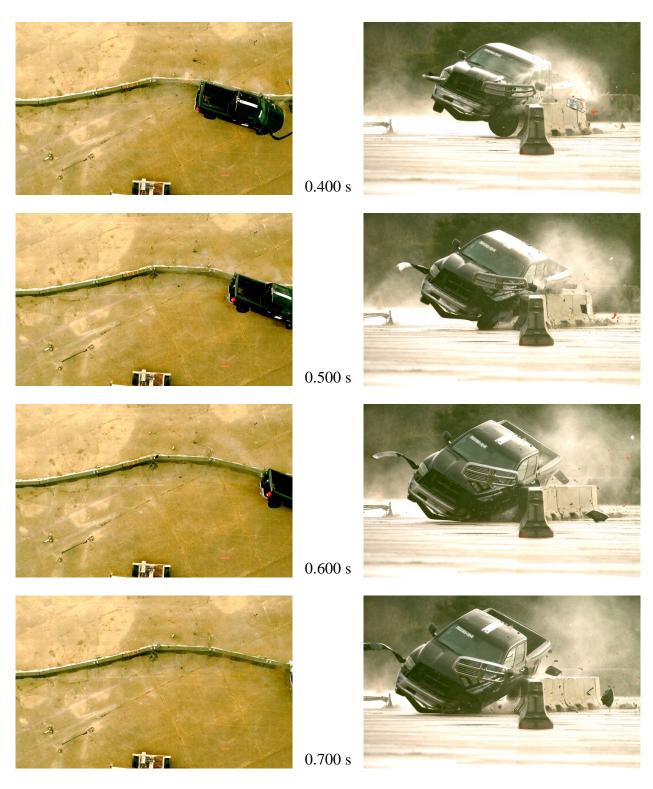


Figure D.1. Sequential Photographs for Test No. 612831-01-2 (Overhead and Frontal Views) (Continued).

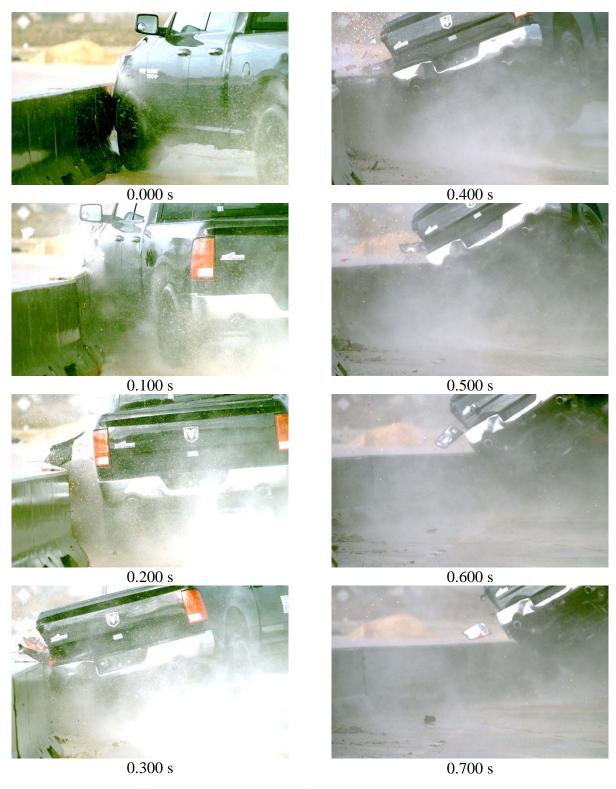


Figure D.2. Sequential Photographs for Test No. 612831-01-2 (Rear View).

Figure D.3. Vehicle Angular Displacements for Test No. 612831-01-2.

**D4** 

X Acceleration at CG

Figure D.4. Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-2 (Accelerometer Located at Center of Gravity).

