

1200 New Jersey Ave., SE Washington, D.C. 20590

In Reply Refer To: HSST-1/B-335

Mr. Omar Fernandez
MTA Bridges and Tunnels – Triborough Bridge & Tunnel Authority
Bronx-Whitestone Facility Building
1 Hutchinson River Parkway
Bronx, NY 10465
USA

Dear Mr. Fernandez:

This letter is in response to your December 12, 2019 request for the Federal Highway Administration (FHWA) to review a roadside safety device, hardware, or system for eligibility for reimbursement under the Federal-aid highway program. This FHWA letter of eligibility is assigned FHWA control number B-335 and is valid until a subsequent letter is issued by FHWA that expressly references this device.

# **Decision**

The following device is eligible within the length-of-need, with details provided in the form which is attached as an integral part of this letter:

• Bronx Whitestone Median Barrier Extension Bridge Rail

# **Scope of this Letter**

To be found eligible for Federal-aid funding, new roadside safety devices should meet the crash test and evaluation criteria contained in the American Association of State Highway and Transportation Officials' (AASHTO) Manual for Assessing Safety Hardware (MASH). However, the FHWA, the Department of Transportation, and the United States Government do not regulate the manufacture of roadside safety devices. Eligibility for reimbursement under the Federal-aid highway program does not establish approval, certification or endorsement of the device for any particular purpose or use.

This letter is not a determination by the FHWA, the Department of Transportation, or the United States Government that a vehicle crash involving the device will result in any particular outcome, nor is it a guarantee of the in-service performance of this device. Proper manufacturing, installation, and maintenance are required in order for this device to function as tested.

This finding of eligibility is limited to the crashworthiness of the system and does not cover other structural features, nor conformity with the Manual on Uniform Traffic Control Devices.

# **Eligibility for Reimbursement**

Based solely on a review of crash test results and certifications submitted by the manufacturer, and the crash test laboratory, FHWA agrees that the device described herein meets the crash test and evaluation criteria of the AASHTO's MASH. Therefore, the device is eligible for reimbursement under the Federal-aid highway program if installed under the range of tested conditions.

Name of system: Bronx Whitestone Median Barrier Extension Bridge Rail

Type of system: Longitudinal Barrier Test Level: MASH Test Level 4 (TL4)

Testing conducted by: Texas A&M Transportation Institute

Date of request: December 12, 2019

FHWA concurs with the recommendation of the accredited crash testing laboratory on the attached form.

In accordance with FHWA's Memo "Federal-aid Reimbursement Eligibility Process for Safety Hardware Devices" dated November 12, 2015, FHWA will make note of any reported damage to a test vehicle's fuel tank. AASHTO's MASH states "Although not a specific factor in assessing test results, integrity of a test vehicle's fuel tank is a potential concern. It is preferable that the fuel tank remains intact and not be punctured. Damage or rupture of the fuel tank, oil pan, or other feature that might serve as a surrogate of the fuel tank should be reported". A test report included in this submittal documenting Test 4-12 (Report for test 611351-1) states that the fuel tank on the right side of the test vehicle was damaged.

# **Full Description of the Eligible Device**

The device and supporting documentation, including reports of the crash tests or other testing done, videos of any crash testing, and/or drawings of the device, are described in the attached form.

## **Notice**

This eligibility letter is issued for the subject device as tested. Modifications made to the device are not covered by this letter. Any modifications to this device should be submitted to the user (i.e., state DOT) as per their requirements.

You are expected to supply potential users with sufficient information on design, installation and maintenance requirements to ensure proper performance.

You are expected to certify to potential users that the hardware furnished has the same chemistry, mechanical properties, and geometry as that submitted for review, and that it will meet the test and evaluation criteria of AASHTO's MASH.

Issuance of this letter does not convey property rights of any sort or any exclusive privilege. This letter is based on the premise that information and reports submitted by you are accurate and correct. We reserve the right to modify or revoke this letter if: (1) there are any inaccuracies in the information submitted in support of your request for this letter, (2) the qualification testing was flawed. (3) in-service performance or other information reveals safety problems, (4) the system is significantly different from the version that was crash tested, or (5) any other information indicates that the letter was issued in error or otherwise does not reflect full and complete information about the crashworthiness of the system.

# **Standard Provisions**

- To prevent misunderstanding by others, this letter of eligibility designated as FHWA control number B-335 shall not be reproduced except in full. This letter and the test documentation upon which it is based are public information. All such letters and documentation may be reviewed upon request.
- This letter shall not be construed as authorization or consent by the FHWA to use, manufacture, or sell any patented system for which the applicant is not the patent holder.
- This FHWA eligibility letter is not an expression of any Agency view, position, or determination of validity, scope, or ownership of any intellectual property rights to a specific device or design. Further, this letter does not impute any distribution or licensing rights to the requester. This FHWA eligibility letter determination is made based solely on the crash-testing information submitted by the requester. The FHWA reserves the right to review and revoke an earlier eligibility determination after receipt of subsequent information related to crash testing.

Sincerely,

Michael S. Griffith

Director, Office of Safety Technologies

Michael S. Fiffith

Office of Safety

**Enclosures** 

# Request for Federal Aid Reimbursement Eligibility of Highway Safety Hardware

	Date of Request:	12/12/2019	<ul><li>New</li></ul>	○ Resubmission
		Gavin Daly, P.E.		
itter	Company:	HNTBCorporation		
bmit	Address:	Empire State Building, 350 Fifth Avenue, 57th Floor, New York, NY 10118		
Suk	Country:	USA		
	To:	Michael S. Griffith, Director FHWA, Office of Safety Technologies		

I request the following devices be considered eligible for reimbursement under the Federal-aid highway program.

