



Test Report No. 610221-01-1
Test Report Date: October 2018

**MASH TEST 4-12 ON KEYED-IN SINGLE-SLOPE BARRIER WITH
40-FT SEGMENT LENGTH**

by

Nauman M. Sheikh, P.E.
Associate Research Engineer

Bill L. Griffith
Research Specialist

and

Darrell L. Kuhn, P.E.
Research Specialist



Contract No.: T4541
Test No.: 610221-01-1
Test Date: 2018-06-15

Sponsored by
Roadside Safety Research Program Pooled Fund
Study No. TPF-5(114)

TEXAS A&M TRANSPORTATION INSTITUTE PROVING GROUND

Mailing Address:
Roadside Safety & Physical Security
Texas A&M University System
3135 TAMU
College Station, TX 77843-3135

Located at:
Texas A&M University REllIS Campus
Building 7091
3100 State Highway 47
Bryan, TX 77807



DISCLAIMER

The contents of this report reflect the views of the authors, who are solely responsible for the facts and accuracy of the data, and the opinions, findings, and conclusions presented herein. The contents do not necessarily reflect the official views or policies of the Roadside Safety Research Pooled Fund, The Texas A&M University System, or Texas A&M Transportation Institute. This report does not constitute a standard, specification, or regulation. In addition, the above listed agencies/ companies assume no liability for its contents or use thereof. The names of specific products or manufacturers listed herein do not imply endorsement of those products or manufacturers.

The results reported herein apply only to the article being tested. The full-scale crash test was performed according to TTI Proving Ground quality procedures, and according to the *MASH* guidelines and standards.

The Proving Ground Laboratory within the Texas A&M Transportation Institute's Proving Ground ("TTI Lab" or "TTI LAB") strives for accuracy and completeness in its crash test reports. On rare occasions, unintentional or inadvertent clerical errors, technical errors, omissions, oversights, or misunderstandings (collectively referred to as "errors") may occur, and may not be identified for corrective action prior to the final report being published and issued. When the TTI Lab discovers an error in a published and issued final report, the TTI Lab shall promptly disclose such error to the Roadside Safety Research Pooled Fund, and both parties shall endeavor in good faith to resolve this situation. The TTI Lab will be responsible for correcting the error that occurred in the report, which may be in form of errata, amendment, replacement sections, or up to and including full reissuance of the report. The cost of correcting an error in the report shall be borne by TTI Lab. Any such errors or inadvertent delays that occur in connection with the performance of the related testing contract shall not constitute a breach of the testing contract.

THE TTI LAB SHALL NOT BE LIABLE FOR ANY INDIRECT, CONSEQUENTIAL, PUNITIVE, OR OTHER DAMAGES SUFFERED BY ROADSIDE SAFETY RESEARCH POOLED FUND OR ANY OTHER PERSON OR ENTITY, WHETHER SUCH LIABILITY IS BASED, OR CLAIMED TO BE BASED, UPON ANY NEGLIGENT ACT, OMISSION, ERROR, CORRECTION OF ERROR, DELAY, OR BREACH OF AN OBLIGATION BY THE TTI LAB.

1. Report No. 610221-01-1		2. Government Accession No.		3. Recipient's Catalog No.	
4. Title and Subtitle MASH TEST 4-12 ON KEYED-IN SINGLE-SLOPE BARRIER WITH 40-FT SEGMENT LENGTH				5. Report Date October 2018	
				6. Performing Organization Code	
7. Author(s) N. M. Sheikh, B.L. Griffith, and D.L. Kuhn				8. Performing Organization Report No. Test Report No. 610221-01-1	
9. Performing Organization Name and Address Texas A&M Transportation Institute Proving Ground 3135 TAMU College Station, Texas 77843-3135				10. Work Unit No. (TRAIS)	
				11. Contract or Grant No. 1806061	
12. Sponsoring Agency Name and Address Washington State Department of Transportation Transportation Building, MS 47372 Olympia, Washington 98504-7372				13. Type of Report and Period Covered Technical Report: March 2018 – October 2018	
				14. Sponsoring Agency Code	
15. Supplementary Notes Project Title: MASH Testing of Keyed-In Single Slope Barrier Name of Contacting Representative: Kurt Brauner, LADODT					
16. Abstract <p>The objective of this research was to determine the impact performance of a single slope barrier restrained by keying it 1-inch into asphalt pavement. The barrier was constructed with a 40-ft long segment unconnected to the adjacent barrier segment. The objective of this test was to determine if the 40-ft long keyed-in barrier segment could successfully contain and redirect an impacting vehicle. The barrier was evaluated by performing American Association of State Highway and Transportation Officials (AASHTO), <i>Manual for Assessing Safety Hardware (MASH)</i> Test 4-12 with a single-unit truck. <i>MASH</i> Test 4-12 involves a 10000S vehicle impacting the barrier at a target impact speed and angle of 56 mi/h and 15°, respectively.</p> <p>This report provides details of the keyed-in single slope barrier with the 40-ft segment, detailed documentation of the crash test results, and an assessment of the performance of the barrier for <i>MASH</i> Test 4-12 evaluation criteria. Based on the results of the test, the keyed-in single-slope barrier with the unconnected 40-ft segment performed acceptably for <i>MASH</i> Test 4-12.</p>					
17. Key Words Longitudinal Barrier, Concrete Barrier, Single Slope, Crash Testing, Median Barrier, Roadside Safety			18. Distribution Statement Copyrighted. Not to be copied or reprinted without consent from Roadside Safety Pooled Fund.		
19. Security Classif.(of this report) Unclassified		20. Security Classif.(of this page) Unclassified		21. No. of Pages 52	
				22. Price	

REPORT AUTHORIZATION

REPORT REVIEWED BY:

DocuSigned by:
Glenn Schroeder
E692F9CB5047487...

Glenn Schroeder, Research Specialist
Drafting & Reporting

DocuSigned by:
Gary Gerke
FBA2101E9F6B4B7...

Gary Gerke, Research Specialist
Construction

DocuSigned by:
Scott Dobrovolsky
1C613885787C44C...

Scott Dobrovolsky, Research Specialist
Mechanical Instrumentation

DocuSigned by:
Ken Reeves
60D556935596468...

Ken Reeves, Research Specialist
Electronics Instrumentation

DocuSigned by:
Richard Badillo
0F51DA60AB144F9...

Richard Badillo, Research Specialist
Photographic Instrumentation

DocuSigned by:
Wanda L. Menges
B92179622AF24EF...

Wanda L. Menges, Research Specialist
Reporting & Deputy QM

DocuSigned by:
Darrell L. Kuhn
D4CC23E85D5B4E7...

Darrell L. Kuhn, P.E., Research Specialist
Quality Manager

DocuSigned by:
Matt Robinson
FAA22BFA5BFD417...

Matthew N. Robinson, Research Specialist
Test Facility Manager & Technical Manager

DocuSigned by:
Nauman M. Sheikh
662F8266A604403...

Nauman M. Sheikh, P.E.
Associate Research Engineer

ACKNOWLEDGMENTS

This research project was performed under a pooled fund program between the following States and Agencies. The authors acknowledge and appreciate their guidance and assistance.

Roadside Safety Research Pooled Fund Committee Revised June 2018

ALASKA

Jeff C. Jeffers, P.E.

Statewide Standard Specifications
Alaska Department of Transportation &
Public Facilities
3132 Channel Drive
P.O. Box 112500
Juneau, AK 99811-2500
(907) 465-8962
Jeff.Jeffers@alaska.gov

CALIFORNIA

Bob Meline, P.E.

Caltrans
Office of Materials and Infrastructure
Division of Research and Innovation
5900 Folsom Blvd
Sacramento, CA 95819
(916) 227-7031
Bob.Meline@dot.ca.gov

John Jewell, P.E.

(916) 227-5824
John_Jewell@dot.ca.gov

COLORADO

Joshua Keith, P.E.

Standards & Specifications Engineer
Project Development Branch
4201 E Arkansas Ave, 4th Floor
Denver, CO 80222
(303) 757-9021
Josh.Keith@state.co.us

CONNECTICUT

David Kilpatrick

State of Connecticut Department of
Transportation
2800 Berlin Turnpike
Newington, CT 06131-7546
(806) 594-3288
David.Kilpatrick@ct.gov

DELAWARE

Mark Buckalew, P.E.

Safety Programs Manager
Delaware Department of Transportation
169 Brick Store Landing Road
Smyrna, DE 19977
(302) 659-4073
Mark.Buckalew@state.de.us

FLORIDA

Derwood C. Sheppard, Jr., P.E.

Design Standards Administrator
FDOT Roadway Design Office
Florida Department of Transportation
605 Suwannee Street
Tallahassee, FL 32399-0450
(850) 414-4334
Derwood.Sheppard@dot.state.fl.us

IDAHO

Kevin Sablan

Design and Traffic Engineer
Idaho Transportation Department
P. O. Box 7129
Boise, ID 83707-1129
(208) 334-8558
Kevin.Sablan@ITD.idaho.gov

ILLINOIS

Martha A. Brown, P.E.

Safety Design Bureau Chief
Bureau of Safety Programs and Engineering
Illinois Department of Transportation
2300 Dirksen Parkway, Room 005
Springfield, IL 62764
(217) 785-3034
Martha.A.Brown@illinois.gov

Filberto (Fil) Sotelo

Safety Evaluation Engineer
(217) 557-2563
Filiberto.Sotelo@illinois.gov

ILLINOIS (Continued)

Jon M. McCormick

Safety Policy & Initiatives Engineer
(217) 785-5678
Jon.M.McCormick@illinois.gov

LOUISIANA

Chris Guidry

Bridge Manager
Louisiana Transportation Center
Bridge & Structural Design Section
P.O. Box 94245
Baton Rouge, LA 70802
(225) 379-1933
Chris.Guidry@la.gov

Kurt Brauner, P.E.

Bridge Engineer Manager
Louisiana Transportation Center
1201 Capital Road, Suite 605G
Baton Rouge, LA 70802
(225) 379-1302
Kurt.Brauner@la.gov

Paul B. Fossier, Jr., P.E.

Bridge Design Engineer Administrator
Paul.Fossier@la.gov

Steve Mazur

Bridge Design
(225) 379-1094
Steven.Mazur@la.gov

MASSACHUSETTS

Alex Bardow

Director of Bridges and Structure
Massachusetts Department of
Transportation
10 Park Plaza, Room 6430
Boston, MA 02116
(517) 335-9430
Alexander.Bardow@state.ma.us

James Danila

Assistant State Traffic Engineer
Massachusetts Department of
Transportation
10 Park Plaza, Room 7210
Boston, MA 02116
(857) 368-9640
James.Danila@state.ma.us

MICHIGAN

Carlos Torres, P.E.

Crash Barrier Engineer
Geometric Design Unit, Design Division
Michigan Department of Transportation
P. O. Box 30050
Lansing, MI 48909
(517) 335-2852
TorresC@michigan.gov

MINNESOTA

Michael Elle, P.E.

Design Standards Engineer
Minnesota Department of Transportation
395 John Ireland Blvd, MS 696
St. Paul, MN 55155-1899
(651) 366-4622
Michael.Elle@dot.state.mn.us

Michelle Moser

Assistant Design Standards Engineer
MnDOT-Office of Project Management and
Technical Support
(651) 366-4708
Michelle.Moser@state.mn.us

MISSOURI

Ronald Effland, P.E., ACTAR

Non-Motorized Transportation Engineer
Missouri Department of Transportation
P.O. Box 868
Springfield, MO 65801
(417) 895-7649
Ronald.Effland@modot.mo.gov

OKLAHOMA

Hebret Bokhru, P.E.