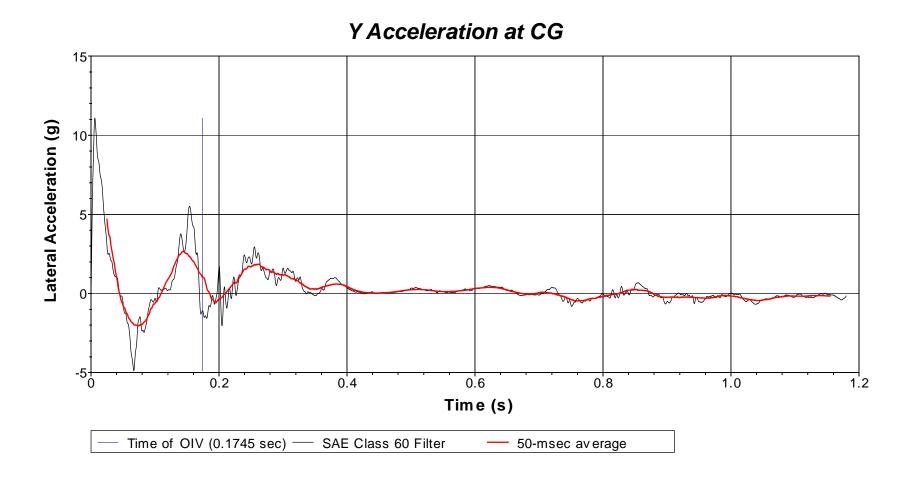


Figure D.5. Vehicle Lateral Accelerometer Trace for Test No. 612831-01-2 (Accelerometer Located at Center of Gravity).

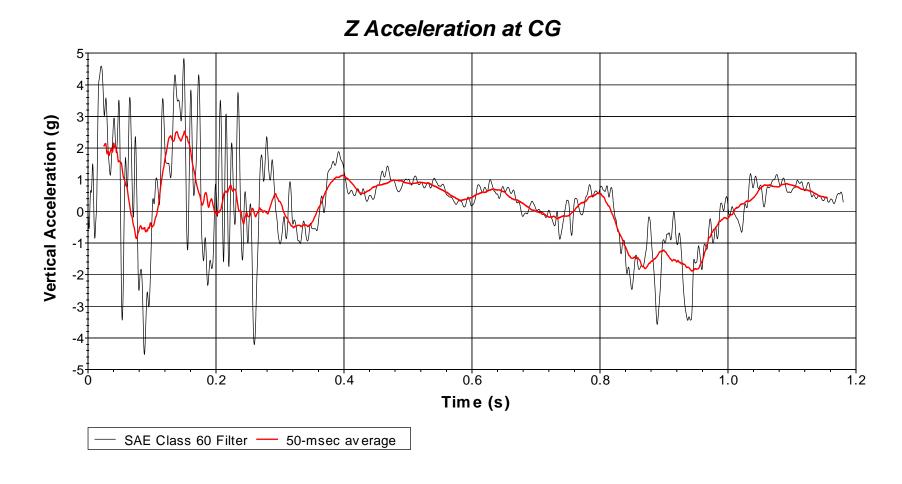


Figure D.6. Vehicle Vertical Accelerometer Trace for Test No. 612831-01-2 (Accelerometer Located at Center of Gravity).

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# APPENDIX E. MASH TEST 4-12 (CRASH TEST NO. 612831-01-3)

#### E1 VEHICLE PROPERTIES AND INFORMATION

Table E.1. Vehicle Properties for Test No. 612831-01-3.