Device & Testing Criterion - Enter from right to left starting with Test Level					!-!-!	
System Type	SubmissionType	Device Name / Vai	riant	TestingCriterion	Test Level	
'B':Rigid/Semi-Rigid Barriers (Roadside, Median, Bridge Railings)	Finding Analysis	Bronx Whitestone Median Barrier Exter Bridge Rail		AASHTOMASH	TL4	

By submitting this request for review and evaluation by the Federal Highway Administration, I certify that the product(s) was (were) tested in conformity with the AASHTO Manual for Assessing Safety Hardware and that the evaluation results meet the appropriate evaluation criteria in the MASH.

# **Individual or Organization responsible for the product:**

Contact Name: Omar Fernandez Sameas Submitter						
CompanyName: MTABridges&Tunnels-TriboroughBridge&TunnelAuthority SameasSubmitter						
Address: Bronx-Whitestone Facility Building, 1 Hutchinson River Parkway Same as Submitt						
Country:	Country: Bronx,NY10465,USA SameasSubmitter					
	closures of financial interests as required by the FHWA `Federal or Safety Hardware Devices' document.	-Aid Reimbursement				
HNTB: HNTBCorporation is a paid consultant for MTA-TBTA for this Bronx Whitestone Median Barrier Extension Bridge Rail project and eligibility request. HNTB has no further financial interest in the use of this barrier system.						
TTI:Texas A&M Transportation Institute (TTI) was contracted by HNTB to perform full-scale crash testing of the Bronx Whitestone Median Barrier Extension Bridge Rail. There are no shared financial interests in the Bronx Whitestone Median Barrier Extension Bridge Rail design by TTI, or between HNTB and TTI, other than costs involved in the actual crash testsand reports for this submission to FHWA.						

# PRODUCT DESCRIPTION

Help		
New Hardware or Significant Modification	Modification to Existing Hardware	
	and was comprised of 14 barrier segments that were ea ed of a lower steel median barrier and an upper barrier gh bolted splice connections.	
inchesapart. Steel tubes were at	ras comprised of posts fabricated from steel plates that tached to each side of these postsand an outer steel on each side of the lower median barrier.	
median barrier posts. Acrylic pan	comprised of vertical posts that were attached on to lels were installed between adjacent posts of the upper les wasattached to each side of the upper barrier externation.	barrier extension. A
one end of the installation. No up a top rail comprised of two steel t	er barrier extension, with posts on each side, was built in oper barrier extension posts or acrylic panels were presonables spanned the gap and connected the adjacent upostubes of the typical rail extend across the gap.	ent in this gap. Instead,
•	n barrier was 32% inchesabove grade, the centerline of 4 inchesabove grade, and the top of the railing posts	
	CRASH TESTING	
all of the critical and relevant cra	affiliated with the testing laboratory, agrees in support sh tests for this device listed above were conducted nined that no other crash testsare necessary to deter	to meet the MASH test
Engineer Name:	Nauman M.Sheikh, P.E.	
EngineerSignature:		ned by Nauman Sheikh 11.25 17:16:58 -06'00'
Address:	TTI,TAMUS3135,CollegeStation,TX77843-3135	SameasSubmitter
Country:	USA	SameasSubmitter

A brief description of each crash test and its result: Help

	+	Page 3 01 7
RequiredTest	Narrative	Evaluation
Number	Description	Results
	Test 4-10 involves an 1100C vehicle	
	impacting the test article at a target impact	
	speed of 62 mi/h and target angle of 25°.	
	The target CIP for the right corner of the	
	front bumper was 3.6 ft upstream of the	
	joint between segments 6 & 7.	
	The results of the test conducted on April	
	30, 2019, are found in TTI Test Report No.	
	611351-01.The 2008 Kia Rio test vehicle was	
	traveling at an impact speed of 62.7 mi/h as	
	it made contact with the Bronx Whitestone	
	Median Barrier Extension Bridge Rail 3.7 ft	
	upstream of the joint between segments 6	
	& 7and at an impact angle of 25.1°. After	
	loss of contact with the barrier, the vehicle	
	came to rest 235 ft downstream of the	
	impact point and 64 ft toward the traffic	
	lanes.	
	The Bronx Whitestone Median Barrier	
	Extension Bridge Rail contained and	
	redirected the 1100C vehicle. The vehicle	
	did not penetrate, underride, or override	
	the installation. No dynamic deflection or permanent deformation was observed. No	
	<b>!</b>	
4 40 (4400C)	detached elements, fragments, or other	DACC
4-10 (1100C)	debris were present to penetrate or show potential for penetrating the occupant	PASS
	compartment, or to present hazard to	
	others in the area. The 1100C vehicle	
	remained upright during and after the	
	collision event. Maximum roll and pitch	
	angles were 27° and 8°, respectively.	
	angles were 27 and 6, respectively.	
	Langitudinal OlV (was 40.4 ft/s and lateral	
	Longitudinal OIV was 18.4 ft/s and lateral	
	OIV was 33.8 ft/s. Maximum longitudinal	
	occupant ridedown acceleration was 6.0 g,	
	and maximum lateral occupant ridedown	
	acceleration was 10.2 g. Occupant risk	
	factors were within the maximum limits	
	specified in MASH.	
	Maximum exterior crush to the vehicle was	
	8.0 inches in the side plane at the right front	
	corner at bumper height. Maximum	
	occupant compartment deformation was	
	1.5 inches in the right front floor pan.	
	The Drawy Whitestone Marking Drawing	
	The Bronx Whitestone Median Barrier	
	ExtensionBridge Rail performed acceptably	
	forMASH test 4-10.	