Engineering Manager
Traffic Engineering Division
Oklahoma Department of Transportation
200 NE 21st Street, 2-A7
Oklahoma City, OK 73105-3204
Office (direct): (405) 522-5373
Office (Traffic Div.): (405) 521-2861
Hebret.Bokhru@odot.org

OREGON

Christopher Henson

Senior Roadside Design Engineer
Oregon Department of Transportation
Technical Service Branch
4040 Fairview Industrial Drive, SE
Salem, OR 97302-1142
(503) 986-3561
Christopher.S.Henson@odot.state.or.us

PENNSYLVANIA

Divyang P. Pathak, EIT

Standards & Criteria Engineer
Pennsylvania Department of Transportation
Bureau of Project Delivery
400 North Street, 7th Floor
Harrisburg, PA 17120-0094
(717) 705-4190
DPathak@pa.gov

Guozhou Li

GuLi@pa.gov

Hassan Raza

HRaza@pa.gov

TENNESSEE

Ali Hangul, P.E., CPESC

Assistant Director
Tennessee Department of Transportation
Roadway Design & Office of Aerial Surveys
James K. Polk State Office Bldg, Ste 1300
505 Deaderick Street
Nashville, TN 37243
(615) 741-0840
Ali.Hangul@tn.gov

TEXAS

Chris Lindsey

Transportation Engineer
Design Division
Texas Department of Transportation
125 East 11th Street
Austin, TX 78701-2483
(512) 416-2750
Christopher.Lindsey@txdot.gov

Taya Retterer P.E.

TXDOT Bridge Standards Engineer
(512) 416-2719
Taya.Retterer@txdot.gov

TEXAS (Continued)

Wade Odell

Transportation Engineer
Research & Technology Implementation
200 E. Riverside Drive
Austin, TX 78704
Wade.Odell@txdot.gov

WASHINGTON

Jeffery K. Petterson, P.E.

Design Policy & Strategic Analysis
Estimating Manager
Roadside Safety Engineer
Washington State Department of
Transportation
P. O. Box 47329
Olympia, WA 98504-7246
(360) 705-7278
PetterJ@wsdot.wa.gov

Mustafa Mohamedali

Assistant Research Project Manager
(360) 704-6307
BrookRh@wsdot.wa.gov

Rhonda Brooks

Director of Research Office
(360) 705-7945
BrookRh@wsdot.wa.gov

WEST VIRGINIA

Donna J. Hardy, P.E.

Safety Programs Engineer
West Virginia Department of
Transportation – Traffic Engineering
Building 5, Room A-550
1900 Kanawha Blvd E.
Charleston, WV 25305-0430
(304) 558-9576
Donna.J.Hardy@wv.gov

Ted Whitmore

Traffic Services Engineer
(304) 558-9468
Ted.J.Whitmore@wv.gov

WEST VIRGINIA (Continued)

Joe Hall, P.E., P.S.

Division of Highways & Engineering
Technical Policy QA/QC Engineer
Value Engineering Coordinator
1334 Smith Street
Charleston, WV 25305-0430
(304) 558-9733
Joe.H.Hall@wv.gov

WISCONSIN

Erik Emerson, P.E.

Standards Development Engineer –
Roadside Design
Wisconsin Department of Transportation
Bureau of Project Development
4802 Sheboygan Avenue, Room 651
P. O. Box 7916
Madison, WI 53707-7916
(608) 266-2842
Erik.Emerson@wi.gov

CANADA – ONTARIO

Mark Ayton, P. Eng.

Senior Engineer, Highway Design
Design & Contract Standards Office
Ontario Ministry of Transportation
Garden City Tower, 2nd Floor North
301 St. Paul Street
St. Catharines, Ontario
L2R 7R4
(904) 704-2051
Mark.Ayton@ontario.ca

FEDERAL HIGHWAY ADMINISTRATION (FHWA)

WebSite: safety.fhwa.dot.gov

Richard B. (Dick) Albin, P.E.

Safety Engineer
FHWA Resource Center Safety & Design
Technical Services Team
711 S. Capital
Olympia, WA 98501
(303) 550-8804
Dick.Albin@dot.gov

FHWA (Continued)

William Longstreet

Highway Engineer
FHWA Office of Safety Design
Room E71-107
1200 New Jersey Avenue, S.E.
Washington, DC 20590
(202) 366-0087
Will.Longstreet@dot.gov

Eduardo Arispe

Research Highway Safety Specialist
U.S. Department of Transportation
Federal Highway Administration
Turner-Fairbank highway Research Center
Mail Code: HRDS-10
6300 Georgetown Pike
McLean, VA 22101
(202) 493-3291
Eduardo.arispe@dot.gov

Greg Schwartz, P.E.

FHWA – Federal Lands Highway Division
Safety Discipline Champion
123 West Dakota Ave. Ste. 210
Lakewood, CO 80228
(720)-963-3764
Greg.Schwartz@dot.gov

TEXAS A&M TRANSPORTATION INSTITUTE (TTI)

WebSite: tti.tamu.edu
www.roadsidepooledfund.org

D. Lance Bullard, Jr., P.E.

Senior Research Engineer
Roadside Safety & Physical Security Div.
Texas A&M Transportation Institute
3135 TAMU
College Station, TX 77843-3135
(979) 845-6153
L-Bullard@tti.tamu.edu

Roger P. Bligh, Ph.D., P.E.

Senior Research Engineer
(979) 845-4377
R-Bligh@tti.tamu.edu

Chiara Silvestri Dobrovoly, Ph.D.

Associate Research Scientist
(979) 845-8971
C-Silvestri@tti.tamu.edu

TABLE OF CONTENTS

	Page
Disclaimer	ii
List of Figures.....	x
List of Tables	xi
Chapter 1. Introduction.....	1
1.1 Background	1
1.2 Objective	1
Chapter 2. System Details.....	3
2.1. Test Article and Installation Details	3
Chapter 3. Test Requirements and Evaluation Criteria	7
3.1. Crash Test Performed / Matrix	7
3.2. Evaluation Criteria	7
Chapter 4. Test Conditions.....	9
4.1. Test Facility	9
4.2 Vehicle Tow and Guidance System	9
4.3 Data Acquisition Systems	9
4.3.1 Vehicle Instrumentation and Data Processing	9
4.3.3 Photographic Instrumentation Data Processing	10
Chapter 5. MASH Test 4-12 (Crash Test No. 610221-01-1)	13
5.1 Test Designation and Actual Impact Conditions	13
5.2 Weather Conditions	13
5.3 Test Vehicle	13
5.4 Test Description	14
5.5 Damage to Test Installation	14
5.6 Vehicle Damage.....	16
5.7 Occupant Risk Factors	17
Chapter 6. Summary and Conclusions	21
6.1. Assessment of Test Results.....	21
6.2 Conclusions.....	21
6.3 Implementation	21
References.....	25
Appendix A. Details of the Keyed-In Single Slope Barrier.....	27
Appendix B. MASH Test 4-12 (Crash Test No. 610221-01-1)	29
B1 Vehicle Properties and Information	29
B2 Sequential Photographs.....	31
B3 Vehicle Angular Displacements	34
B4 Vehicle Accelerations	35

LIST OF FIGURES

	Page
Figure 2.1. Details of the Keyed-In Single Slope Barrier.....	4
Figure 2.2. Keyed-In Single Slope Barrier prior to Testing.	5
Figure 5.1. Keyed-In Single Slope Barrier/Test Vehicle Geometrics for Test No. 610221-01-1.....	13
Figure 5.2. Test Vehicle before Test No. 610221-01-1.	14
Figure 5.3. Keyed-In Single Slope Barrier after Test No. 610221-01-1.	15
Figure 5.4. Keyed-In Single Slope Barrier Asphalt after Test No. 610221-01-1.	16
Figure 5.5. Test Vehicle after Test No. 610221-01-1.....	17
Figure 5.6. Interior of Test Vehicle for Test No. 610221-01-1.	17
Figure 5.7. Summary of Results for <i>MASH</i> Test 4-12 on Keyed-In Single Slope Barrier.	19
Figure B.1. Sequential Photographs for Test No. 610221-01-1 (Overhead and Frontal Views).	31
Figure B.2. Sequential Photographs for Test No. 610221-01-1 (Rear View).	33
Figure B.3. Vehicle Angular Displacements for Test No. 610221-01-1.....	34
Figure B.4. Vehicle Longitudinal Accelerometer Trace for Test No. 610221-01-1 (Accelerometer Located at Center of Gravity).....	35
Figure B.5. Vehicle Lateral Accelerometer Trace for Test No. 610221-01-1 (Accelerometer Located at Center of Gravity).....	36
Figure B.6. Vehicle Vertical Accelerometer Trace for Test No. 610221-01-1 (Accelerometer Located at Center of Gravity).....	37
Figure B.7. Vehicle Longitudinal Accelerometer Trace for Test No. 610221-01-1 (Accelerometer Located Rear of Center of Gravity).....	38
Figure B.8. Vehicle Lateral Accelerometer Trace for Test No. 610221-01-1 (Accelerometer Located Rear of Center of Gravity).....	39
Figure B.9. Vehicle Vertical Accelerometer Trace for Test No. 610221-01-1 (Accelerometer Located Rear of Center of Gravity).....	40

LIST OF TABLES

	Page
Table 3.1. Test Conditions and Evaluation Criteria Specified for <i>MASH</i> Test 4-12 for Longitudinal Barriers.	7
Table 3.2. Evaluation Criteria Required for <i>MASH</i> Test 4-12 for Longitudinal Barriers.....	8
Table 5.1. Events during Test No. 610221-01-1.....	14
Table 5.2. Occupant Risk Factors for Test No. 610221-01-1.	18
Table 6.1. Performance Evaluation Summary for <i>MASH</i> Test 4-12 on Keyed-In Single Slope Barrier.	23
Table B.1. Vehicle Properties for Test No. 610221-01-1.....	29

SI* (MODERN METRIC) CONVERSION FACTORS				
APPROXIMATE CONVERSIONS TO SI UNITS				
Symbol	When You Know	Multiply By	To Find	Symbol
LENGTH				
in	inches	25.4	millimeters	mm
ft	feet	0.305	meters	m
yd	yards	0.914	meters	m
mi	miles	1.61	kilometers	km
AREA				
in ²	square inches	645.2	square millimeters	mm ²
ft ²	square feet	0.093	square meters	m ²
yd ²	square yards	0.836	square meters	m ²
ac	acres	0.405	hectares	ha
mi ²	square miles	2.59	square kilometers	km ²
VOLUME				
fl oz	fluid ounces	29.57	milliliters	mL
gal	gallons	3.785	liters	L
ft ³	cubic feet	0.028	cubic meters	m ³
yd ³	cubic yards	0.765	cubic meters	m ³
NOTE: volumes greater than 1000L shall be shown in m ³				
MASS				
oz	ounces	28.35	grams	g
lb	pounds	0.454	kilograms	kg
T	short tons (2000 lb)	0.907	megagrams (or metric ton")	Mg (or "t")
TEMPERATURE (exact degrees)				
°F	Fahrenheit	5(F-32)/9 or (F-32)/1.8	Celsius	°C
FORCE and PRESSURE or STRESS				
lbf	poundforce	4.45	newtons	N
lbf/in ²	poundforce per square inch	6.89	kilopascals	kPa
APPROXIMATE CONVERSIONS FROM SI UNITS				
Symbol	When You Know	Multiply By	To Find	Symbol
LENGTH				
mm	millimeters	0.039	inches	in
m	meters	3.28	feet	ft
m	meters	1.09	yards	yd
km	kilometers	0.621	miles	mi
AREA				
mm ²	square millimeters	0.0016	square inches	in ²
m ²	square meters	10.764	square feet	ft ²
m ²	square meters	1.195	square yards	yd ²
ha	hectares	2.47	acres	ac
km ²	Square kilometers	0.386	square miles	mi ²
VOLUME				
mL	milliliters	0.034	fluid ounces	oz
L	liters	0.264	gallons	gal
m ³	cubic meters	35.314	cubic feet	ft ³
m ³	cubic meters	1.307	cubic yards	yd ³
MASS				
g	grams	0.035	ounces	oz
kg	kilograms	2.202	pounds	lb
Mg (or "t")	megagrams (or "metric ton")	1.103	short tons (2000lb)	T
TEMPERATURE (exact degrees)				
°C	Celsius	1.8C+32	Fahrenheit	°F
FORCE and PRESSURE or STRESS				
N	newtons	0.225	poundforce	lbf
kPa	kilopascals	0.145	poundforce per square inch	lb/in ²

*SI is the symbol for the International System of Units

Chapter 1. INTRODUCTION

1.1 BACKGROUND

The Pooled Fund Program stated a desire for a design of a single slope barrier restrained by keying into asphalt pavement, but with a shorter segment length than the recently tested Texas Department of Transportation's (TxDOT) single slope barrier design, which had a segment length of 75 ft (1).