Da	ate: 2019-12-	17 Test	No.:	612831-01-	3 VIN	l No.: _	3HAMMAAN8CL	641700		
Ye	ear:2012	M	ake:	INTERNATION	NAL M	odel: _	4300			
0	dometer:	Tire :	Size	Front: 275/80	R22.5 -	Tire Size	e Rear: 275/80	0R22.5		
X	N N N N N N N N N N N N N N N N N N N	В	P	Z P	W Y D	C	B K J			
Α	Front Bumper Width:	✓ inches o 92.50 145.50		mm Rear Bumper Bottom: Rear Frame	38.00	U V	Cab Length: Trailer/Box	106.00		
В	Overall Height: Overall Length:	321.50	M	Top: Front Track Width:	80.00	W	Length:  Gap Width:	1.50		
D	Rear Overhang:	74.50	Ν	Roof Width:	71.00	Х	Overall Front Height:	98.50		
Е	Wheel Base:	207.00	0	Hood Height:	58.50	Υ	Roof-Hood Distance:	30.00		
F	Front Overhang:	40.00	P Q	Bumper Extension: Front Tire		Z AA	Roof-Box Height Difference: Rear Track	47.00		
G H	C.G. Height: C.G. Horizontal Dist. w/Ballast:	126.77	R	Width: Front Wheel Width:	39.00 23.50	вв	Width: Ballast Center of Mass:	73.00 62.00		
I	Front Bumper Bottom:	18.25	S	Bottom Door Height:	37.00	CC	Cargo Bed Height:	50.00		
J	Front Bumper Top:	33.25	Т	Overall Width:	96.00					
	Allowable Range: C = 394 inches max.; E = 240 inches max.; CC = 49 ±2 inches; BB = 63 ±2 inches above ground;									
١	Wheel Center Height Front	19.00		Wheel Well Clearance (Front)	9	.00	Bottom Frame Height (Front)	25.50		
	Wheel Center  Height Rear  e information needed o	19.00	<b>→</b>	Wheel Well Clearance (Rear)	3	.50	Bottom Frame Height (Rear)	27.50		

## Table E.1. Vehicle Properties for Test No. 612831-01-3 (Continued).

Date:	2019-12-17	Test No.:	612831-01-3	_ VIN No.: _	3HAMMAAN8CL641700	
Year:	2012	Make:	INTERNATIONAL	_ Model: _	4300	
	<b>WEIGHT</b> : (☑ lb or		CURB	TEST	INERTIAL	
	V	Vfront axle	7290		8670	
	٧	V <sub>rear axle</sub>	6650		13700	
		W <sub>TOTAL</sub>	13940			
	Allowable	Range for CURB =	13,200 ±2200 lb   Allowable R	ange for TIM = 22,	046 ±660 lb	
Balla	st: <u>8430</u>	(	(as-nee √Ib or kg) (See M		.1.2 for recommen	ded ballasting)
Mass Distri (☑lb or ☐	bution kg ): LF	: 4380	<b>RF</b> : 4290	LR: 6880	RR:	6820
Engine Type Engine Size	100		Accelero	meter Locatio <b>x</b> 1	ns ( 🔽 inches o	or mm)
			– Front:			
Transmissio  Auto		Manual	Center:	126.70	0.00	49.50
FWD	RWD	4WD	Rear:	218.70	0.00	49.50
Describe an	y damage to t	he vehicle prio	r to test: none			
Other notes attachment		allast type, di	mensions, mass, loc	ation, center	of mass, and i	nethod of
Two block	ks: Height 30 i	nches x Width	60 inches x Length 30	)''		
Centered	in middle of b	ed				
62 inches	from ground	to center of blo	ock			
Each bloc	k tied down w	rith four 5/16-in	ich cables			
Performed	by: SCD			Dat	e:2019-	12-17

<sup>&</sup>lt;sup>1</sup> Referenced to the front axle <sup>2</sup> Above ground