		Page 4 of 7
RequiredTest	Narrative	Evaluation
Number	Description	Results
	Test 4-11 involves a 2270P vehicle	
	impacting the test article at a target impact	
	speed of 62 mi/h and target angle of 25°.	
	The target CIP for the right corner of the	
	front bumper was 4.3 ft upstream of the	
	joint between segments 8 & 9.	
	John between segments o & 3.	
	The results of the test conducted on May 2,	
	2019, are found in TTITest Report No.	
	611351-01.The 2014 RAM 1500 test vehicle	
	was traveling at an impact speed of 62.9 mi/	
	h as it made contact with the Bronx	
	Whitestone Median Barrier Extension Bridge	
	Rail 4.1 ft upstream of the joint between	
	segments 8 & 9 and at an impact angle of	
	25.9°. After loss of contact with the barrier,	
	the vehicle came to rest 215 ft downstream	
	of the impact point and 2 ft toward the	
	traffic lanes.	
	u amo la los.	
	The Bronx Whitestone Median Barrier	
	Extension Bridge Rail contained and	
	redirected the 2270P vehicle. The vehicle	
	did not penetrate, underride, or override	
	the installation. Maximum dynamic	
	deflection was 0.7 inches, and no	
	permanent deformation was observed. No	
4-11 (2270P)	detached elements, fragments, or other	PASS
, , (==, ,,	debris were present to penetrate or show	
	potential for penetrating the occupant	
	compartment, or to present hazard to	
	others in the area. The 2270P vehicle	
	remained upright during and after the	
	collision event. Maximum roll and pitch	
	angles were 19° and 5°, respectively.	
	angles were to and s prospessively.	
	Longitudinal OIV was 20.3 ft/s and lateral	
	OIV was 29.5 ft/s. Maximum longitudinal	
	occupant ridedown acceleration was 5.3 g,	
	and maximum lateral occupant ridedown	
	acceleration was 9.7 g. Occupant risk factors	
	were within the preferred limitsspecified in	
	MASH.	
	Maximum exterior crush to the vehicle was	
	11.0 inches in the side plane at the right	
	front corner at bumper height. Maximum	
	occupant compartment deformation was	
	4.5 inches in the right front firewall area.	
	34	
	The Bronx Whitestone Median Barrier	
ı	Extension Bridge Rail performed acceptably	
	forMASH test 4-11.	

Test 4-12 involves a 10000S vehicle impacting the test article at a target impact speed of 56 mi/h and target angle of 15°. Two Test 4-12's were performed on the barrier. Test No. 611351-1 was performed in the main length-of-need of the barrier. Test 611351-2 was performed to evaluate the open section of the upper barrier extension.

The Bronx Whitestone Median Barrier ExtensionBridge Rail performed acceptably for both MASH test 4-12's, assummarized below:

|--

### TEST611351-1:

The target CIP for the right corner of the front bumper was 5.0 ft upstream of the joint between segments 2 & 3.

The results of the test conducted on May 6, 2019, are found in TTITest Report No. 611351-01. The 2012 International 4300 SUT test vehicle was traveling at an impact speed of 57.2 mi/h as it made contact with the Bronx Whitestone Median Barrier Extension Bridge Rail 5.8 ft upstream of the joint between segments 2 & 3, and at an impact angle of 14.9°. After loss of contact with the barrier, the vehicle came to rest 352 ft downstream of the impact point and 30 ft toward the field side.

4-12 (10000S)

The Bronx Whitestone Median Barrier Extension Bridge Rail contained and redirected the 10000Svehicle. The vehicle did not penetrate, underride, or override the installation. Any dynamic deflection was obscured in the camera views, but no permanent deformation was observed. No detached elements, fragments, or other debris were present to penetrate or show potential for penetrating the occupant compartment, or to present hazard to others in the area. The 10000S vehicle remained upright during and after the collision event. Maximum roll and pitch angles were 25° and 4°, respectively.

Longitudinal OIV was 7.5 ft/s, and lateral OIV was 16.1 ft/s.

Maximum longitudinal occupant ridedown acceleration was 2.0 g, and maximum lateral occupant ridedown acceleration was 4.5 g.

Maximum exterior crush to the vehicle was 10.0 inchesat the right front corner at bumper height. Maximum occupant compartment deformation was 3.0 inches in the right floor pan area.

See Additional Test 4-12 info next page----

PASS

		1 age 0 or 1
	ADDITIONALTEST611351-2:	
	The target CIP for the left corner of the front	
	bumper was 16.5 ft upstream of the	
	upstream edge of top of post #4.	
	upstream edge of top of post#4.	
	The results of the test conducted on May 8,	
	2019, are found in TTITestReport No.	
	-	
	611351-01.The 2013 International 4300 SUT	
	test vehicle was traveling at an impact	
	speed of 57.1 mi/h as it made contact with	
	the Bronx Whitestone Median Barrier	
	ExtensionBridgeRail 16.5ft upstream of the	
	upstream edge of top of post #4, and at an	
	impact angle of 15.2°. After loss of contact	
	with the barrier, the vehicle came to rest	
	315 ft downstream of the impact point and	
	18 ft toward the field side.	
	The Breny Whitestone Medien Berrier	
	The Bronx Whitestone Median Barrier	
	Extension Bridge Rail contained and	
	redirected the 10000S vehicle. The vehicle	
	did not penetrate, underride, or override	
	· ·	
	the installation. Maximum dynamic	
4-12 (10000S)	deflection was 1.5 inches, and no	PASS
	permanent deformation was observed. No	
	detached elements, fragments, or other	
	debris were present to penetrate or show	
	· · ·	
	potential for penetrating the occupant	
	compartment, or to present hazard to	
	others in the area. The 10000S vehicle	
	remained upright during and after the	
	collision event. Maximum roll and pitch	
	angles were 23° and 7°, respectively.	
	Longitudinal OIV was 6.2 ft/s, and lateral OIV	
	was 14.1 ft/s.	
	Maximum longitudinal occupant ridedown	
	acceleration was 3.5 g, and maximum lateral	
	occupant ridedown acceleration was 16.7 g.	
	occupant nuedown acceleration was 10.7 g.	
	Maximum exterior crush to the vehicle was	
	14.0 inchesat the left front corner at	
	bumper height. Maximum occupant	
	compartment deformation was 5.0 inches in	
	the left firewall area.	
4-20 & 4-21	4-20 & 4-21: Device is not a transition	Non-Relevant Test, not conducted
		·
4-22 (10000S)	4-22: Device is not a transition	Non-Relevant Test, not conducted