Texas A&M Transportation Institute (TTI) previously tested a 42-inch tall single slope barrier keyed 1-inch deep into a 1-inch thick layer of asphalt pavement on the traffic and field sides of the barrier. An American Association of State Highway and Transportation Officials (AASHTO), *Manual for Assessing Safety Hardware (MASH)* Test 4-12 with a single-unit truck was performed on this barrier. The vehicle impacted a 75 ft long segment of the barrier and successfully contained and redirected the test vehicle (1, 2). The test installation of this system was still available at the TTI Proving Ground and the Pooled Fund stated a desire to use the same installation, but test a 40-ft segment length of the barrier using *MASH* Test 4-12 evaluation criteria.

1.2 OBJECTIVE

The objective of this research was to use an existing installation of the 42-inch tall, single slope barrier keyed into asphalt, and determine the impact performance of the barrier with a segment length of 40 feet. The performance of the barrier was evaluated by performing *MASH* Test 4-12 with a single-unit truck. This test involves a 10000S vehicle impacting the barrier at a target impact speed and impact angle of 56 mi/h and 15°, respectively.

This report provides details of the keyed-in single slope barrier with a 40-ft segment length, detailed documentation of the crash test results, and an assessment of the performance of the barrier for *MASH* Test 4-12 evaluation criteria.

This page intentionally left blank.

Chapter 2. SYSTEM DETAILS

2.1. TEST ARTICLE AND INSTALLATION DETAILS

The test installation consisted of four collinear single slope reinforced concrete barrier segments, 42 inches tall, 8 inches wide at top, and sloping symmetrically on both sides to 24 inches wide at bottom. The total test installation length was 120 ft. The first barrier segment, which was impacted in the test, was 40 ft long. All barrier segments were sitting on unreinforced jointed-concrete pavement and keyed into a 1-inch thick asphalt overlay that was 9 ft wide on each side of the barrier. Adjacent barrier segments were not connected to each other.

Figure 2.1 presents overall information on the keyed-in single slope barrier, and Figure 2.2 provides photographs of the installation. Appendix A provides further details of the barrier.

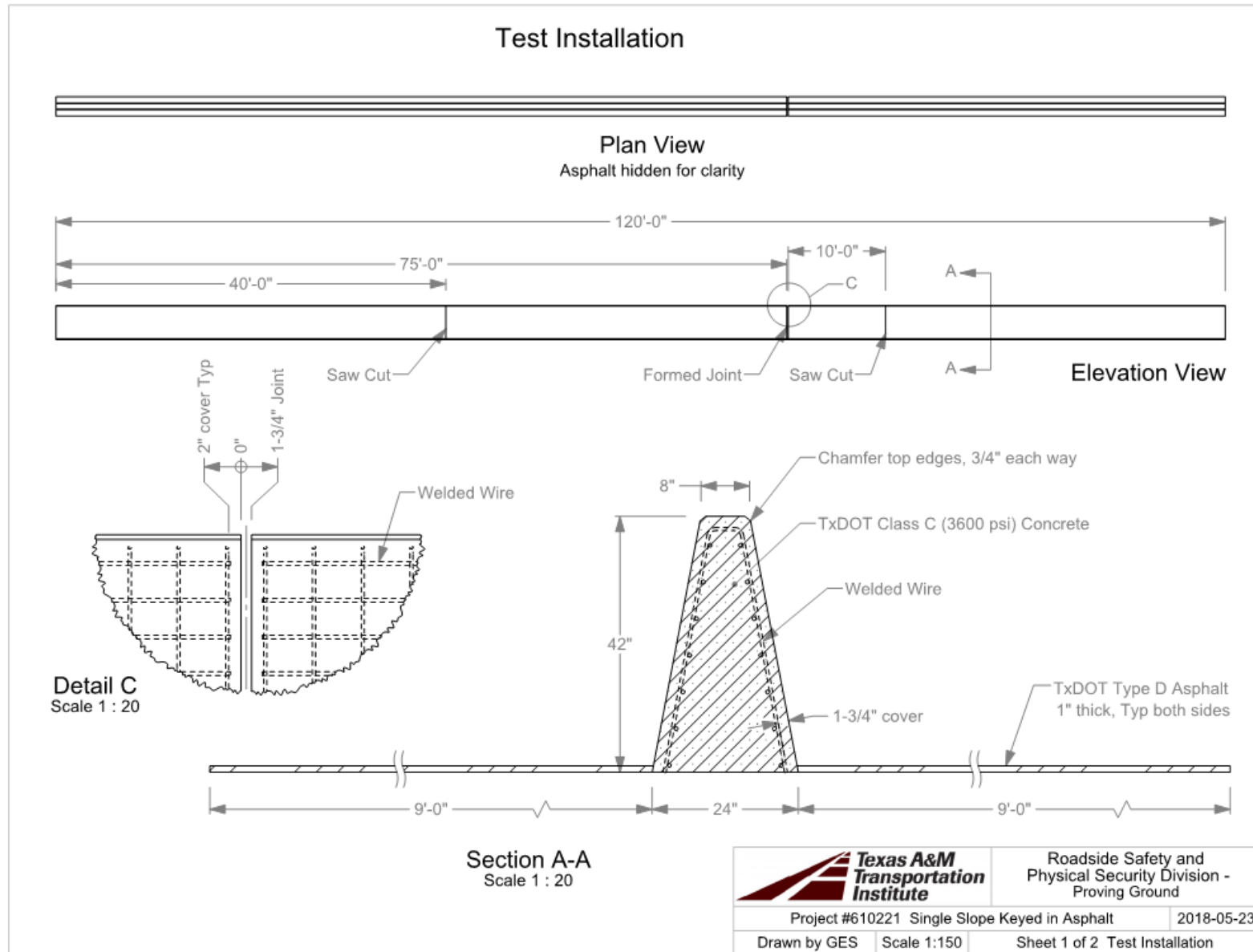


Figure 2.1. Details of the Keyed-In Single Slope Barrier.



Figure 2.2. Keyed-In Single Slope Barrier prior to Testing.

This page intentionally left blank.

Chapter 3. TEST REQUIREMENTS AND EVALUATION CRITERIA

3.1. CRASH TEST PERFORMED / MATRIX

Table 3.1 shows the test conditions and evaluation criteria for *MASH* Test 4-12. *MASH* Test 4-12 involves a 10000S vehicle weighing 22,046 lb (± 660 lb) impacting the critical impact point (CIP) of the barrier at a speed of 56 mi/h (± 2.5 mi/h) and an angle of 15° ($\pm 1.5^\circ$). To maximize the toppling potential and the lateral load into the 40-ft barrier segment, the target CIP was determined to be 5 ft upstream of the middle of the 40-ft segment (25 ft upstream of the joint). Information provided in *MASH* Section 2.2.1, Section 2.3.2, and Figure 2-1 were also used in determining the CIP. The crash test and data analysis procedures were in accordance with guidelines presented in *MASH*. Chapter 4 presents brief descriptions of these procedures.

Table 3.1. Test Conditions and Evaluation Criteria Specified for *MASH* Test 4-12 for Longitudinal Barriers.

Test Article	Test Designation	Test Vehicle	Impact Conditions		Evaluation Criteria
			Speed	Angle	
Longitudinal Barrier	4-12	10000S	56 mi/h	15°	A, D, G

3.2. EVALUATION CRITERIA

The appropriate safety evaluation criteria from Tables 2-2A and 5-1 of *MASH* were used to evaluate the crash test reported herein. The test conditions and evaluation criteria required for *MASH* Test 4-12 are listed in Table 3.1, and the substance of the evaluation criteria in Table 3.2. An evaluation of the crash test results is presented in detail under the section Assessment of Test Results.

Table 3.2. Evaluation Criteria Required for MASH Test 4-12 for Longitudinal Barriers.

Evaluation Factors	Evaluation Criteria
Structural Adequacy	A. <i>Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underide, or override the installation although controlled lateral deflection of the test article is acceptable.</i>
Occupant Risk	D. <i>Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present undue hazard to other traffic, pedestrians, or personnel in a work zone.</i> <i>Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.2.2 and Appendix E of MASH.</i>
	G. <i>It is preferable, although not essential, that the vehicle remain upright during and after the collision.</i>

Chapter 4. TEST CONDITIONS

4.1. TEST FACILITY

The full-scale crash test reported herein was performed at Texas A&M Transportation Institute (TTI) Proving Ground, an International Standards Organization (ISO)/International Electrotechnical Commission (IEC) 17025-accredited laboratory with American Association for Laboratory Accreditation (A2LA) Mechanical Testing Certificate 2821.01. The full-scale crash test was performed according to TTI Proving Ground quality procedures, and according to the *MASH* guidelines and standards.

The test facilities of the TTI Proving Ground are located on the Texas A&M University System RELLIS Campus, which consists of a 2000-acre complex of research and training facilities situated 10 miles northwest of the flagship campus of Texas A&M University. The site, formerly a United States Army Air Corps base, has large expanses of concrete runways and parking aprons well suited for experimental research and testing in the areas of vehicle performance and handling, vehicle-roadway interaction, durability and efficacy of highway pavements, and evaluation of roadside safety hardware and perimeter protective devices. The site selected for construction and testing of the keyed-in single slope barrier was an out-of-service apron. The apron consists of an unreinforced jointed-concrete pavement in 12.5-ft × 15-ft blocks nominally 6 inches deep. The aprons were built in 1942, and the joints have some displacement, but are otherwise flat and level.

4.2 VEHICLE TOW AND GUIDANCE SYSTEM

The test vehicle was towed into the test installation using a steel cable guidance and reverse tow system. A steel cable for guiding the test vehicle was tensioned along the path, anchored at each end, and threaded through an attachment to the front wheel of the test vehicle. An additional steel cable was connected to the test vehicle, passed around a pulley near the impact point, through a pulley on the tow vehicle, and then anchored to the ground such that the tow vehicle moved away from the test site. A 2:1 speed ratio between the test and tow vehicle existed with this system. Just prior to impact with the installation, the test vehicle was released and ran unrestrained. The vehicle remained freewheeling (i.e., no steering or braking inputs) until it cleared the immediate area of the test site (no sooner than 2 s after impact), after which the brakes were activated to bring the test vehicle to a safe and controlled stop.

4.3 DATA ACQUISITION SYSTEMS

4.3.1 Vehicle Instrumentation and Data Processing

The test vehicle was instrumented with a self-contained, on-board data acquisition system. The signal conditioning and acquisition system is a 16-channel, Tiny Data Acquisition System (TDAS) Pro produced by Diversified Technical Systems, Inc. The accelerometers, which measure the x, y, and z axis of vehicle acceleration, are strain gauge type with linear millivolt output proportional to acceleration. Angular rate sensors, measuring vehicle roll, pitch, and yaw rates, are ultra-small, solid state units designed for crash test service. The TDAS Pro hardware

and software conform to the latest SAE J211, Instrumentation for Impact Test. Each of the 16 channels is capable of providing precision amplification, scaling, and filtering based on transducer specifications and calibrations. During the test, data are recorded from each channel at a rate of 10,000 values per second with a resolution of one part in 65,536. Once data are recorded, internal batteries back these up inside the unit should the primary battery cable be severed. Initial contact of the pressure switch on the vehicle bumper provides a time zero mark as well as initiates the recording process. After each test, the data are downloaded from the TDAS Pro unit into a laptop computer at the test site. The Test Risk Assessment Program (TRAP) software then processes the raw data to produce detailed reports of the test results.

Each of the TDAS Pro units is returned to the manufacturer annually for complete recalibration and all instrumentation used in the vehicle conforms to all specifications outlined by SAE J211. All accelerometers are calibrated annually by means of an ENDEVCO® 2901, precision primary vibration standard. This standard and its support instruments are checked annually and receive a National Institute of Standards Technology (NIST) traceable calibration. The rate transducers used in the data acquisition system receive a calibration via a Genisco Rate-of-Turn table. The subsystems of each data channel are also evaluated annually, using instruments with current NIST traceability, and the results are factored into the accuracy of the total data channel, per SAE J211. Calibrations and evaluations are also made any time data are suspect. Acceleration data is measured with an expanded uncertainty of ± 1.7 percent at a confidence factor of 95 percent ($k=2$).