## **E2 SEQUENTIAL PHOTOGRAPHS**

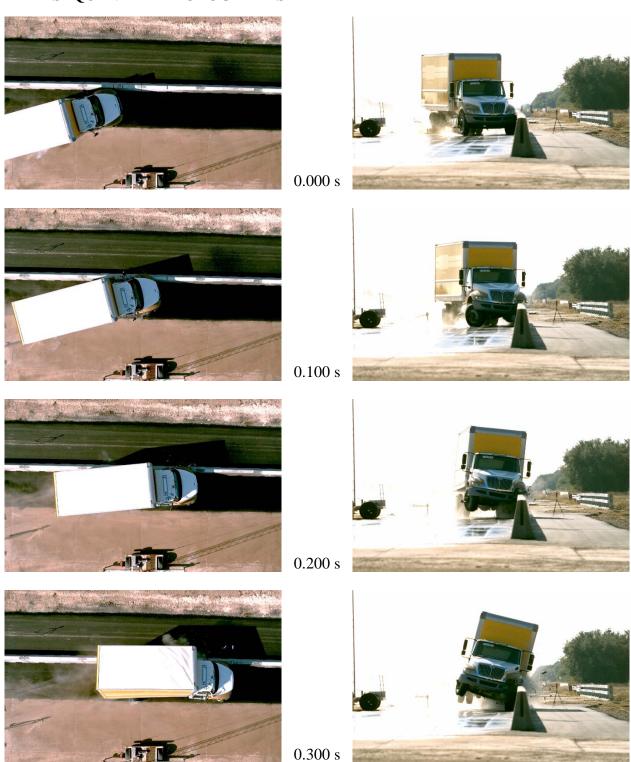


Figure E.1. Sequential Photographs for Test No. 612831-01-3 (Overhead and Frontal Views).

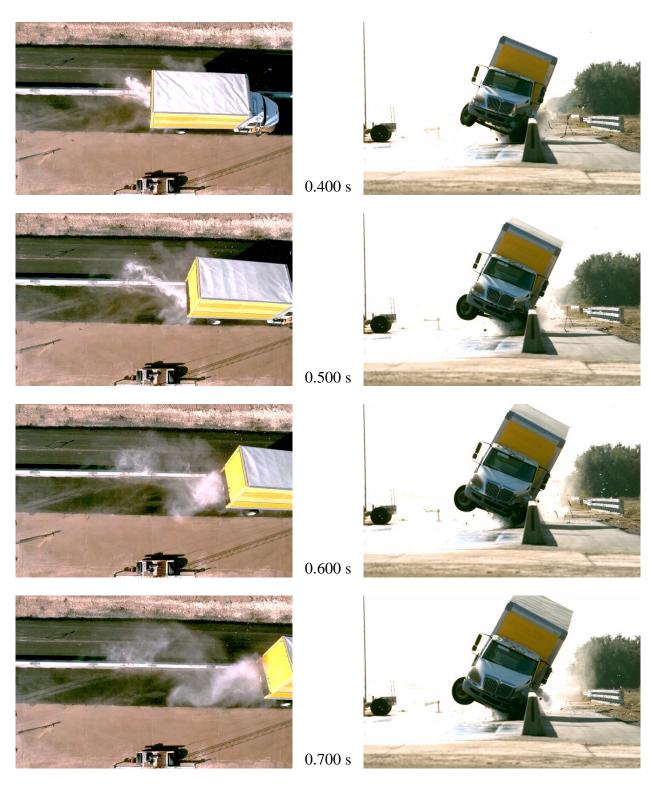


Figure E.1. Sequential Photographs for Test No. 612831-01-3 (Overhead and Frontal Views) (Continued).



Figure E.2. Sequential Photographs for Test No. 612831-01-3 (Rear View).

Figure E.3. Vehicle Angular Displacements for Test No. 612831-01-3.