Full Scale Crash Testing was done in compliance with MASH by the following accredited crash test laboratory (cite the laboratory's accreditation status as noted in the crash test reports.):

Laboratory Name:	Texas A&M Transportation Institute	
LaboratorySignature:	Digitally signed by Darrell L.Kuhn 'Date: 2019.11.2513:51:30-06'00	LKulm
Address:	TTI,TAMUSMS3135,CollegeStation,TX77843-3135	SameasSubmitter
Country:	USA	SameasSubmitter
Accreditation Certificate	ISO 17025-2017 Laboratory	•
Number and Dates of current	A2LACertificate Number: 2821.01	
Accreditation period :	Valid To: April 30, 2021	

SubmitterSignature*: Gavin Daly	Digitally signed by Gavin Daly Date: 2019.11.26 17:49:10
---------------------------------	--

Submit Form
-------------

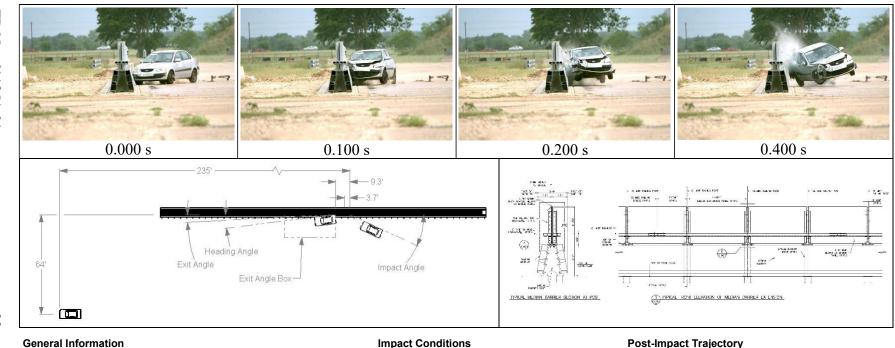
# **ATTACHMENTS**

### Attach to this form:

- 1) Additional disclosures of related financial interest as indicated above.
- 2) A copy of the full test report, video, and a Test Data Summary Sheet for each test conducted in support of this request.
- 3) A drawing or drawings of the device(s) that conform to the Task Force-13 Drawing Specifications [Hardware Guide Drawing Standards]. For proprietary products, a single isometric line drawing is usually acceptable to illustrate the product, with detailed specifications, intended use, and contact information provided on the reverse. Additional drawings (not in TF-13 format) showing details that are relevant to understanding the dimensions and performance of the device should also be submitted to facilitate our review.

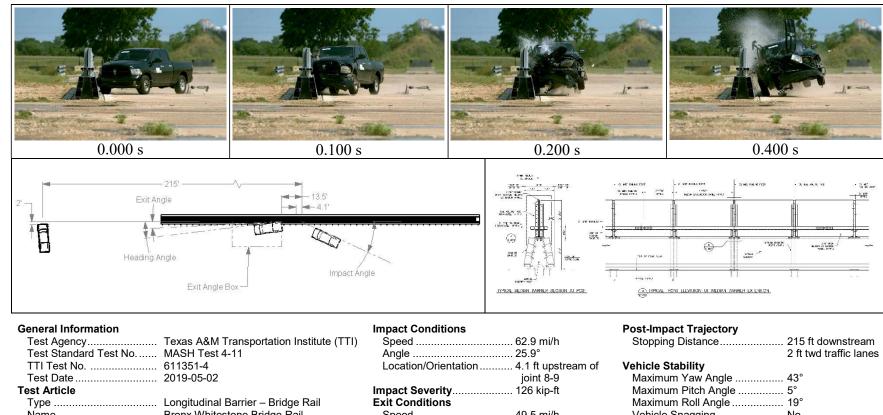
# FHWA Official Business Only:

Eligi	bility Letter	
Number	Date	Key Words



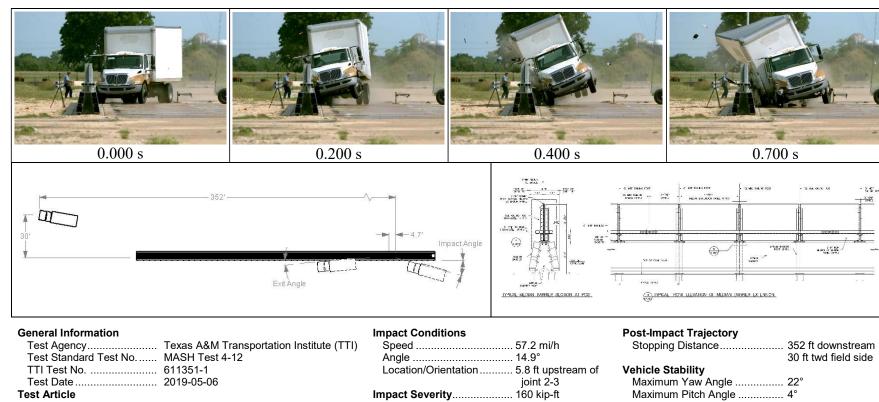
General Information		Impact Conditions		Post-Impact Trajectory	
Test Agency	Texas A&M Transportation Institute (TTI)	Speed	. 62.7 mi/h	Stopping Distance	235 ft downstream
Test Standard Test No	MASH Test 4-10	Angle	25.1°		64 ft twd traffic lanes
TTI Test No	611351-3	Location/Orientation	. 3.7 ft upstream of	Vehicle Stability	
Test Date	2019-04-30		joint 6-7	Maximum Yaw Angle	97°
Test Article		Impact Severity	. 58 kip-ft	Maximum Pitch Angle	
Type	Longitudinal Barrier – Bridge Rail	Exit Conditions		Maximum Roll Angle	27°
	Bronx Whitestone Bridge Rail	Speed	53.9 mi/h	Vehicle Snagging	No
Installation Length	210 ft	Trajectory/Heading Angle	. 3.9° / 8.1°	Vehicle Pocketing	No
Material or Key Elements	Steel plate posts, steel parapet covering	Occupant Risk Values		Test Article Deflections	
-	steel beams secured to posts, fabricated	Longitudinal OIV	. 18.4 ft/s	Dynamic	None
	steel rail above the parapet, acrylic	Lateral OIV	33.8 ft/s	Permanent	None
	windscreen above parapet and rail	Longitudinal Ridedown	. 6.0 g	Working Width	30.0 inches
	supported by posts	Lateral Ridedown	. 10.2 g	Height of Working Width	6.0 inches
Soil Type and Condition	Reinforced concrete foundation, damp	THIV	. 41.7 km/h		
Test Vehicle		PHD	. 10.4 g	Vehicle Damage	
Type/Designation	1100C	ASI	. 2.61	VDS	01RFQ5
Make and Model	2008 Kia Rio	Max. 0.050-s Average		CDC	01FREW4
Curb	2430 lb	Longitudinal	<b>-</b> 10.3 g	Max. Exterior Deformation	8.0 inches
Test Inertial	2455 lb	Lateral	<b>-</b> 20.2 g	OCDI	RF0102100
Dummy	. 165 lb	Vertical	<b>-</b> 4.2 g	Max. Occupant Compartment	
Gross Static				Deformation	1.5 inches

Figure 5.6. Summary of Results for MASH Test 4-10 on Bronx Whitestone Bridge Rail.



General Information		impact Conditions		Post-Impact Trajectory	
Test Agency	Texas A&M Transportation Institute (TTI)	Speed	62.9 mi/h	Stopping Distance	215 ft downst
Test Standard Test No	MASH Test 4-11	Angle		•	2 ft twd traffic
TTI Test No	611351-4	Location/Orientation	4.1 ft upstream of	Vehicle Stability	
Test Date	2019-05-02		joint 8-9	Maximum Yaw Angle	43°
Test Article		Impact Severity	126 kip-ft	Maximum Pitch Angle	5°
Type	Longitudinal Barrier – Bridge Rail	Exit Conditions		Maximum Roll Angle	19°
Name	Bronx Whitestone Bridge Rail	Speed	49.5 mi/h	Vehicle Snagging	No
Installation Length	210 ft	Trajectory/Heading Angle	3.9° / 2.1°	Vehicle Pocketing	No
Material or Key Elements	Steel plate posts, steel parapet covering	Occupant Risk Values		Test Article Deflections	
	steel beams secured to posts, fabricated	Longitudinal OIV	20.3 ft/s	Dynamic	0.7 inch
	steel rail above the parapet, acrylic	Lateral OIV	29.5 ft/s	Permanent	None
	windscreen above parapet and rail	Longitudinal Ridedown	5.3 g	Working Width	30.7 inches
	supported by posts	Lateral Ridedown	9.7 g	Height of Working Width	6.0 inches
Soil Type and Condition	Reinforced concrete foundation, damp	THIV	39.2 km/h		
Test Vehicle		PHD			
Type/Designation	2270P	ASI	2.17	Vehicle Damage	
Make and Model	2014 RAM 1500 Pickup	Max. 0.050-s Average		VDS	
Curb		Longitudinal	<b>-</b> 9.5 g	CDC	
Test Inertial		Lateral		Max. Exterior Deformation	11.0 inches
Dummy	165 lb	Vertical	<b>-</b> 4.2 g	OCDI	RF0020000
Gross Static	5175 lb			Max. Occupant Compartme	nt

Figure 6.6. Summary of Results for MASH Test 4-11 on Bronx Whitestone Bridge Rail.



Test Agency	Texas A&M Transportation Institute (TTI)	Speed	. 57.2 mi/h
Test Standard Test No	MASH Test 4-12	Angle	
TTI Test No	611351-1	Location/Orientation	. 5.8 ft upstre
Test Date	2019-05-06		joint 2-3
Test Article		Impact Severity	. 160 kip-ft
Type	Longitudinal Barrier – Bridge Rail	Exit Conditions	•
Name	Bronx Whitestone Bridge Rail	Speed	. 50.4 mi/h
Installation Length	210 ft	Trajectory/Heading Angle	. 0.6° / 0°
Material or Key Elements	Steel plate posts, steel parapet covering	Occupant Risk Values	
	steel beams secured to posts, fabricated	Longitudinal OIV	. 7.5 ft/s
	steel rail above the parapet, acrylic	Lateral OIV	. 16.1 ft/s
	windscreen above parapet and rail	Longitudinal Ridedown	. 2.0 g
	supported by posts	Lateral Ridedown	. 4.5 g
Soil Type and Condition	Reinforced concrete foundation, damp	THIV	. 19.9 km/h
Test Vehicle		PHD	. 4.6 g
Type/Designation	10000S	ASI	. 0.84
Make and Model	2012 International 4300 Single-Unit Truck	Max. 0.050-s Average	
Curb	14,070 lb	Longitudinal	. <b>-</b> 1.9 g
Test Inertial	22,110 lb	Lateral	. <b>-</b> 7.0 g
Dummy	No dummy	Vertical	3.3 g
Gross Static			_