TRAP uses the data from the TDAS Pro to compute occupant/compartiment impact velocities, time of occupant/compartiment impact after vehicle impact, and the highest 10-millisecond (ms) average ridedown acceleration. TRAP calculates change in vehicle velocity at the end of a given impulse period. In addition, maximum average accelerations over 50-ms intervals in each of the three directions are computed. For reporting purposes, the data from the vehicle-mounted accelerometers are filtered with a 60-Hz low-pass digital filter, and acceleration versus time curves for the longitudinal, lateral, and vertical directions are plotted using TRAP.

TRAP uses the data from the yaw, pitch, and roll rate transducers to compute angular displacement in degrees at 0.0001-s intervals, then plots yaw, pitch, and roll versus time. These displacements are in reference to the vehicle-fixed coordinate system with the initial position and orientation of the vehicle-fixed coordinate systems being initial impact. Rate of rotation data is measured with an expanded uncertainty of ± 0.7 percent at a confidence factor of 95 percent ($k=2$).

4.3.3 Photographic Instrumentation Data Processing

Photographic coverage of the test included three digital high-speed cameras:

- One overhead with a field of view perpendicular to the ground and directly over the impact point;
- One placed behind the installation at an angle; and
- A third placed to have a field of view parallel to and aligned with the installation at the downstream end.

A flashbulb on the impacting vehicle was activated by a pressure-sensitive tape switch to indicate the instant of contact with the keyed-in single slope barrier. The flashbulb was visible from each camera. The video files from these digital high-speed cameras were analyzed to observe phenomena occurring during the collision and to obtain time-event, displacement, and angular data. A digital camera recorded and documented conditions of each test vehicle and the installation before and after the test.

This page intentionally left blank.

Chapter 5. *MASH* TEST 4-12 (CRASH TEST NO. 610221-01-1)

5.1 TEST DESIGNATION AND ACTUAL IMPACT CONDITIONS

MASH Test 4-12 involves a 10000S vehicle weighing 22,046 lb \pm 660 lb impacting the CIP of the barrier at an impact speed of 56 mi/h \pm 2.5 mi/h and an angle of 15° \pm 1.5°. The target CIP for *MASH* Test 4-12 on the keyed-in single slope barrier was 5 ft upstream of the middle of the 40-ft long barrier segment (25 ft upstream of the joint) \pm 1 ft.

The 2013 International 4300 single-unit truck used in the test weighed 22,210 lb, and the actual impact speed and angle were 56.2 mi/h and 14.6°, respectively. The actual impact point was 24.2 ft upstream of the first joint. Minimum target impact severity (IS) was 142 kip-ft, and actual IS was 149 kip-ft.

5.2 WEATHER CONDITIONS

The test was performed on the morning of June 8, 2018. Weather conditions at the time of testing were as follows: wind speed: 6 mi/h; wind direction: 176 degrees (vehicle was traveling in a northerly direction); temperature: 89°F; relative humidity: 72 percent.

5.3 TEST VEHICLE

Figures 5.1 and 5.2 show the 2013 International 4300 single-unit truck used for the crash test. The vehicle's test inertia weight was 22,210 lb, and its gross static weight was 22,210 lb. The height to the lower edge of the vehicle bumper was 18.5 inches, and height to the upper edge of the bumper was 33.5 inches. The height to the ballast's center of gravity was 62.5 inches. Table B.1 in Appendix B1 gives additional dimensions and information on the vehicle. The vehicle was directed into the installation using a cable reverse tow and guidance system, and was released to be freewheeling and unrestrained just prior to impact.



Figure 5.1. Keyed-In Single Slope Barrier/Test Vehicle Geometrics for Test No. 610221-01-1.



Figure 5.2. Test Vehicle before Test No. 610221-01-1.

5.4 TEST DESCRIPTION

The test vehicle was traveling at an impact speed of 56.2 mi/h when it contacted the keyed-in single slope barrier 24.2 ft upstream of the first joint, at an impact angle of 14.6°. Table 5.1 lists events that occurred during Test No. 610221-01-1. Figures B.1 and B.2 in Appendix B2 present sequential photographs during the test.

Table 5.1. Events during Test No. 610221-01-1.

TIME (s)	EVENTS
0.000	Vehicle contacts barrier
0.046	Vehicle begins to redirect
0.104	Front right tire lifts off of pavement
0.197	Right rear tires come off of pavement
0.245	Vehicle parallel with barrier
0.248	Rear left corner of box impacts barrier
0.484	Vehicle lost contact with barrier while traveling at 52.8 mi/h
0.827	Front right tire lands back on pavement
1.311	All tires land back on pavement

5.5 DAMAGE TO TEST INSTALLATION

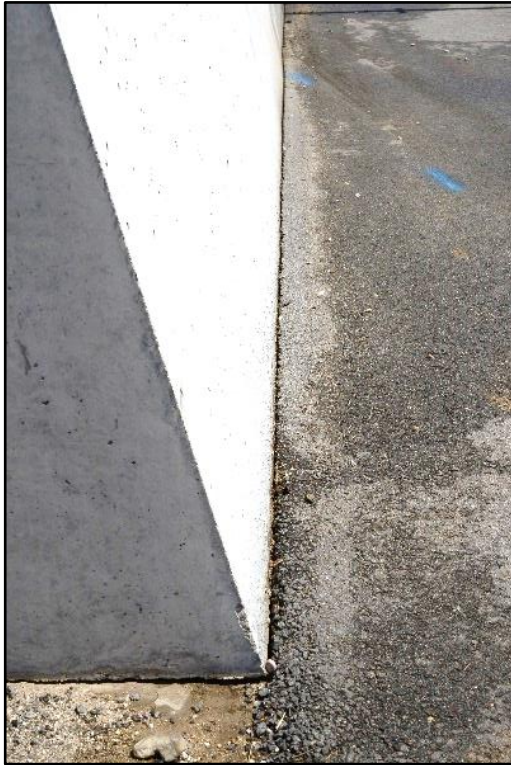
Figure 5.3 shows the damage to the barrier. The traffic face of the barrier was gouged 1.25 inches at impact. The downstream end of the 40-ft long impact segment was leaning toward the field side 0.75 inch at the top. Working width was 59.8 inches at a height of 112.4 inches. Maximum dynamic deflection during the test was 5.8 inches, and maximum permanent deflection was 0.75 inch.

Figure 5.4 shows the damage to the asphalt at the base of the barrier. The base of the barrier was pushed back toward the protected side leaving a ½-inch gap from the face of the

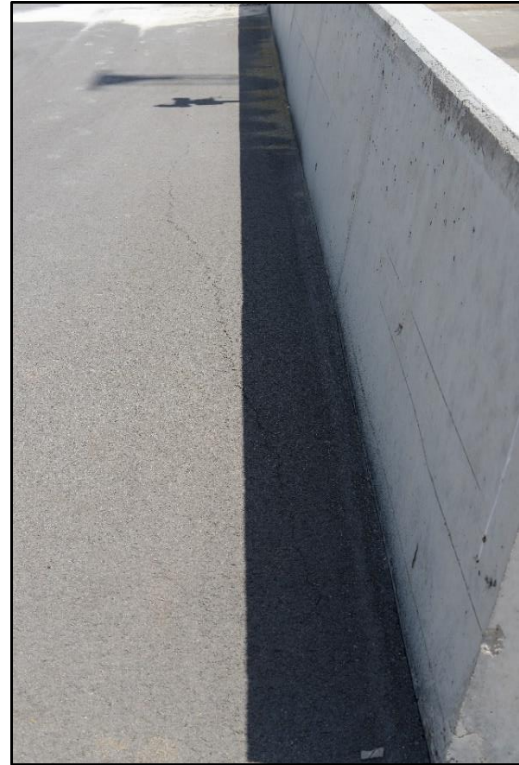
barrier to the start of the asphalt. A single crack in the asphalt began on the back side of the barrier at the upstream end. The crack propagated through the asphalt to 28 inches back from the impact zone.



Figure 5.3. Keyed-In Single Slope Barrier after Test No. 610221-01-1.



Impact Side



Field Side



Field Side

Figure 5.4. Keyed-In Single Slope Barrier Asphalt after Test No. 610221-01-1.

5.6 VEHICLE DAMAGE

Figure 5. shows the damage sustained by the vehicle. The front bumper, hood, left front tire and rim, left battery box, left lower edge of cargo box, and left rear outer tire and rim were damaged. Maximum exterior crush to the vehicle was 18.0 inches in the side plane at the left front corner at bumper height. Maximum occupant compartment deformation was 4.0 inches in the left side kick panel area. Figure 5. shows the interior of the vehicle.



Figure 5.5. Test Vehicle after Test No. 610221-01-1.



Before Test

After Test

Figure 5.6. Interior of Test Vehicle for Test No. 610221-01-1.

5.7 OCCUPANT RISK FACTORS

Data from the accelerometer, located at the vehicle center of gravity, were digitized for informational purposes only, and are shown in Table 5.2. Figure 5. summarizes these data and other pertinent information from the test. Figure B.3 in Appendix B3 shows the vehicle angular displacements, and Figures B.4 through B.9 in Appendix B4 show acceleration versus time traces.

Table 5.2. Occupant Risk Factors for Test No. 610221-01-1.

Occupant Risk Factors		
Occupant Impact Velocity (OIV)	at 0.1746 s on left side of interior	
x-direction	6.2 ft/s	
y-direction	13.1 ft/s	
THIV (km/hr):	16.6	at 0.1692 s on left side of interior
THIV (m/s):	4.6	
Ridedown Accelerations (g)		
x-direction	3.4	(0.2497 - 0.2597 s)
y-direction	15.8	(0.2514 - 0.2614 s)
PHD (g):	15.9	(0.2513 - 0.2613 s)
ASI:	0.74	(0.2992 - 0.3492 s)
Max. 50msec Moving Avg. Accelerations (g's)		
x-direction	-1.7	(0.0075 - 0.0575 s)
y-direction	6.6	(0.2690 - 0.3190 s)
z-direction	-4.3	(0.0223 - 0.0723 s)
Max Roll, Pitch, and Yaw Angles (degrees)		
Roll	32	(0.7790 s)
Pitch	12	(0.8002 s)
Yaw	27	(1.2069 s)