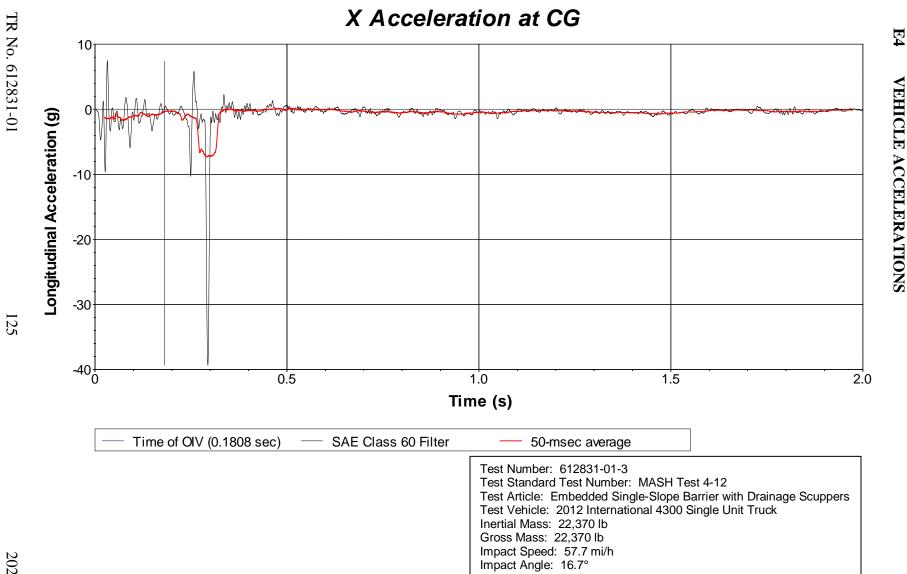
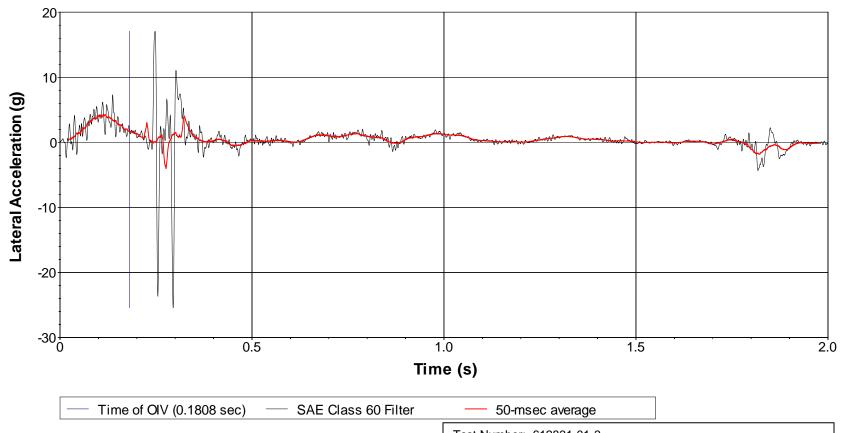


Figure E.4. Vehicle Longitudinal Accelerometer Trace for Test No. 612831-01-3 (Accelerometer Located at Center of Gravity).

# Y Acceleration at CG



Test Number: 612831-01-3

Test Standard Test Number: MASH Test 4-12

Test Article: Embedded Single-Slope Barrier with Drainage Scuppers

Test Vehicle: 2012 International 4300 Single Unit Truck

Inertial Mass: 22,370 lb Gross Mass: 22,370 lb Impact Speed: 57.7 mi/h

Impact Angle: 16.7°

Figure E.5. Vehicle Lateral Accelerometer Trace for Test No. 612831-01-3 (Accelerometer Located at Center of Gravity).

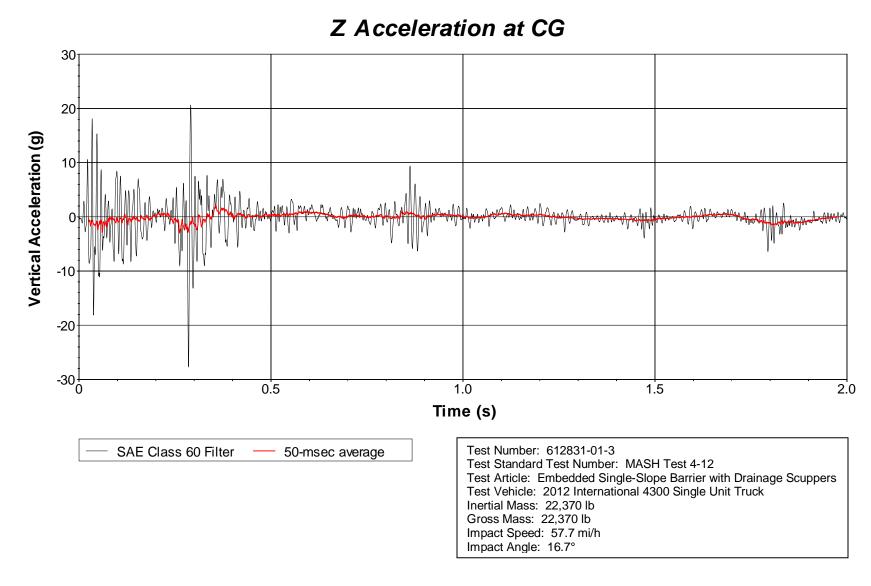


Figure E.6. Vehicle Vertical Accelerometer Trace for Test No. 612831-01-3 (Accelerometer Located at Center of Gravity).