Post-impact Trajectory	
Stopping Distance	352 ft downstream
	30 ft twd field side
Vehicle Stability	
Maximum Yaw Angle	22°
Maximum Pitch Angle	4°
Maximum Roll Angle	
Vehicle Snagging	
Vehicle Pocketing	No
Test Article Deflections	
Dynamic	Not Obtainable
Permanent	0 inches
Working Width	73.3 inches
Height of Working Width	123.8 inches
Vehicle Damage	
VDS	NA
CDC	01FREW3
Max. Exterior Deformation	10.0 inches
OCDI	NA
Max. Occupant Compartment	
Deformation	3.0 inches

Figure 7.6. Summary of Results for MASH Test 4-12 on Bronx Whitestone Bridge Rail.

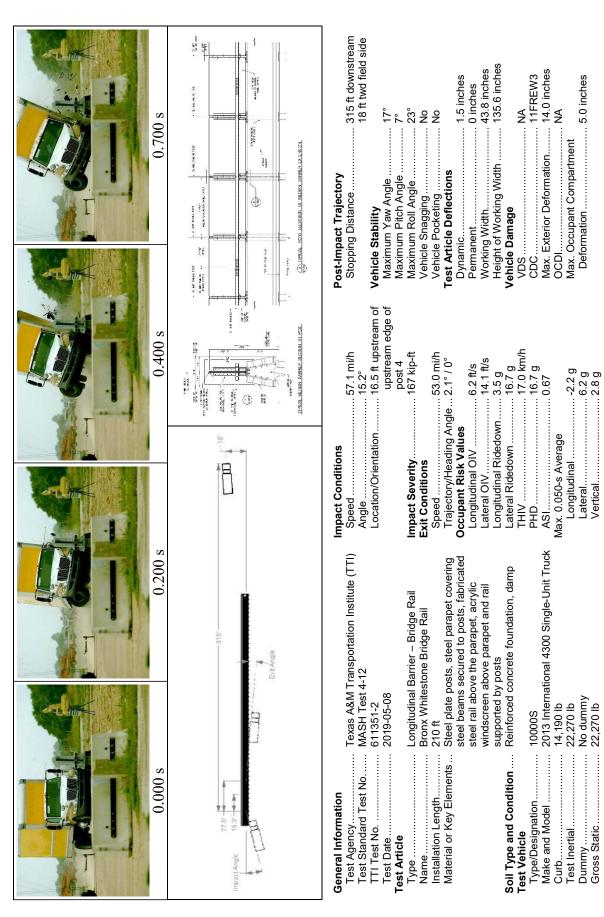
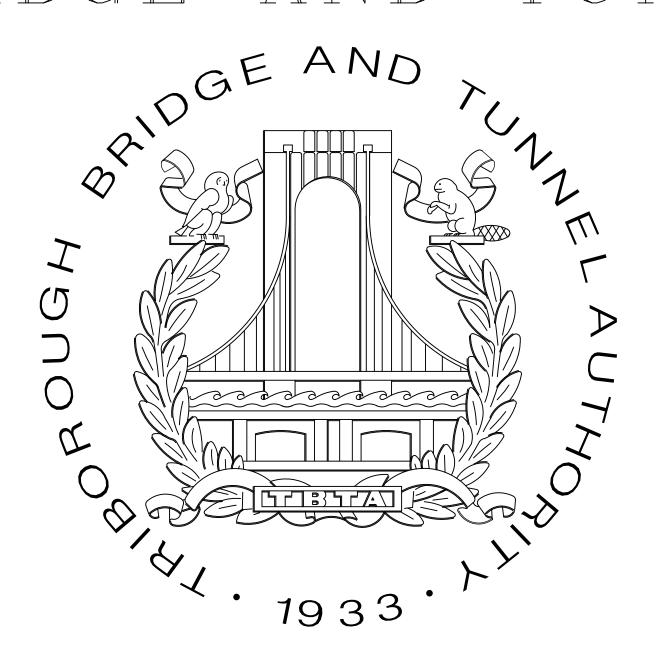


Figure 8.6. Summary of Results for MASH Test 4-12 on Bronx Whitestone Bridge Rail.