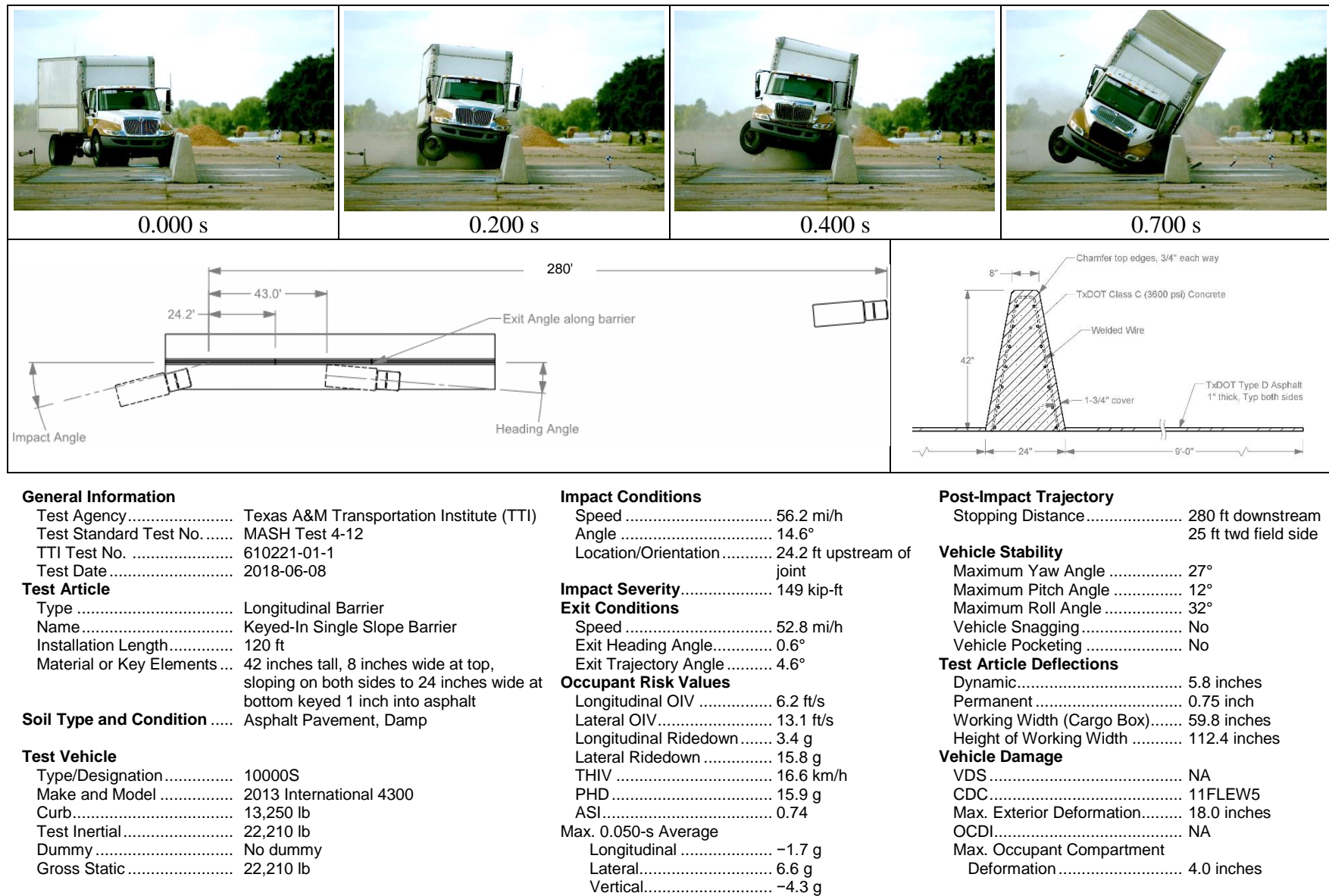


Figure 5.7. Summary of Results for *MASH* Test 4-12 on Keyed-In Single Slope Barrier.

Chapter 6. SUMMARY AND CONCLUSIONS

6.1. ASSESSMENT OF TEST RESULTS

The crash test reported herein was performed in accordance with evaluation criteria for *MASH* Test 4-12. A 10000S vehicle impacted the keyed-in single slope barrier with 40-ft segment at an impact speed and impact angle of 56.2 mi/h and 14.6°, respectively. An assessment of the test based on the applicable safety evaluation criteria for *MASH* Test 4-12 for longitudinal barriers is provided in Table 6.1.

6.2 CONCLUSIONS

The keyed-in single slope barrier with a 40-ft segment performed acceptably for *MASH* Test 4-12 criteria for longitudinal barriers.

6.3 IMPLEMENTATION*

For *MASH* TL- 4, in addition to the Test 4-12 with the single unit truck, *MASH* also requires longitudinal barriers to be evaluated with the pickup truck (Test 4-11) and the small car (Test 4-10). Both of these tests were not considered critical and were thus not performed. Under this project, the keyed-in single slope barrier with a 40-ft segment length was primarily being evaluated for its potential to contain and redirect the *MASH* single unit truck. There were concerns that under the impact from this vehicle, the barrier segment might topple or have enough lateral rotation or movement that results in the barrier being unable to contain and redirect the vehicle. The results of the crash test showed that the barrier was successful in containing and redirecting the vehicle in a very stable manner, and with minimal deflection or rotation. Testing with the small car and the pickup truck is not expected to impart greater lateral load into the barrier segment compared to the single unit truck. Thus, there are no concerns for these vehicles to topple or breach the barrier. Furthermore, in past testing, the single slope barrier has been successfully crash tested with the small car and the pickup truck vehicles under *MASH* Test 4-10 and 4-11 conditions, respectively (3, 4). Based on these facts, it was determined that Tests 4-10 and 4-11 of *MASH* are not critical and therefore not needed.

The 40-ft barrier segment in this project was tested keyed into 1-inch of asphalt. Using greater thicknesses of asphalt or restraining the barrier by keying it into concrete are also considered acceptable alternatives. Increasing pavement thickness and/or using concrete material increase the lateral restraint capacity of the keyed-in barrier system. These changes are not expected to negatively affect the performance of the barrier. It should, however, be noted that for *MASH* TL-4, the barrier should have a minimum 36-inch height. For this reason, the key-in design or the extent of overlays should not reduce the effective height of the barrier below 36 inches.

* The opinions/interpretations in this section of the report are outside the scope of TTI Proving Ground's A2LA Accreditation.

The 1-inch asphalt layer that keyed in the test installation barrier was constructed on an existing concrete pavement. In a field installation, having the underlying concrete pavement is not a requirement. Any base material such as asphalt, road base, soil, etc. that is well compacted and stable enough to construct and compact a 1-inch thick asphalt pad for keying in the barrier is expected to result in similar barrier performance.

It is sometimes desired to have drainage slots along the base of the barrier. Presence of drainage slots decreases the total length of the barrier that is keyed into asphalt, thus reducing the lateral restraint provided by the asphalt key-in. If drainage slots are desired, the length of the barrier segment should be increased so that at least 40 ft length is keyed in. As an example, if it is desired to have drainage slots that are 2 ft long, spaced 10 ft on centers, the length of the barrier segment should be increased to 50 ft so that at least 40 ft of the barrier is restrained by the asphalt.

Table 6.1. Performance Evaluation Summary for MASH Test 4-12 on Keyed-In Single Slope Barrier.

Test Agency: Texas A&M Transportation Institute

Test No.: 610221-01-1

Test Date: 2018-06-08

MASH Test 4-12 Evaluation Criteria	Test Results	Assessment
<u>Structural Adequacy</u>		
A. <i>Test article should contain and redirect the vehicle or bring the vehicle to a controlled stop; the vehicle should not penetrate, underride, or override the installation although controlled lateral deflection of the test article is acceptable.</i>	The keyed-in single slope barrier contained and redirected the 10000S vehicle. The vehicle did not penetrate, underride, or override the installation. Maximum dynamic deflection during the test was 5.8 inches.	Pass
<u>Occupant Risk</u>		
D. <i>Detached elements, fragments, or other debris from the test article should not penetrate or show potential for penetrating the occupant compartment, or present an undue hazard to other traffic, pedestrians, or personnel in a work zone.</i>	No detached elements, fragments, or other debris were present to penetrate or show potential to penetrate the occupant compartment, or to present hazard to others in the area.	Pass
<i>Deformations of, or intrusions into, the occupant compartment should not exceed limits set forth in Section 5.2.2 and Appendix E of MASH.</i>	Maximum occupant compartment deformation was 4.0 inches in the left kick panel area.	
G. <i>It is preferable, although not essential, that the vehicle remain upright during and after collision.</i>	The 10000S vehicle remained upright during and after the collision event. Maximum roll angle was 32°.	Pass

REFERENCES

1. Bligh, R.P., Menges, W.L., and Kuhn, D.L. (2017). “MASH Test 4-12 of the TxDOT 42-inch Single Slope Concrete Barrier (SSCB) in 1-inch Asphalt.” Texas A&M Transportation Institute, College Station, Texas.
2. AASHTO. *Manual for Assessing Roadside Safety Hardware, Second Edition*. 2016, American Association of State Highway and Transportation Officials: Washington, D.C.
3. Whitesel, D., Jewell, J., and Meline, R. (2018). “Compliance Crash Testing of the Type 60 Median Barrier, Test 140MASH3C16-04.” Roadside Safety Research Group, California Department of the Transportation, Sacramento, California.
4. Sheikh, N.M., Bligh, R. P., and Menges, W.L. (2009). “Development and Testing of a Concrete Barrier Design for Use in Front of Slope or on MSE Wall.” Report 405160-13-1, Texas A&M Transportation Institute, College Station, Texas.

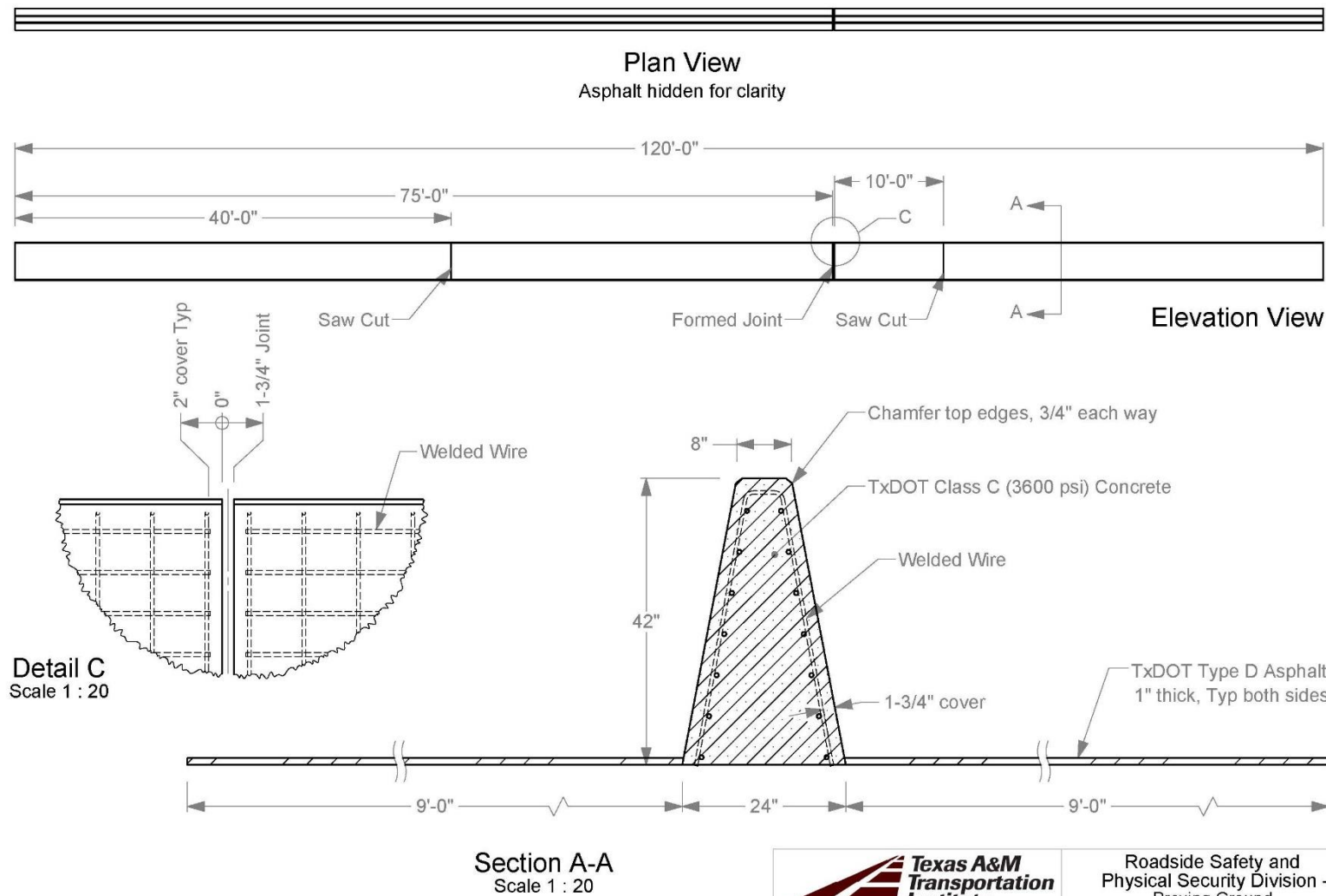
This page intentionally left blank.