**Gross Static** 

# TRIBOROUGH BRIDGE AND TUNNEL AUTHORITY

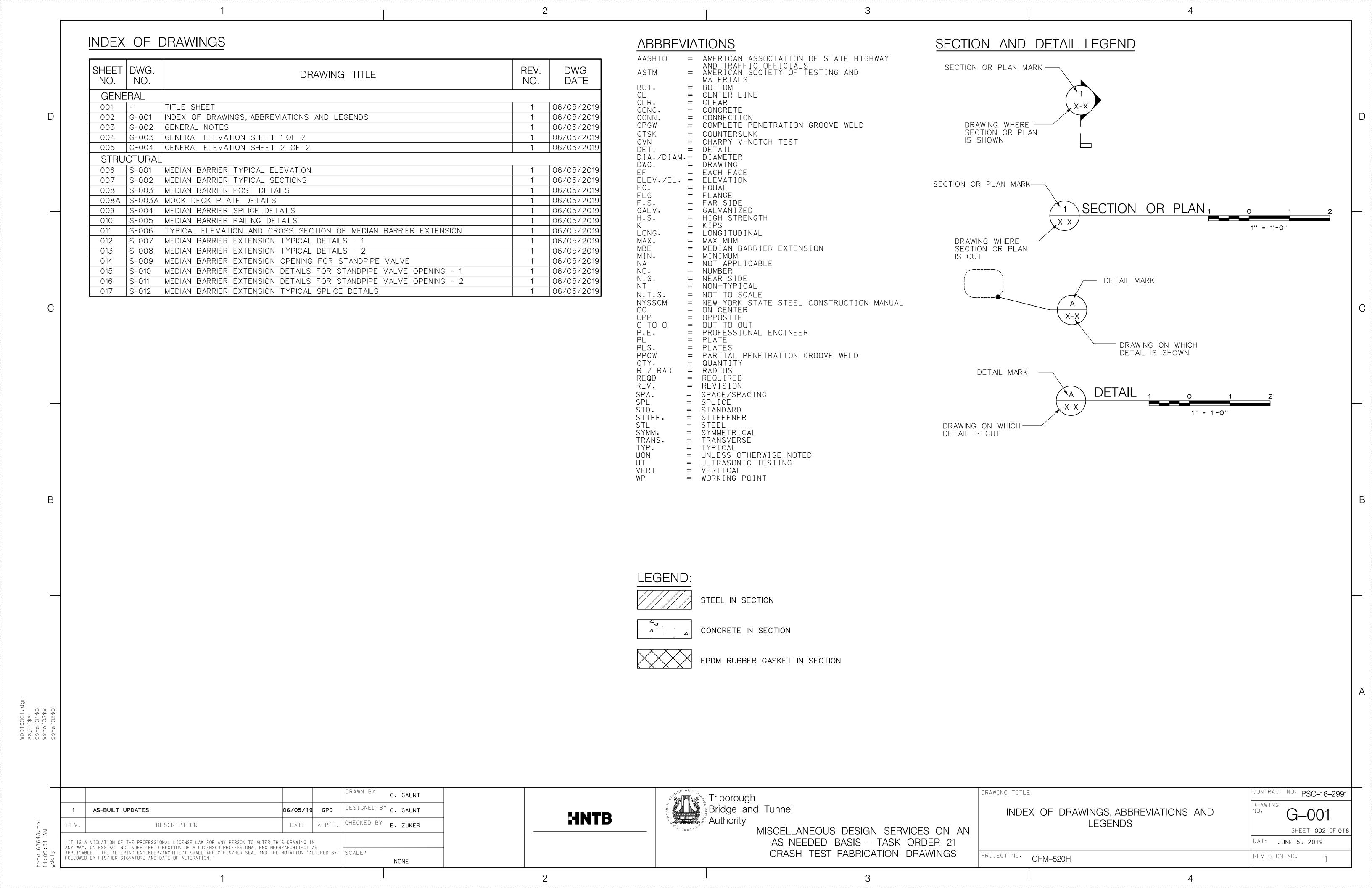


# CONTRACT NO.PSC-16-2991 MEDIAN BARRIER EXTENSION CRASH TEST AND FABRICATION FOR THE BRONX WHITESTONE BRIDGE AS-BUILT DRAWINGS

PREPARED BY:

HNTB CORPORATION
EMPIRE STATE BUILDING
350 5TH AVE, 57TH FL.
NEW YORK, NY 10118

DATE: 06/05/2019



SUGGESTED INSTALLATION SEQUENCE: REQUIREMENTS IN THIS CONTRACT 1. FABRICATION OF THE MEDIAN BARRIER AND MEDIAN BARRIER EXTENSION FOR THE LIMITS

DESIGN SPECIFICATIONS DESIGN, DETAILING, FABRICATION AND EXECUTION OF ALL CONSTRUCTION SHALL CONFORM

TO THE REQUIREMENTS OF THE CURRENT EDITIONS OF THE FOLLOWING PUBLICATIONS:

SHOWN ON THE CONTRACT DRAWINGS AND SHIPMENT TO THE TESTING FACILITY.

A. AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, CUSTOMARY U.S. UNITS

B. NEW YORK STATE DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS CONSTRUCTION AND MATERIALS AND ANY PROJECT SPECIAL SPECIFICATIONS

C. AASHTO LRFD BRIDGE CONSTRUCTION SPECIFICATIONS

D. NEW YORK STATE STEEL CONSTRUCTION MANUAL (NYSSCM)

E. AASHTO/AWS BRIDGE WELDING CODE D1.5

STRUCTURAL STEEL

GENERAL NOTES:

SCOPE OF WORK

D

С

1. ALL STRUCTURAL STEEL FOR THE MEDIAN BARRIER SHALL CONFORM TO THE FOLLOWING:

ASTM A709, GR 50 HSS RAILING AND SPLICE TUBES ASTM A500, GR B ASTM F3125, GR A325, TYPE 1 H.S. BOLTS NUTS ASTM A563 ASTM F436 WASHERS SPLICE BOLTS ASTM A449 FLAT HEAD COUNTER SUNK SCREWS ASTM F835

2. ALL STRUCTURAL STEEL FOR THE MEDIAN BARRIER EXTENSION SHALL CONFORM TO THE FOLLOWING:

PLATES AND ANGLES ASTM A709, GR 50 HSS POSTS, RAILING AND SPLICE TUBES ASTM A500, GR C ASTM F3125, GR A325, TYPE 1 H.S. BOLTS ASTM A563 NUTS WASHERS ASTM F436

3. ALL STRUCTURAL STEEL SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH SECTION 719 OF THE NYS STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED.