APPENDIX A. DETAILS OF THE KEYED-IN SINGLE SLOPE BARRIER

BARRIER

T:\1-ProjectFiles\610221-01-Keyed-In SS Barrier-Sheikh\Drafting, 610221\610221 Drawing

Test Installation



Roadside Safety and
Physical Security Division -
Proving Ground

Project #610221 Single Slope Keyed in Asphalt

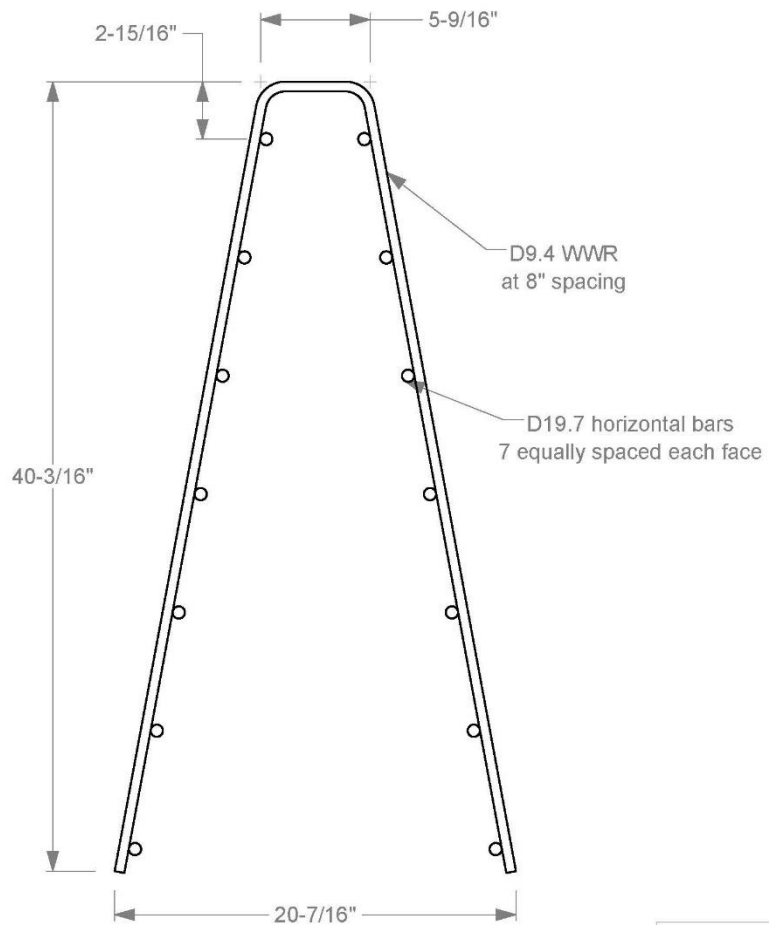
2018-05-23

Drawn by GES

Scale 1:150

Sheet 1 of 2 Test Installation

Welded Wire



Welded Wire
Grade 70



Roadside Safety and
Physical Security Division -
Proving Ground

Project #610221 Single Slope Keyed in Asphalt

2018-05-23

Drawn by GES

Scale 1:8

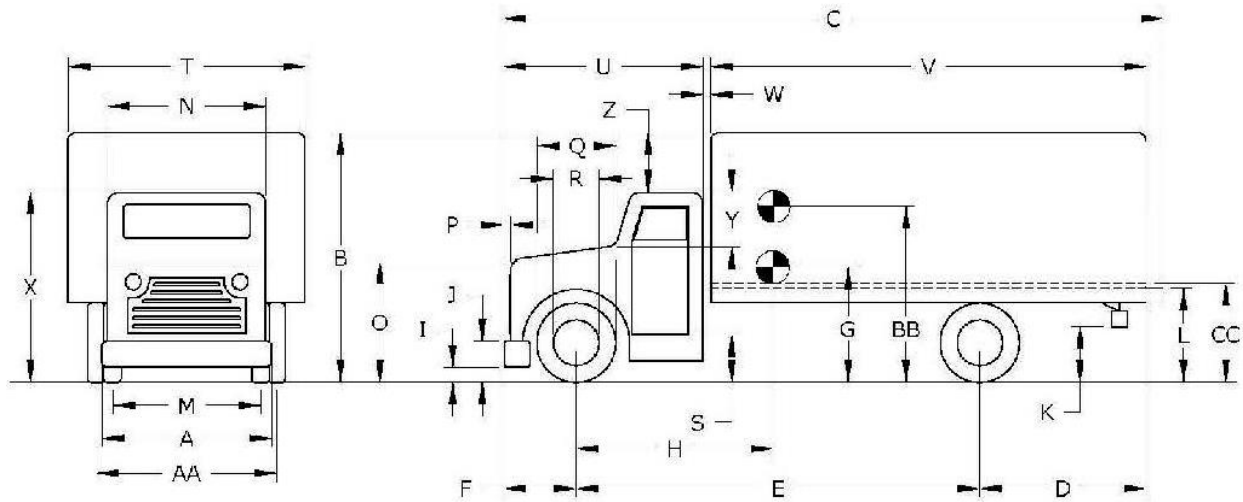
Sheet 2 of 2 Welded Wire

APPENIDX B. MASH TEST 4-12 (CRASH TEST NO. 610221-01-1)

B1 VEHICLE PROPERTIES AND INFORMATION

Table B.1. Vehicle Properties for Test No. 610221-01-1.

Date:	<u>2018-06-08</u>	Test No.:	<u>610221-01-1</u>	VIN No.:	<u>1HTMMAAND7H156262</u>
Year:	<u>2013</u>	Make:	<u>International</u>	Model:	<u>4300</u>
Odometer:	<u>82887</u>	Tire Size Front:	<u>275/80 R22.5</u>	Tire Size Rear:	<u>275/80 R22.5</u>



Vehicle Geometry: inches

A Front Bumper Width:	<u>92.00</u>	K Rear Bumper Bottom:		U Cab Length:	<u>103.50</u>
B Overall Height:	<u>134.00</u>	L Rear Frame Top:	<u>37.75</u>	V Trailer/Box Length:	<u>214.00</u>
C Overall Length:	<u>319.75</u>	M Front Track Width:	<u>79.50</u>	W Gap Width:	<u>2.50</u>
D Rear Overhang:	<u>76.00</u>	N Roof Width:	<u>72.00</u>	X Overall Front Height:	<u>97.00</u>
E Wheel Base:	<u>204.75</u>	O Hood Height:	<u>59.00</u>	Y Roof-Hood Distance:	<u>22.00</u>
F Front Overhang:	<u>39.00</u>	P Bumper Extension:	<u>4.00</u>	Z Roof-Box Height Difference:	<u>36.00</u>
G C.G. Height:		Q Front Tire Width:	<u>40.00</u>	AA Rear Track Width:	<u>73.00</u>
H C.G. Horizontal Dist. w/Ballast:	<u>126.29</u>	R Front Wheel Width:	<u>23.25</u>	BB Ballast Center of Mass:	<u>62.50</u>
I Front Bumper Bottom:	<u>18.50</u>	S Bottom Door Height:	<u>37.50</u>	CC Cargo Bed Height:	<u>51.00</u>
J Front Bumper Top:	<u>33.50</u>	T Overall Width:	<u>96.00</u>		
Allowable Range: C = 394 inches max.; E = 240 inches max.; CC = 51 ±2 inches; BB = 63 ±2 inches above ground;					
Wheel Center Height Front	<u>18.75</u>	Wheel Well Clearance (Front)	<u>8.50</u>	Bottom Frame Height (Front)	<u>26.00</u>
Wheel Center Height Rear	<u>18.75</u>	Wheel Well Clearance (Rear)	<u>7.50</u>	Bottom Frame Height (Rear)	<u>27.50</u>

Table B1. Vehicle Properties for Test No. 610221-01-1 (Continued).

Date: 2018-06-08 Test No.: 610221-01-1 VIN No.: 1HTMMAAND7H156262
 Year: 2013 Make: International Model: 4300

WEIGHTS

lb

CURB

TEST INERTIAL

$W_{\text{front axle}}$

7,150

8,510

$W_{\text{rear axle}}$

6,100

13,700

W_{TOTAL}

13,250

22210

Allowable Range for CURB = 13,200 \pm 2200 lb | Allowable Range for TIM = 22,046 \pm 660 lb

Ballast: 8,960 lb

(as-needed)

(See MASH Section 4.2.1.2 for recommended ballasting)

Mass Distribution

lb

LF: 4370

RF: 4140

LR: 6780

RR: 6920

Engine Type: DT

Accelerometer Locations inches

Engine Size: 466

x^1

y

z^2

Front:

126.25

0.00

50.00

Center:

226.25

0.00

50.00

Rear:

Transmission Type:



Auto

or



Manual



FWD



RWD



4WD

Describe any damage to the vehicle prior to test:

Other notes to include ballast type, dimensions, mass, location, center of mass, and method of attachment:

2 BLOCKS H 30" W 60" L 30"

TIED DOWN WITH 4 5/16 CABLES PER BLOCK

CENTERED IN MIDDLE OF BLOCK

¹ Referenced to the front axle

² Above ground

B2 SEQUENTIAL PHOTOGRAPHS



0.000 s



0.100 s



0.200 s



0.300 s



Figure B.1. Sequential Photographs for Test No. 610221-01-1 (Overhead and Frontal Views).



0.400 s



0.500 s



0.600 s



0.700 s



Figure B.1. Sequential Photographs for Test No. 610221-01-1 (Overhead and Frontal Views) (Continued).



0.000 s



0.400 s



0.100 s



0.500 s



0.200 s



0.600 s



0.300 s



0.700 s

Figure B.2. Sequential Photographs for Test No. 610221-01-1 (Rear View).

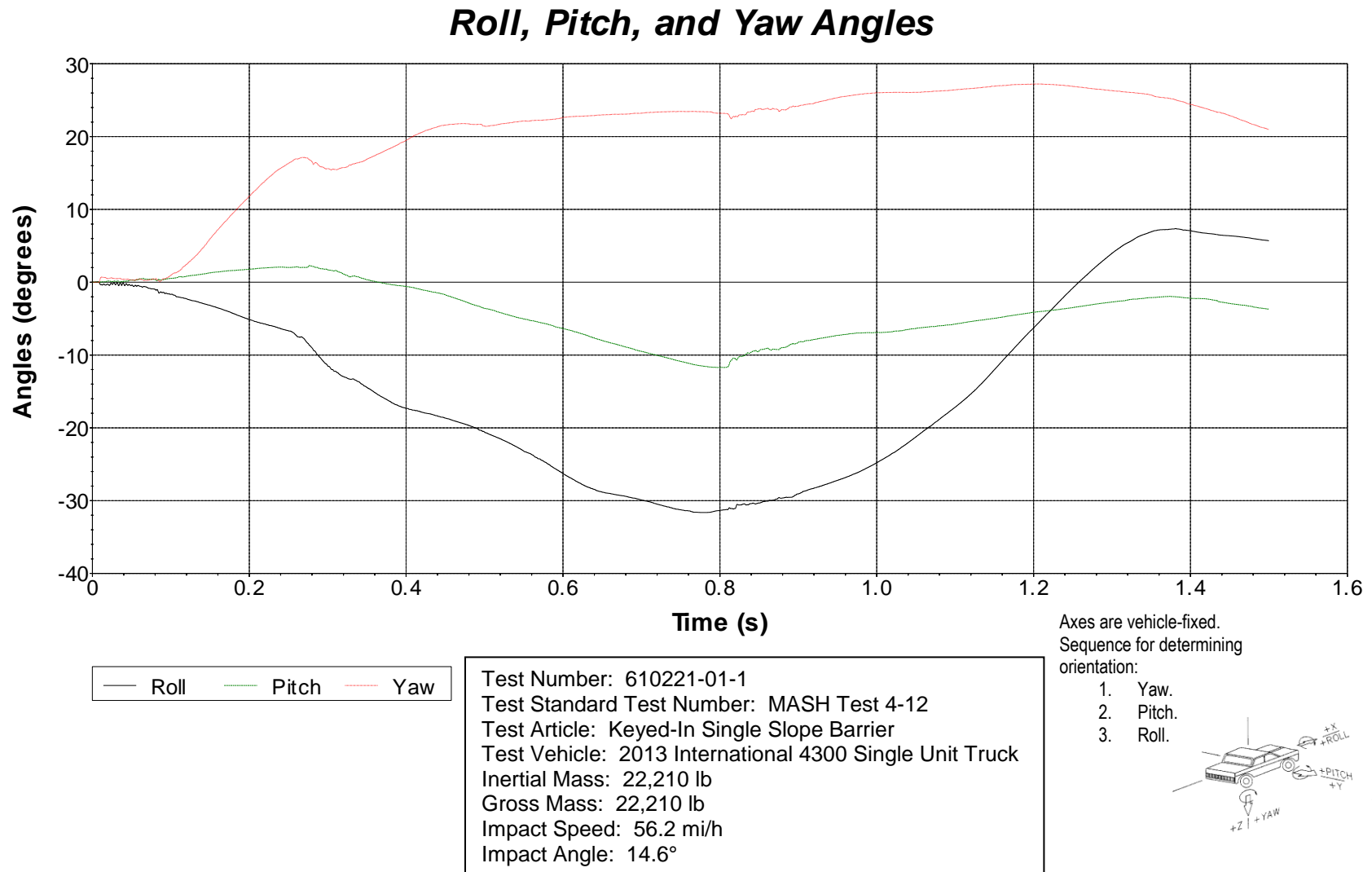
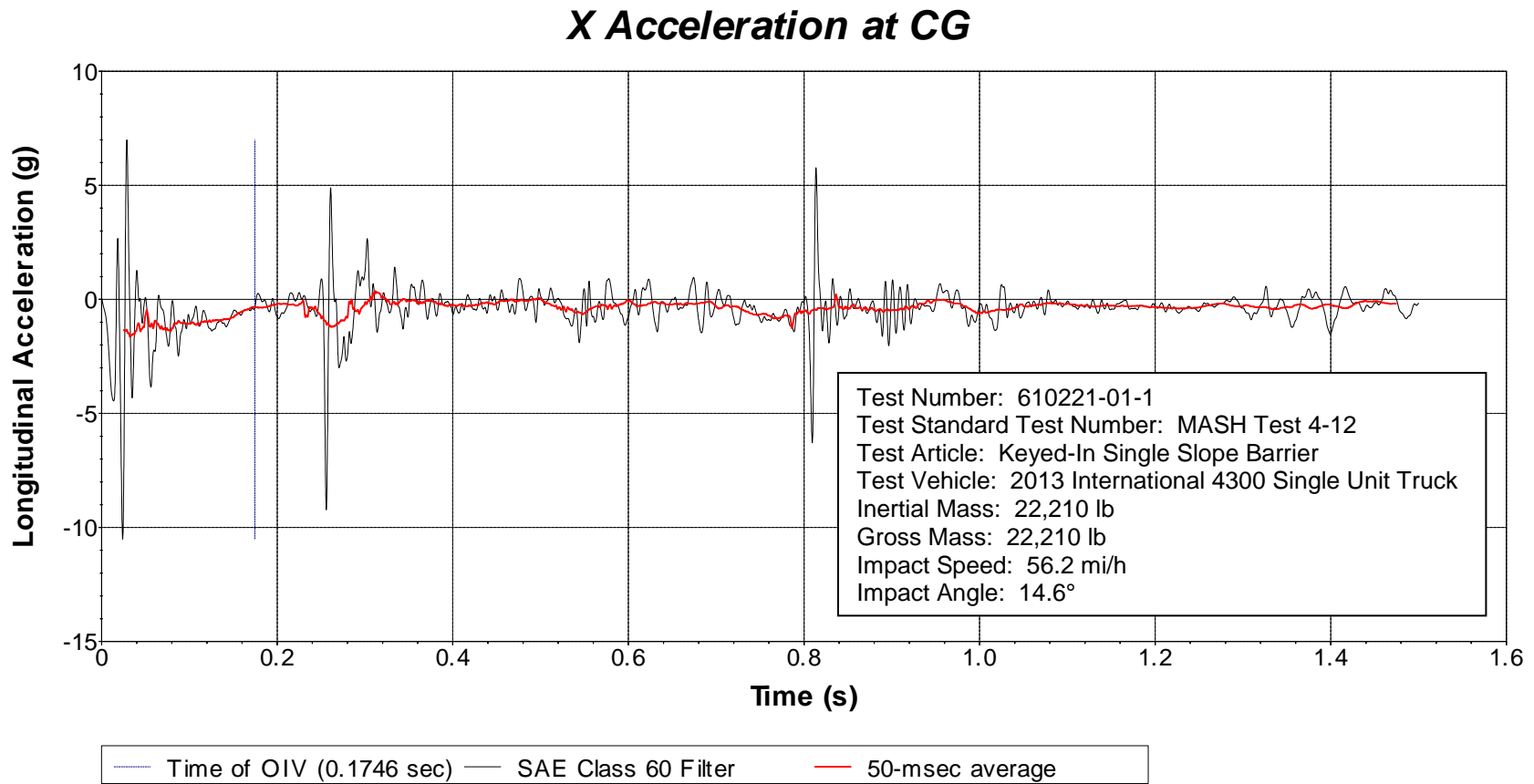
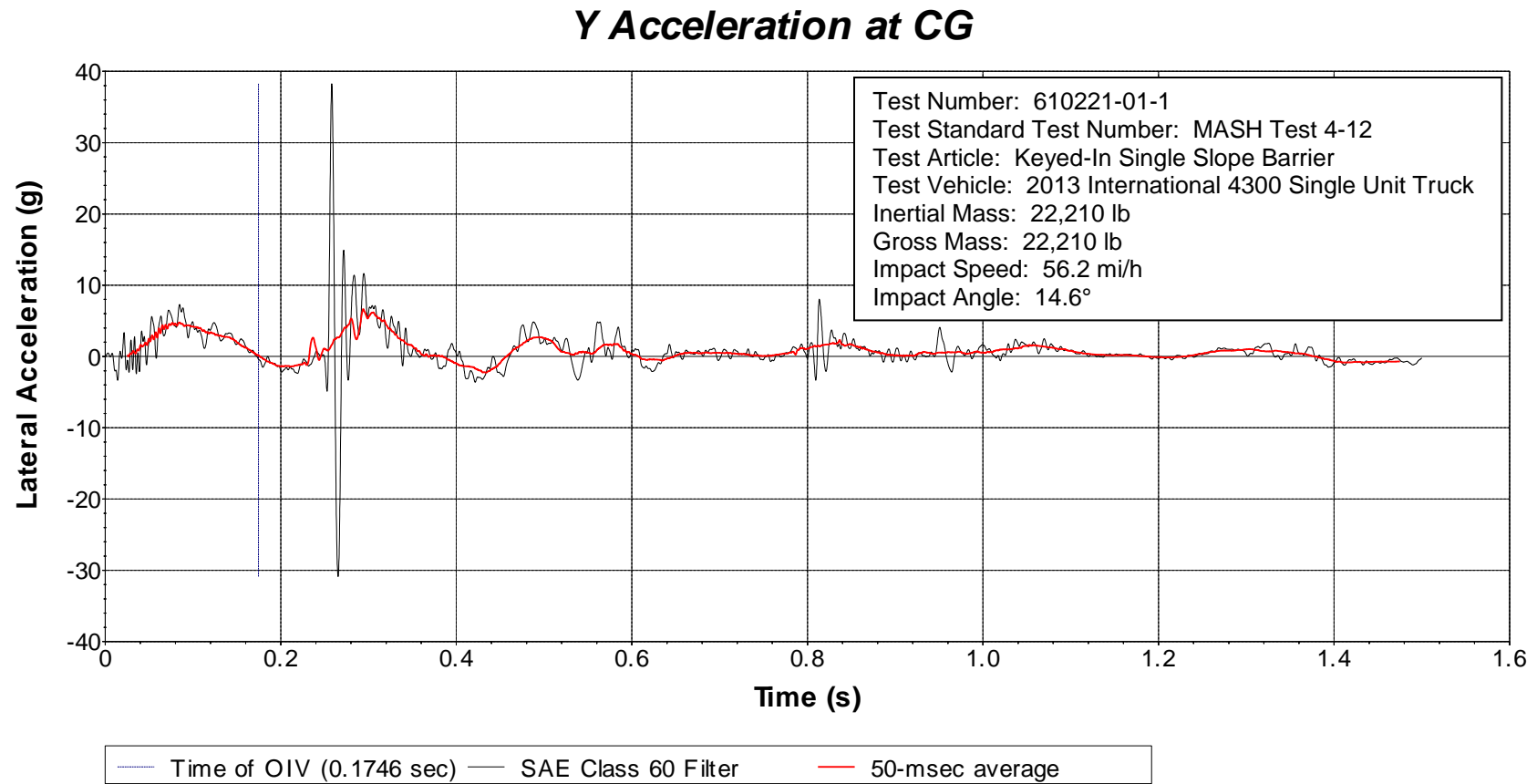


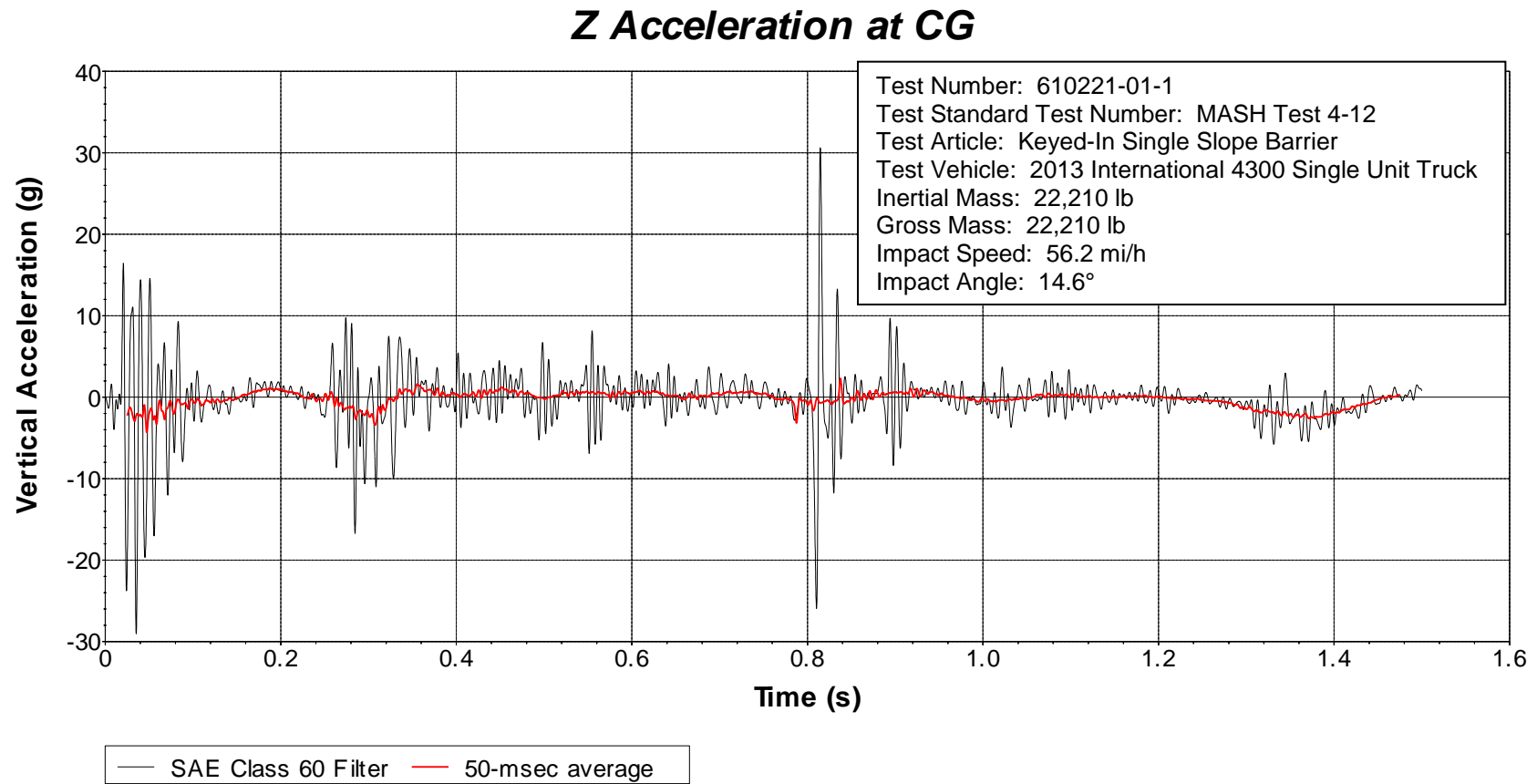
Figure B.3. Vehicle Angular Displacements for Test No. 610221-01-1.