4. ALL FASTENERS, NUTS AND WASHERS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH SECTION 719 OF THE NYS STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED.

5. REPAIRS TO DAMAGED GALVANIZING SHALL BE MADE IN ACCORDANCE WITH ASTM A780.

ALL HIGH STRENGTH BOLTED CONNECTIONS SHALL BE INSTALLED IN STANDARD SIZE HOLES, EXCEPT WHERE SLOTTED HOLES OR OVERSIZED HOLES ARE SHOWN ON THE CONTRACT DRAWINGS. THREADS SHALL BE EXCLUDED FROM THE SHEAR PLANE. HIGH STRENGTH BOLTS SHALL BE TIGHTENED USING THE TURN-OF-NUT METHOD IN ACCORDANCE WITH THE NYSSCM, UNLESS OTHERWISE NOTED. HARDENED WASHERS SHALL BE PLACED UNDER BOTH THE HEAD AND THE NUT.

7. HD BLIND BOLTS WHERE INDICATED TO BE COATED WITH DACRALON ZINC FINISH.

WELDING SHALL BE PERFORMED IN ACCORDANCE WITH AASHTO/AWS D1.5 AND THE NYSSCM, THE STRICTER PROVISIONS SHALL APPLY. ALL WELDING SHALL BE PERFORMED WITH MATCHING FILLER METAL BY AN AASHTO/AWS OR NYSDOT QUALIFIED WELDER.

NON-DESTRUCTIVE TESTING SHALL BE PERFORMED IN ACCORDANCE WITH AASHTO/AWS D1.5, NYSSCM AND THE SPECIFICATIONS.

10. TRANSPARENT COLORLESS ACRYLIC PANELS SHALL BE "ACRYLITE SOUNDSTOP GS CC", 0.78" (20 MM) THICK EMBEDDED WITH COLORLESS POLYAMIDE FILAMENTS.

11. RUBBER GASKETS SHALL BE 70 DUROMETER EPDM OR APPROVED EQUAL.

1. FABRICATE MEDIAN BARRIER FOR THE EXTENTS SHOWN ON THE CONTRACT DRAWINGS.

2. FABRICATE MEDIAN BARRIER EXTENSION FOR THE EXTENTS SHOWN ON THE CONTRACT

3. SHOP ASSEMBLE MEDIAN BARRIER AND MEDIAN BARRIER EXTENSION AND VERIFY GEOMETRY AND FIELD SPLICES.

4. DISASSEMBLE AND SHIP MEDIAN BARRIER AND MEDIAN BARRIER EXTENSION TO THE CRASH TESTING FACILITY.

REQUIREMENTS BY OTHERS:

5. INSTALL CONCRETE SLAB.

6. PLACE MEDIAN BARRIER POSTS AT SPACING INDICATED IN S-001.

7. POST-INSTALL  $\frac{1}{8}$ " DIA. ADHESIVE ANCHOR BOLTS INTO THE CONCRETE SLAB.

8. INSTALL MEDIAN BARRIER SHELL / TUBE ASSEMBLY ON EACH SIDE OF THE POSTS.

9. INSTALL BOTH SIDES OF THE MEDIAN BARRIER EXTENSION WITH CONNECTION ANGLES LOOSE ON TOP OF THE MEDIAN BARRIER POSTS.

10. INSTALL MEDIAN EXTENSION PANELS, TIGHTEN BLIND BOLTS ON CONNECTION ANGLES.

# SPARE PARTS FOR CRASH TESTING:

THE FOLLOWING SPARE PARTS FOR REPLACEMENT DURING CRASH TESTING SHALL BE FABRICATED AND SHIPPED ALONG WITH THE TEST BARRIER LENGTH:

1. SIX (6) MEDIAN BARRIER EXTENSION PANELS (LENGTH 4'-9 $\frac{1}{4}$ '') WITH GASKETS.

2. TWO (2) TYPICAL MEDIAN BARRIER EXTENSION STEEL FRAMES (LENGTH 14'-9<sup>1</sup>/<sub>4</sub>'')

3. TWO (2) TYPICAL MEDIAN BARRIER UNITS (LENGTH 14'-91/4'')

4. ANY ASSOCIATED CONNECTION HARDWARE

				DRAWN BY	C. GAUNT
1	AS-BUILT UPDATES	06/05/19	GPD	DESIGNED BY	C. GAUNT
REV.	DESCRIPTION	DATE	APP'D.	CHECKED BY	E. ZUKER
	"IT IS A VIOLATION OF THE PROFESSIONAL LICENSE LAW FOR ANY PERSON TO ALTER THIS DRAWING IN				
APPLICAE	ANY WAY, UNLESS ACTING UNDER THE DIRECTION OF A LICENSED PROFESSIONAL ENGINEER/ARCHITECT AS APPLICABLE. THE ALTERING ENGINEER/ARCHITECT SHALL AFFIX HIS/HER SEAL AND THE NOTATION 'ALTERED BY' FOLLOWED BY HIS/HER SIGNATURE AND DATE OF ALTERATION."			SCALE:	NONE

HNTB

Triborough Eridge and Tunnel **Authority** 

> MISCELLANEOUS DESIGN SERVICES ON AN AS-NEEDED BASIS - TASK ORDER 21 CRASH TEST FABRICATION DRAWINGS

GENERAL NOTES

DRAWING TITLE

PROJECT NO. GFM-520H

CONTRACT NO. PSC-16-2991 DRAWING G-002SHEET 003 OF 018 DATE JUNE 5, 2019 REVISION NO.