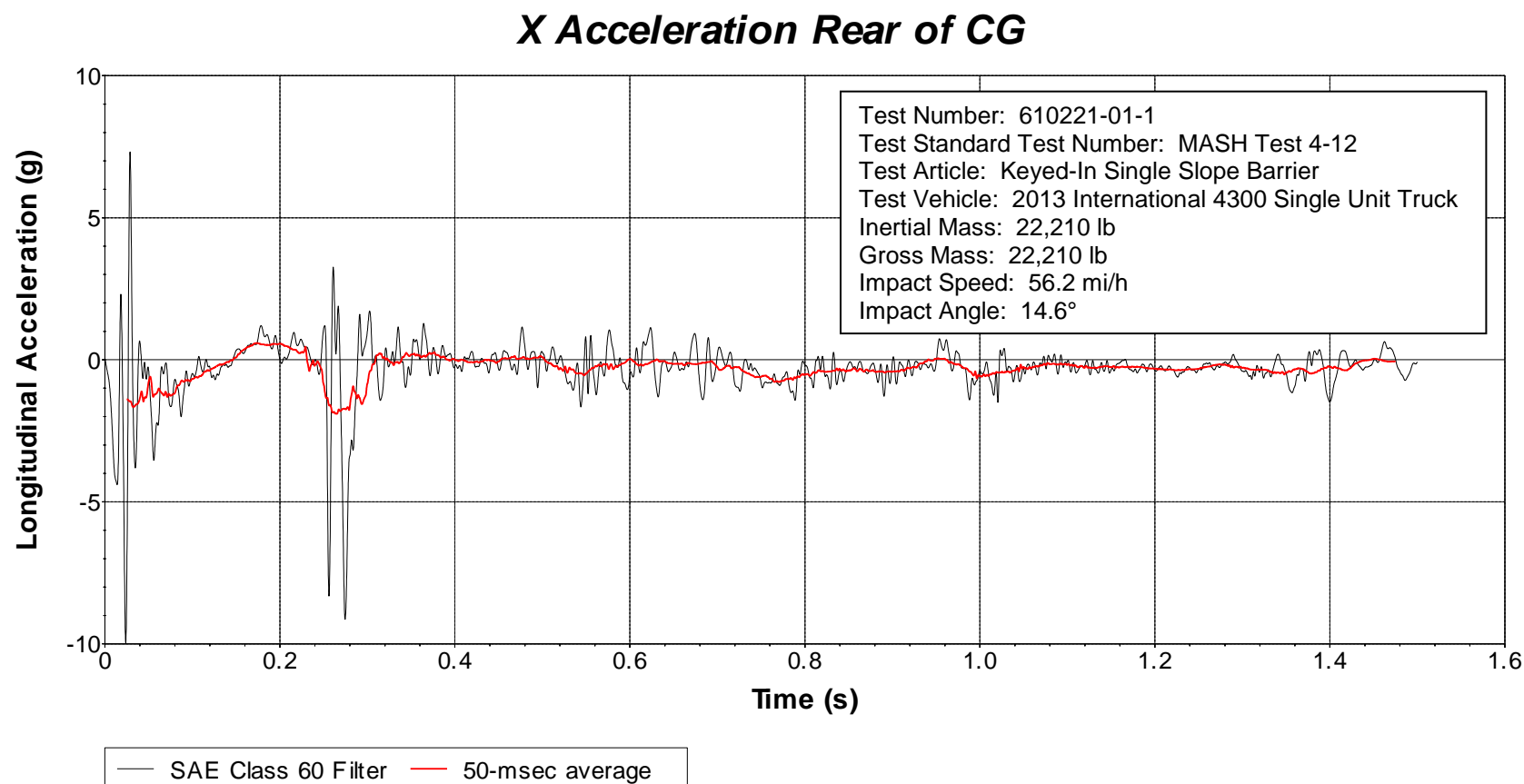
**Figure B.4. Vehicle Longitudinal Accelerometer Trace for Test No. 610221-01-1
 (Accelerometer Located at Center of Gravity).**



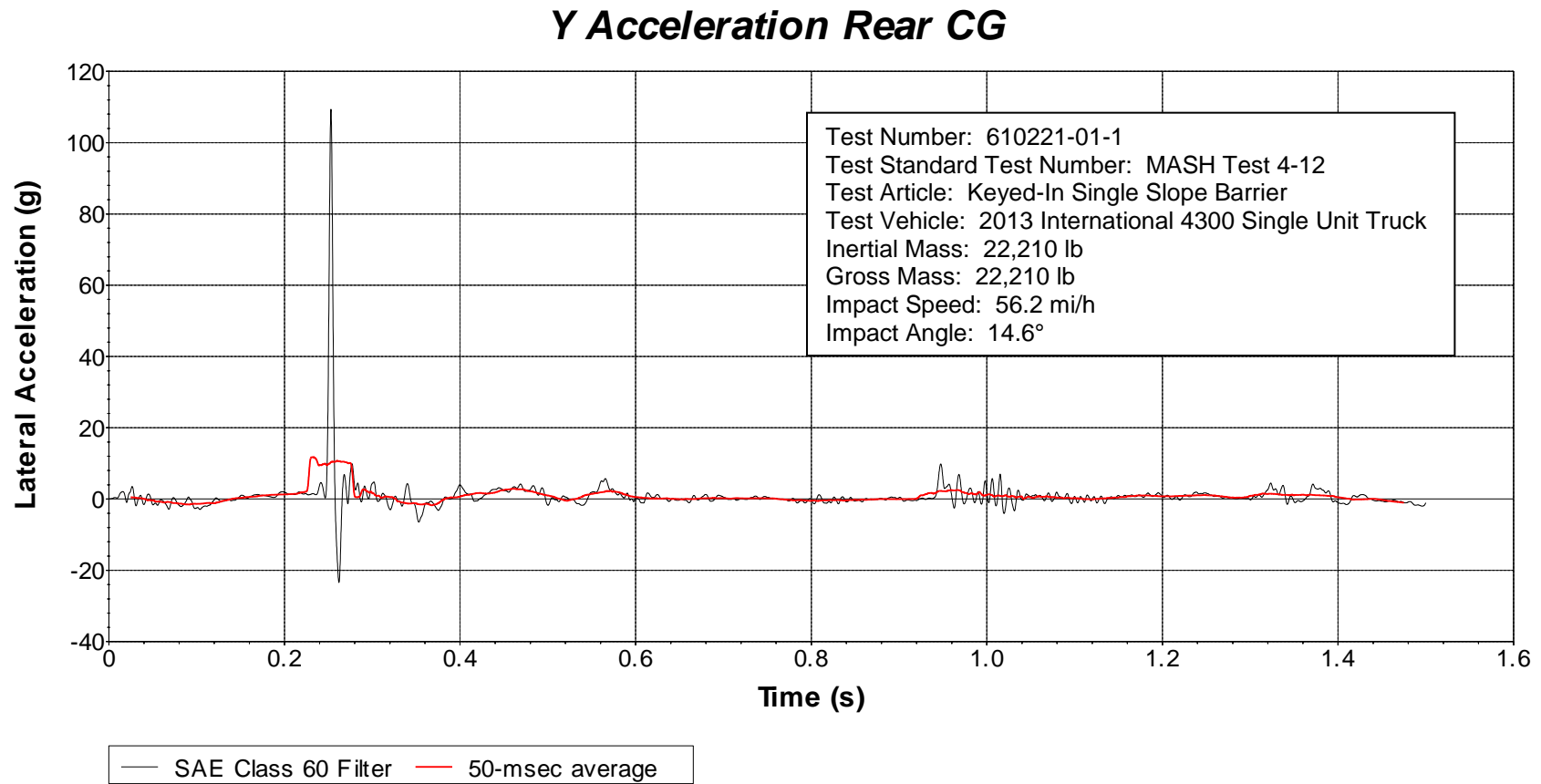
**Figure B.5. Vehicle Lateral Accelerometer Trace for Test No. 610221-01-1
(Accelerometer Located at Center of Gravity).**



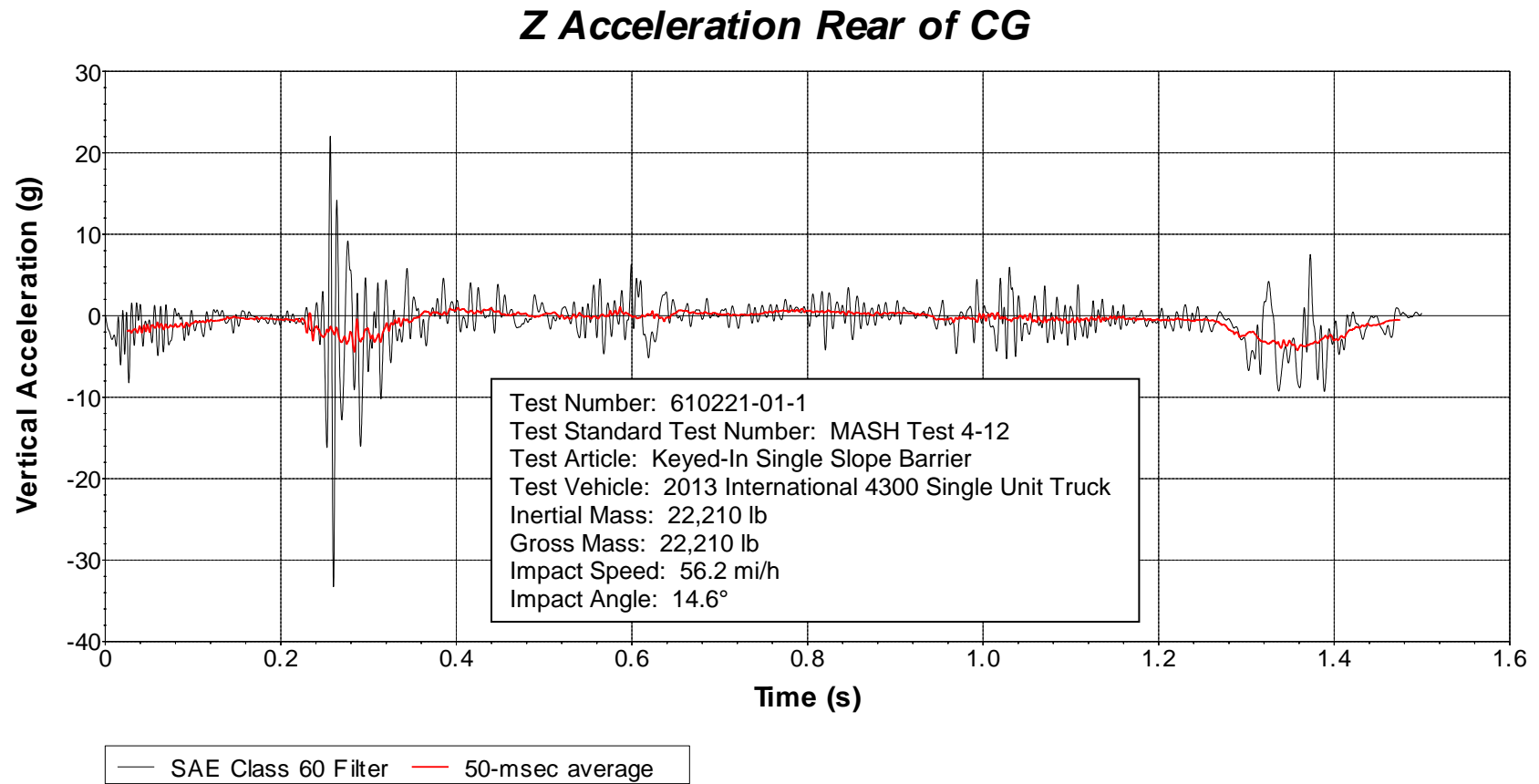
**Figure B.6. Vehicle Vertical Accelerometer Trace for Test No. 610221-01-1
(Accelerometer Located at Center of Gravity).**



**Figure B.7. Vehicle Longitudinal Accelerometer Trace for Test No. 610221-01-1
(Accelerometer Located Rear of Center of Gravity).**



**Figure B.8. Vehicle Lateral Accelerometer Trace for Test No. 610221-01-1
(Accelerometer Located Rear of Center of Gravity).**



**Figure B.9. Vehicle Vertical Accelerometer Trace for Test No. 610221-01-1
(Accelerometer Located Rear of Center of Gravity)**